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0470

DECEMBER 2005
100% CON. DOCS.

144th COMPANY READINESS CENTER CAMP W.G. WILLIAMS

RIVERTON, UTAH

DFCM PROJECT #04042480

OWNER UTAH STATE DIVISION OF FACILITIES CONSTRUCTION AND MANAGEMENT
4110 STATE OFFICE BUILDING
Salt Lake City, Utah 84114
PH#: (801) 530-3018
FAX: (801) 538-3267
CONTACT:

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CONTACT: Stoney Jensen

CODE SUMMARY

BASED ON THE 2003 IBC

• OCCUPANCY CLASSIFICATION	TRAINING FACILITY	PROPOSED OCCUPANCY: A-3 - NON SEPARATED PER 302.3.1	SECTION 302
• CONSTRUCTION TYPE	TYPE III-B	FIRE SPRINKLERS PROVIDED	SECTION 601
• ALLOWABLE AREA / HEIGHT	BASIC: 9,500 SF / 2 STORY / 55 FT. ACTUAL SF = 17,547 (2) STORY ALLOWABLE INCREASE WITH SPRINKLER = 200%		TABLE 503
• OCCUPANCY SEPARATION	NOT REQUIRED		TABLE 302.3.3
• FIRE RATINGS IN HOURS	STRUCTURAL FRAME 0 HR. BEARING WALLS 0 HR. *EXTERIOR WALLS 2 HR. INTERIOR WALLS 0 HR. FLOOR CONSTRUCTION 0 HR. ROOF CONSTRUCTION 0 HR.		TABLE 601
	*EXTERIOR WALLS 0 HR. > 30 FT.		TABLE 602
• OCCUPANT LOAD EXIT REQUIREMENTS	MAIN LEVEL 260 OCCUPANTS UPPER LEVEL 111 OCCUPANTS	TOTAL OCCUPANT LOAD = 371	TABLE 1004.1.2.
	TRAVEL DISTANCE MAX. ALLOWABLE = 250' W/ SPRINKLER SYSTEM		TABLE 1015.1
	STAIR WIDTH REQUIRED = 23" (111x.2)MIN. REQUIRED 50" PROVIDED		TABLE 1005.1
	EXIT WIDTH REQUIRED = 39" (260x..15)MIN. REQUIRED 62" PROVIDED		
• PLUMBING FIXTURE REQUIREMENTS	TOILET FIXTURES REQUIRED: WATER CLOSETS MALE: 2 REQUIRED, 9 PROVIDED FEMALE: 1 REQUIRED, 5 PROVIDED LAVATORIES MALE: 2 REQUIRED, 5 PROVIDED FEMALE: 2 REQUIRED, 5 PROVIDED 1 DRINKING FOUNTAIN PROVIDED AND 1 MOP SINK PROVIDED		
• INCIDENTAL USE AREAS	VEHICLE TRAINING BAY TOTAL S.F. = 1072 SF REQUIRES 1 HOUR SEPARATION FOR PARKING GARAGE IN FULLY SPRINKLERED BUILDING.		TABLE 302.1.1

DRAWING INDEX:

ARCHITECT AJC PROJECT #0470

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STATE OF UTAH · KENT R. RIGBY · LICENSED ARCHITECT · 135096 · 12/16/05

OWNER INFORMATION

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Internet: http://www.dfcm.utah.gov

PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

TITLE SHEET

REVISIONS

MARK	DATE	DESCRIPTION

ISSUE DATA

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DFCM PROJECT #	04042480

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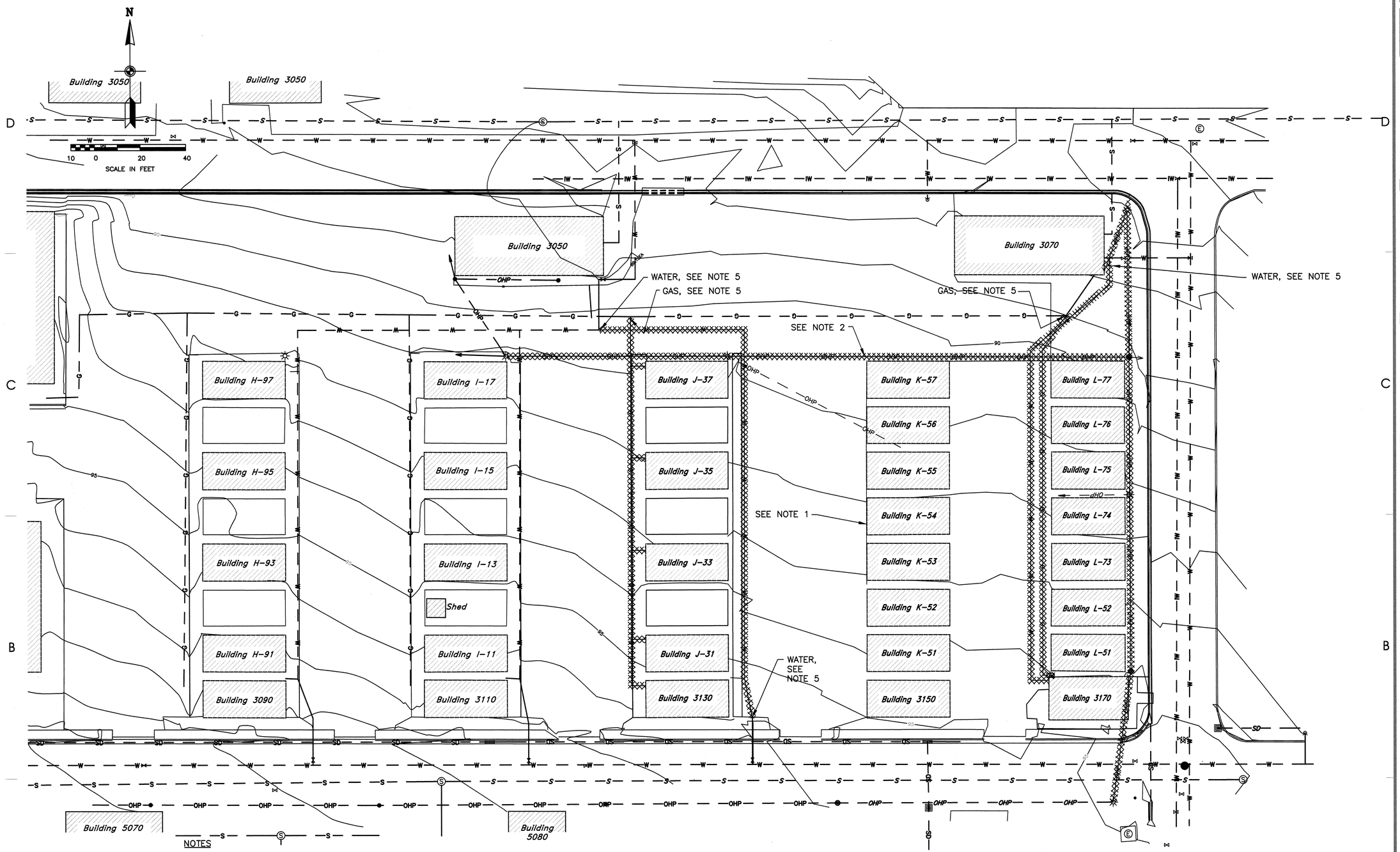
GI001



NOTES

1. SEE ARCHITECTURAL SITE DRAWINGS FOR SITE, AND BUILDING DEMOLITION.
2. SEE ELECTRICAL ENGINEERING PLANS FOR DEMOLITION OF EXISTING POWER.
3. CONTRACTOR SHALL MINIMIZE UTILITY SERVICE DISRUPTIONS. UTILITY DISRUPTIONS SHALL BE COORDINATED WITH OWNER AND CITY AND SHALL REQUIRE 48 HOURS NOTICE AND SHALL TAKE PLACE DURING OFF-HOURS AS APPROVED BY CONSTRUCTION MANAGER. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY TEMPORARY PUMPING, BYPASSING, ETC. THAT MAY BE REQUIRED. CONTRACTOR SHALL NOT INTERRUPT UTILITY SERVICE UNTIL NEW UTILITIES ARE READY TO BE PLACED INTO SERVICE.
4. SEE SHEET CU101 FOR OTHER UTILITY NOTES.
5. FOR UTILITIES TO BE REMOVED, CONTRACTOR SHALL CAP OR PLUG AND SEAL ENDS OF REMAINING PIPES OR OTHER CONDUITS. FOR SANITARY SEWER, REMOVED LINES SHALL BE CAPPED OR PLUGGED AND SEALED AT THE PROPERTY LINE. FOR CULINARY WATER, REMOVED LINES SHALL BE CAPPED OR PLUGGED AND SEALED AT THE WATER MAIN PER CITY REQUIREMENTS AND SHALL BE WITNESSED BY A OWNER REPRESENTATIVE. REMOVING UTILITIES IN TRAFFIC AREAS SHALL INCLUDE NECESSARY TRAFFIC CONTROL, PAVEMENT SAW CUTTING, PAVEMENT DISPOSAL, AND PAVEMENT RESTORATION PER OWNER REQUIREMENTS.

LEGEND:

W	WATER	○	MANHOLE
S	SANITARY SEWER	▦	CATCH BASIN
SD	STORM DRAIN	⋈	VALVE
OHP	OVERHEAD POWER	▨	REMOVE EXISTING UTILITIES
IW	IRRIGATION WATER		
	FIRE HYDRANT		
	POWER POLE		

REGISTERED PROFESSIONAL ENGINEER
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STATE OF UTAH
12-2-2005

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

UTILITY
DEMOLITION PLAN

REVISIONS

MARK	DATE	DESCRIPTION

ISSUE DATA

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ISSUE TYPE:	CONDOCS
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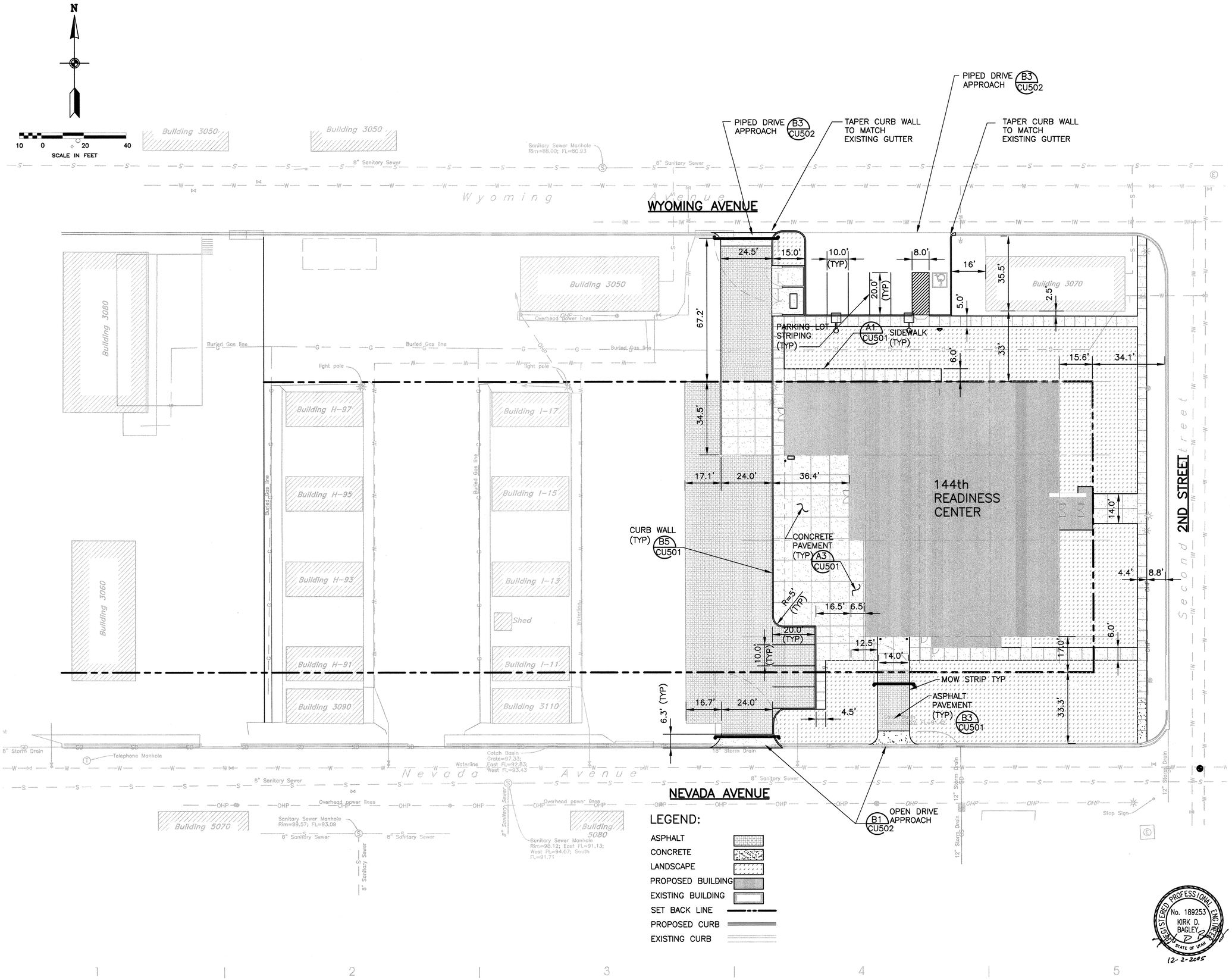

REGISTERED PROFESSIONAL ENGINEER
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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

SITE PLAN

REVISIONS

MARK	DATE	DESCRIPTION

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ISSUE DATE:	DEC. 2005
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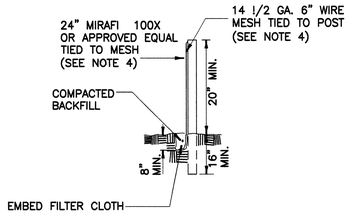
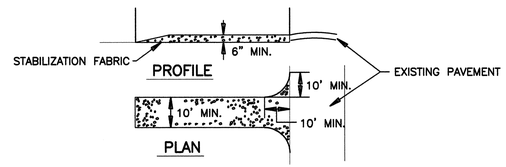


NOTES:

1. POSTS SPACED 10' O.C. MAX.
2. FILTER CLOTH TO BE TIED TO MESH EVERY 24" AT TOP AND MIDDLE WITH 6" FOLDED OVERLAP AT VERTICAL SEAMS
3. FENCE SHALL BE MAINTAINED AND ACCUMULATED MATERIAL REMOVED
4. CONTRACTOR MAY SECURE SILT FENCE FILTER CLOTH TO CHAIN LINK CONSTRUCTION FENCE WHERE APPLICABLE. IN INSTANCES WHERE FILTER CLOTH IS SECURED TO CHAIN LINK FENCE, WIRE MESH IS NOT REQUIRED. IF FILTER CLOTH IS NOT SECURED TO CHAIN LINK FENCE, IT MUST BE ATTACHED TO WIRE MESH PER THE DETAIL.

1 SILT FENCE DETAIL

N.T.S.



CONSTRUCTION NOTES:

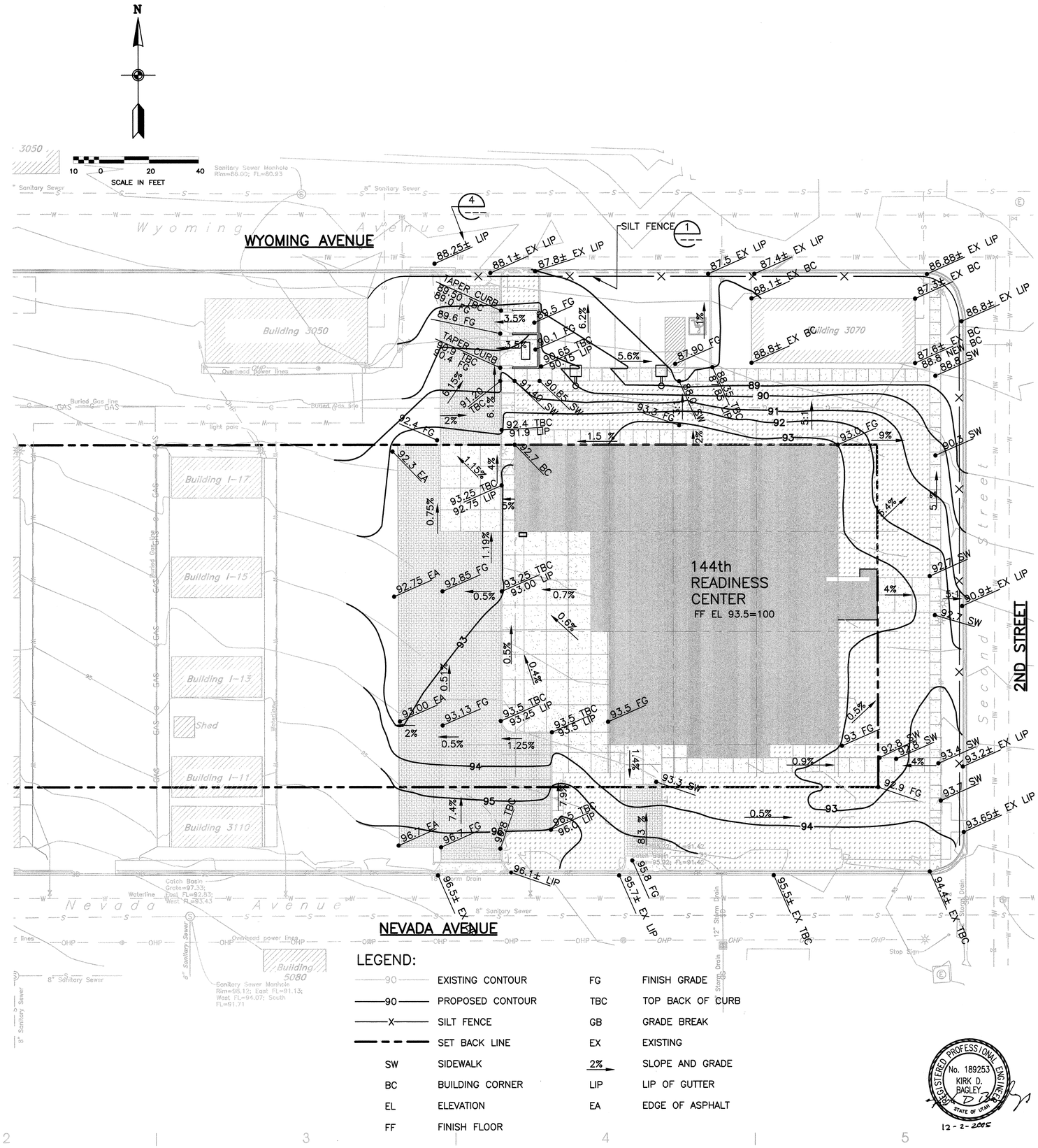
1. STONE SIZE – USE 2" STONE, OR RECLAIMED CONCRETE EQUIVALENT.
2. LENGTH – AS REQUIRED, BUT NOT LESS THAN 50 FEET (EXCEPT ON A SINGLE RESIDENCE LOT WHERE A 30 FOOT MINIMUM LENGTH WOULD APPLY).
3. THICKNESS – NOT LESS THAN SIX (6) INCHES.
4. WIDTH – TEN (10) FOOT MINIMUM, BUT NOT LESS THAN THE FULL WIDTH AT POINTS WHERE INGRESS OR EGRESS OCCURS.
5. STABILIZATION FABRIC – WILL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING STONE.
6. SURFACE WATER – ALL SURFACE WATER FLOWING OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED ACROSS THE ENTRANCE. IF PIPING IS IMPRACTICAL, A MOUNTABLE BERM WITH 5:1 SIDE SLOPES WILL BE PERMITTED.
7. MAINTENANCE – THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS–OF–WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS–OF–WAY MUST BE REMOVED IMMEDIATELY.
8. WASHING – WHEELS SHALL BE CLEANED TO REMOVE SEDIMENT PRIOR TO ENTRANCE ONTO PUBLIC RIGHTS–OF–WAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE AND WHICH DRAINS TO AN APPROVED SEDIMENT TRAPPING DEVICE.
9. PERIODIC INSPECTION AND NEEDED MAINTENANCE SHALL BE PROVIDED AFTER EACH RAIN.

4 STABILIZED CONSTRUCTION ENTRANCE DETAIL

N.T.S.

EROSION CONTROL NOTES:

1. THE CONTRACTOR SHALL PROVIDE THE SEDIMENTATION AND EROSION CONTROL SYSTEMS AS SHOWN HEREON IN ADDITION TO WHATEVER OTHER MEASURES MAY BE NECESSARY TO PREVENT SEDIMENTATION FROM BEING TRANSPORTED TO EXISTING STORM DRAIN AND ADJACENT STREETS.
2. CLEAR AND GRUB SITE PERIMETER WHERE DEMOLITION DID NOT OCCUR.
3. INSTALL TEMPORARY RETENTION AREAS TO ALLOW FOR SEDIMENTATION AND FILTRATION TO OCCUR BEFORE BEING PUMPED TO STORM DRAIN.
4. THIS EROSION CONTROL PLAN AND TEMPORARY DEWATERING DETAILS ARE SUGGESTED METHODS. THE CONTRACTOR MAY SUBMIT ALTERNATIVE EROSION CONTROL PLANS FOR APPROVAL. IT IS THE CONTRACTORS ULTIMATE RESPONSIBILITY TO PROVIDE EROSION CONTROL AND TEMPORARY DEWATERING.
5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ALL SLOPES IN A STABLE CONDITION.
6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ADJACENT IMPROVEMENTS FROM DAMAGE.
7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CLEANING ADJACENT STREETS AFFECTED BY CONSTRUCTION.
8. TRAFFIC CONTROL AND HAUL ROUTES MUST CONFORM TO THE MOST CURRENT EDITION OF SLC TRAFFIC BARRICADE MANUAL PART 6 OF "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES", FOR SALT LAKE COUNTY AND STATE ROADS.
9. SLC TRANSPORTATION MUST APPROVE ALL PROJECT HAUL ROUTES (801–535–7129). THE CONTRACTOR MUST ALSO CONFORM TO UDOT, SALT LAKE COUNTY OR OTHER APPLICABLE GOVERNMENT ENTITIES REQUIREMENTS FOR TRAFFIC CONTROL.



LEGEND:

90	EXISTING CONTOUR	FG	FINISH GRADE
90	PROPOSED CONTOUR	TBC	TOP BACK OF CURB
X	SILT FENCE	GB	GRADE BREAK
	SET BACK LINE	EX	EXISTING
SW	SIDEWALK	2%	SLOPE AND GRADE
BC	BUILDING CORNER	LIP	LIP OF GUTTER
EL	ELEVATION	EA	EDGE OF ASPHALT
FF	FINISH FLOOR		

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

SITE GRADING AND EROSION CONTROL PLAN

REVISIONS

MARK	DATE	DESCRIPTION

ISSUE DATA

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SHEET NUMBER:

CG101

NOTES

1. INSPECTIONS – IT IS THE CONTRACTOR'S RESPONSIBILITY TO SCHEDULE WATER, SEWER AND DRAINAGE INSPECTIONS AS APPLICABLE TO THE PROJECT 5 WORKING DAYS PRIOR TO WHEN NEEDED

2. DAMAGE TO EXISTING UTILITIES – THE CONTRACTOR IS RESPONSIBLE FOR REPAIRING ANY DAMAGE, CAUSED BY ANY CONDITION INCLUDING SETTLEMENT, TO EXISTING UTILITIES FROM WORK PERFORMED AT OR NEAR EXISTING UTILITIES.

3. UTILITY LOCATIONS – CONTRACTOR WILL BE RESPONSIBLE FOR LOCATING AND AVOIDING ALL UTILITIES AND SERVICE LATERALS, AND FOR REPAIRING ALL DAMAGE THAT OCCURS TO THE UTILITIES DUE TO THE CONTRACTOR'S ACTIVITIES.

4. POTHOLING – CONTRACTOR IS TO VERIFY LOCATION, DEPTH, SIZE, TYPE AND OUTSIDE DIAMETERS OF UTILITIES IN THE FIELD BY POTHOLING A MINIMUM OF 300 FEET AHEAD OF PIPELINE CONSTRUCTION TO AVOID CONFLICTS WITH DESIGNED PIPELINE GRADE AND ALIGNMENT. EXISTING UTILITY INFORMATION MUST BE ASSUMED AS APPROXIMATE AND REQUIRING FIELD VERIFICATION.

5. UTILITY RELOCATIONS – FOR UTILITY CONFLICTS REQUIRING MAINLINE RELOCATIONS, THE CONTRACTOR MUST NOTIFY THE APPLICABLE UTILITY COMPANY OR USER A MINIMUM OF 2–WEEKS IN ADVANCE. ONE WEEK NOTIFICATION IS REQUIRED FOR CONFLICTS REQUIRING THE RELOCATION OF SERVICE LATERALS. ALL RELOCATIONS ARE SUBJECT TO APPROVAL FROM THE APPLICABLE UTILITY COMPANY OR USER.

6. FIELD CHANGES – NO ROADWAY OR UTILITY ALIGNMENT OR GRADE CHANGES ARE ALLOWED FROM THE APPROVED CONSTRUCTION PLANS/DOCUMENTS WITHOUT APPROVAL FROM THE OWNER. CHANGES TO HYDRANT LOCATIONS AND/OR FIRE LINES MUST BE REVIEWED AND APPROVED BY THE OWNER.

7. NOTICE FOR WATER MAIN SHUT DOWNS – THROUGH OWNER AND WITH THE CONSTRUCTION MANAGERS APPROVAL, OWNER MUST BE CONTACTED AND APPROVE ALL WATER MAIN SHUTDOWNS. ONCE APPROVED THE OWNER WILL NOTIFY ALL EFFECTED USERS BY WRITTEN NOTICE PRIOR TO THE WATER MAIN SHUT DOWN.

8. WATER AND SEWER SEPARATION – IN ACCORDANCE WITH UTAH'S DEPARTMENT OF HEALTH REGULATIONS AS AMENDED, A MINIMUM TEN FOOT HORIZONTAL AND 1.5–FOOT VERTICAL (WATER ON TOP) SEPARATION IS REQUIRED. IF THESE CONDITIONS CAN NOT BE MET, STATE, APPROVAL IS REQUIRED. ADDITIONAL CONSTRUCTION MEASURES WILL BE REQUIRED FOR THESE CONDITIONS.

9. SALVAGE – ALL METERS, VALVES AND HYDRANTS MUST BE RETURNED TO OWNER, AND AT OWNER'S REQUEST ALL SALVAGED PIPE AND/OR FITTINGS MUST BE RETURNED TO OWNER'S SHOPS.

10. SEWER, WATER/FIRE AND DRAINAGE MAINS AND SERVICES – OWNER MUST APPROVE ALL WATER AND SEWER CONNECTIONS. ALL SEWER LATERALS 6–INCHES AND SMALLER MUST WYE INTO THE MAINS PER OWNER REQUIREMENTS. A MINIMUM 4–FOOT BURY DEPTH IS REQUIRED ON ALL SEWER MAINS AND LATERALS. CONTRACTOR SHALL INSTALL INVERT COVERS IN ALL SEWER MANHOLES IN THE PROJECT AREA.

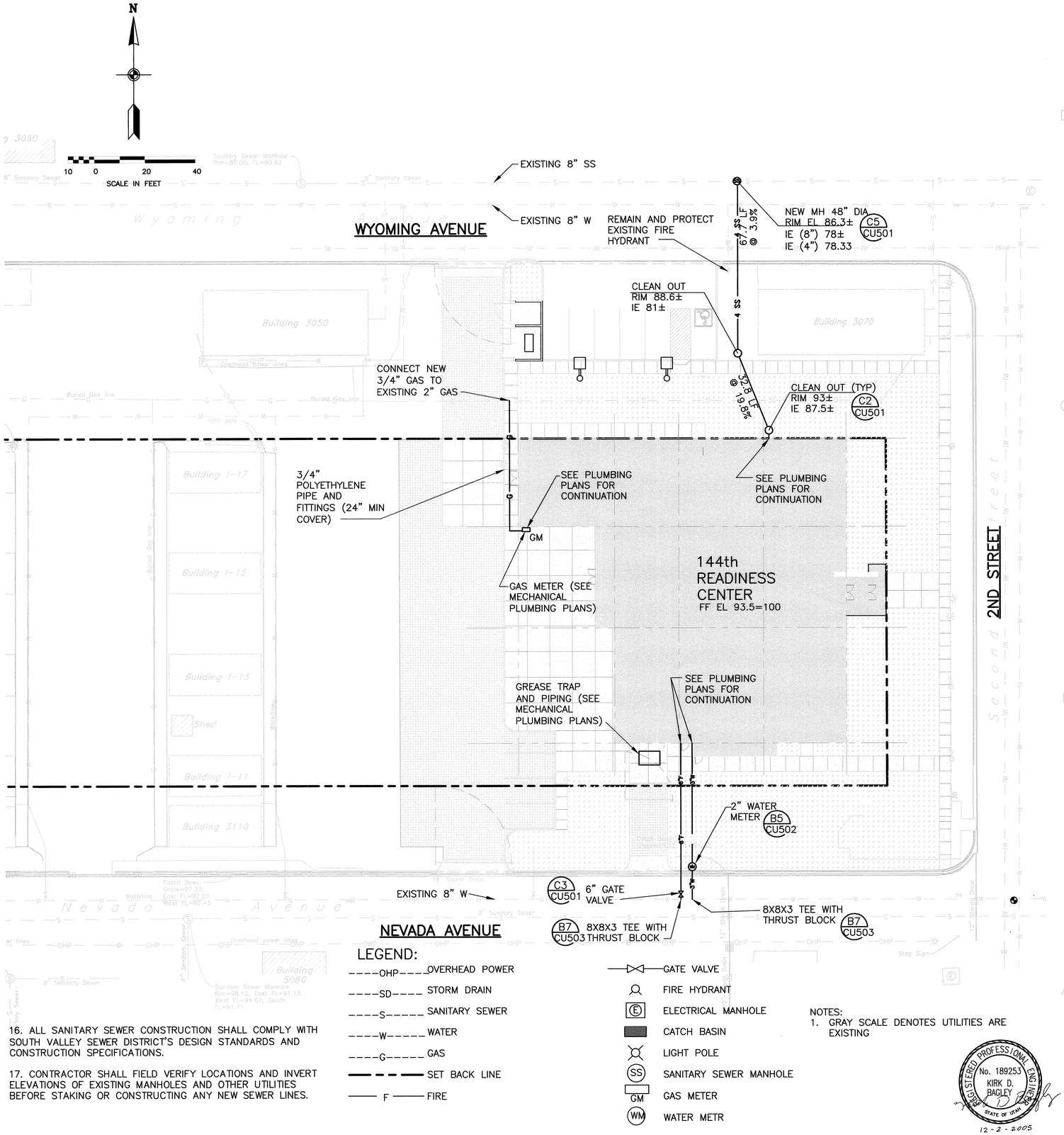
11. A MINIMUM 3–FOOT SEPARATION IS REQUIRED BETWEEN ALL WATER CONNECTIONS OR TAPS INTO THE MAIN. ALL CONNECTIONS MUST BE MADE PER OWNER MINIMUM REQUIREMENTS. A 5–FOOT MINIMUM BURY DEPTH (FINAL GRADE TO TOP OF PIPE) IS REQUIRED ON ALL WATER/FIRE LINES UNLESS OTHERWISE APPROVED BY OWNER. WATER LINE THRUST BLOCK/RESTRAINTS PER OWNER APPROVED DETAIL DRAWINGS AND SPECIFICATIONS. ALL EXPOSED NUTS AND BOLTS WILL BE COATED WITH CHEVRON FM1 GREASE PLUS MINIMUM 8 MIL THICKNESS PLASTIC PER OWNER REQUIREMENTS. PROVIDE STAINLESS STEEL NUTS, BOLTS AND WASHERS FOR HIGH GROUNDWATER/ SATURATED CONDITIONS AT FLANGE FITTINGS, ETC. BACKFLOW PREVENTORS ARE REQUIRED ON ALL IRRIGATION TAPS PER CAMPUS DESIGN AND CONSTRUCTION.

12. ALL WATERLINES INSTALLATIONS AND TESTING TO BE IN ACCORDANCE WITH AWWA SECTIONS C600, C601, C651, C206, C200, C900, C303 AWWA MANUAL M11 AND ALL OTHER APPLICABLE AWWA, UPWS, ASTM AND ANSI SPECIFICATIONS RELEVANT TO THE INSTALLATION AND COMPLETION OF THE PROJECT. (AMENDMENT TO SECTION C600 SECTION 4.1.1, DOCUMENT TO READ MINIMUM TEST PRESSURE SHALL NOT BE LESS THAN 200 P.S.I. GAUGED TO A HIGH POINT OF THE PIPELINE BEING TESTED. ALL MATERIALS USED FOR WATERWORKS PROJECTS TO BE RATED FOR 150 P.S.I. MINIMUM OPERATING PRESSURE.)

13. CONTRACTOR TO INSTALL WATER SERVICE LINE, METER YOKE AND METER BOX WITH LID LOCATED AS SHOWN ON THE PLANS AND APPROVED OWNER DETAIL DRAWINGS. METER BOX TO BE PLACED IN THE PARK STRIP PERPENDICULAR TO THE WATERMAIN SERVICE CONNECTION. ALL DETECTOR CHECK VALVES, WATER METERS, CATCH BASINS, CLEANOUT BOXES AND MANHOLES MUST BE LOCATED OUTSIDE OF ALL APPROACHES, DRIVEWAYS, PEDESTRIAN WALKWAYS AND OTHER TRAVELED WAYS UNLESS OTHERWISE APPROVED ON PLANS.

14. CONTRACTOR TO PROVIDE AIR PRESSURE TESTING OF SEWER MAINS IN ACCORDANCE WITH PIPE MANUFACTURERS RECOMMENDATIONS AND OWNER AS APPLICABLE. ALL PVC SEWER MAIN AND LATERAL TESTING SHALL BE IN ACCORDANCE WITH UNI–BELL UN–B–6–98 RECOMMENDED PRACTICE FOR LOW PRESSURE AIR TESTING OF INSTALLED SEWER PIPE. CONTRACTOR SHALL PROVIDE SEWER LATERAL WATER TESTING AS REQUIRED BY THE OWNER. A MINIMUM 9 FEET OF HEAD PRESSURE AS MEASURED VERTICALLY FROM THE HIGH POINT OF THE PIPELINE AND AT OTHER LOCATIONS ALONG THE PIPELINE AS DETERMINED BY THE OWNER SHALL BE TESTED. TEST TIME WILL BE NO LESS THAN AS SPECIFIED FOR THE AIR TEST DURATION IN TABLE I ON PAGE 12 OF UNI–B–6–98. ALL PIPES SUBJECT TO WATER TESTING SHALL BE FULLY VISIBLE TO THE INSPECTOR AFTER INSTALLATION. TESTING MUST BE PERFORMED IN THE PRESENCE OF AN OWNER. ALL VISIBLE LEAKAGE MUST BE REPAIRED TO THE SATISFACTION OF THE CAMPUS DESIGN AND CONSTRUCTION.

15. CONTRACTOR SHALL NOT ALLOW ANY GROUNDWATER OR DEBRIS TO ENTER THE NEW OR EXISTING PIPE DURING CONSTRUCTION, UTILITY TRENCHING, BACKFILL, AND PIPE ZONE AS PER OWNER.



LEGEND:

----OHP----	OVERHEAD POWER
----SD----	STORM DRAIN
----S----	SANITARY SEWER
----W----	WATER
----G----	GAS
— — —	SET BACK LINE
—— F ——	FIRE
	GATE VALVE
	FIRE HYDRANT
(E)	ELECTRICAL MANHOLE
	CATCH BASIN
	LIGHT POLE
(SS)	SANITARY SEWER MANHOLE
GM	GAS METER
(WM)	WATER METR

16. ALL SANITARY SEWER CONSTRUCTION SHALL COMPLY WITH SOUTH VALLEY SEWER DISTRICT'S DESIGN STANDARDS AND CONSTRUCTION SPECIFICATIONS.

17. CONTRACTOR SHALL FIELD VERIFY LOCATIONS AND INVERT ELEVATIONS OF EXISTING MANHOLES AND OTHER UTILITIES BEFORE STAKING OR CONSTRUCTING ANY NEW SEWER LINES.

NOTES:
1. GRAY SCALE DENOTES UTILITIES ARE EXISTING

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12 - 2 - 2005

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CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:
SITE UTILITY PLAN

REVISIONS

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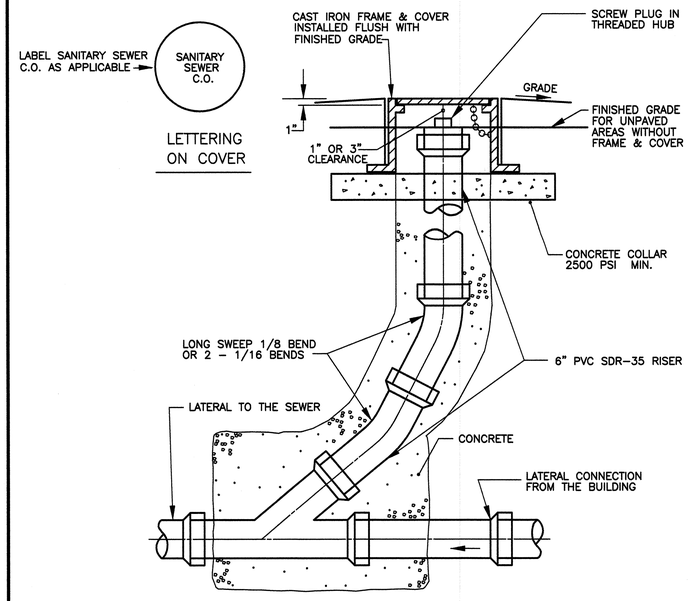
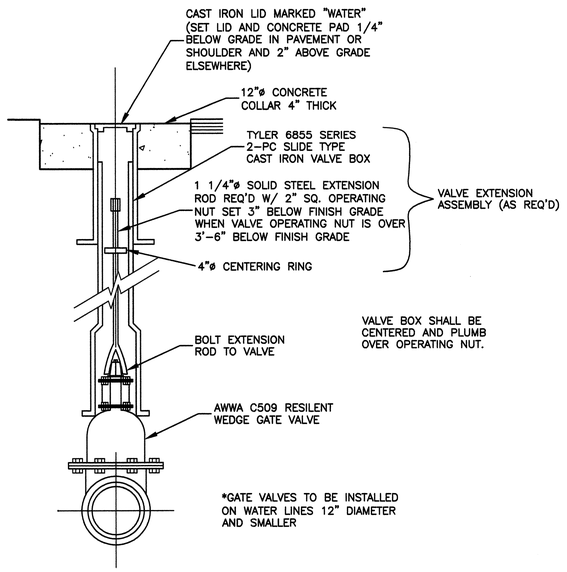


SEWER CLEANOUT C2
NTS CU501



GATE VALVE C3
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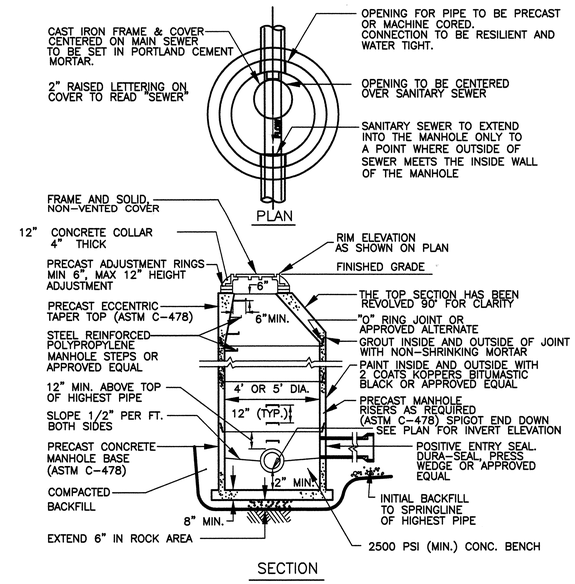
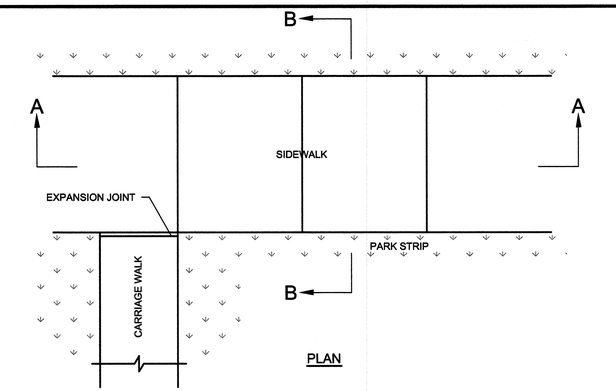
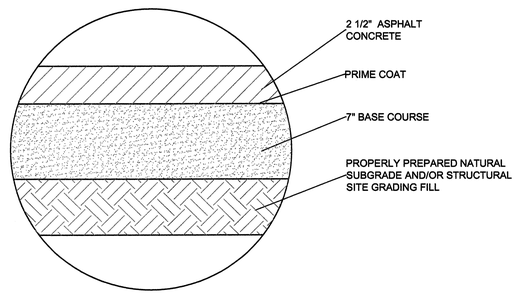


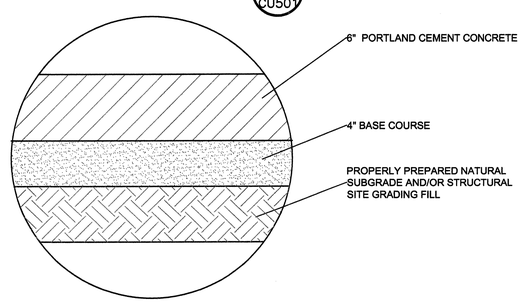
SANITARY SEWER MANHOLE C5
NTS CU501



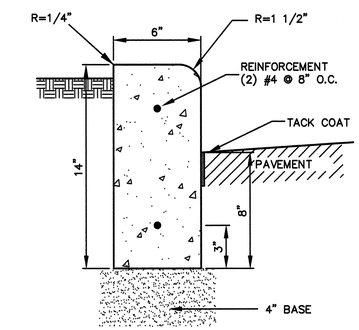
CONCRETE SIDEWALK A1
NTS CU501



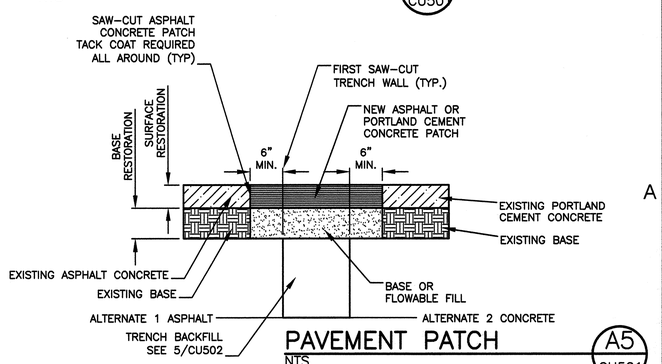
ASPHALT PAVEMENT B3
NTS CU501



CONCRETE PAVEMENT A3
NTS CU501



CURB WALL B5
NTS CU501



PAVEMENT PATCH A5
NTS CU501

REGISTERED PROFESSIONAL ENGINEER
No. 189253
KIRK D. BAGLEY
STATE OF UTAH
12-2-2005

AJC PROJECT # 0470

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:
DETAILS

REVISIONS
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ISSUE DATA
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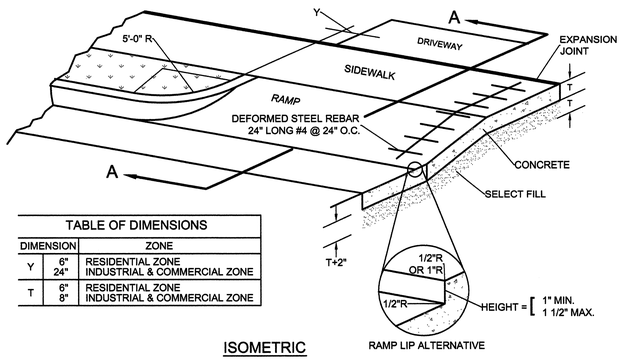


TABLE OF DIMENSIONS		
DIMENSION		ZONE
Y	6" 24"	RESIDENTIAL ZONE INDUSTRIAL & COMMERCIAL ZONE
T	6" 8"	RESIDENTIAL ZONE INDUSTRIAL & COMMERCIAL ZONE

ISOMETRIC

RAMP LIP ALTERNATIVE

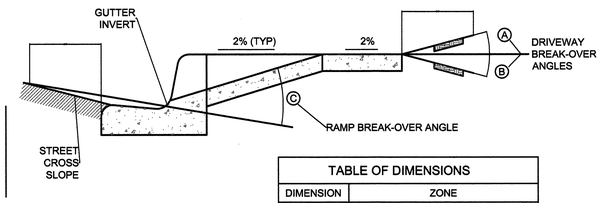


TABLE OF DIMENSIONS		
DIMENSION		ZONE
A	≤ 6% ≤ 6%	RESIDENTIAL ZONE INDUSTRIAL & COMMERCIAL ZONE
B	≤ 12% ≤ 8%	RESIDENTIAL ZONE INDUSTRIAL & COMMERCIAL ZONE
C	≤ 12% ≤ 10%	RESIDENTIAL ZONE INDUSTRIAL & COMMERCIAL ZONE

SECTION A-A

B1 CU502 OPEN DRIVEWAY APPROACH
NTS

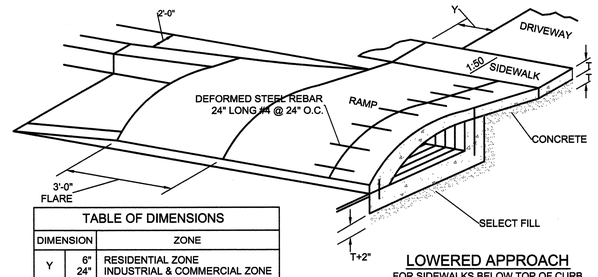
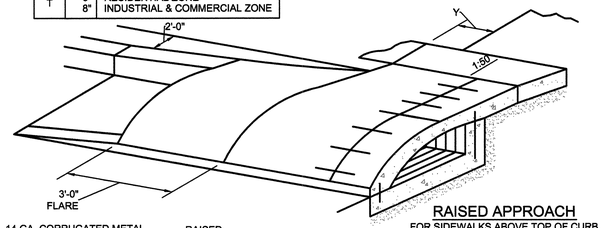
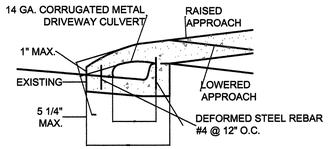


TABLE OF DIMENSIONS		
DIMENSION		ZONE
Y	6" 24"	RESIDENTIAL ZONE INDUSTRIAL & COMMERCIAL ZONE
T	6" 8"	RESIDENTIAL ZONE INDUSTRIAL & COMMERCIAL ZONE

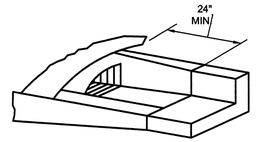
LOWERED APPROACH
FOR SIDEWALKS BELOW TOP OF CURB



RAISED APPROACH
FOR SIDEWALKS ABOVE TOP OF CURB

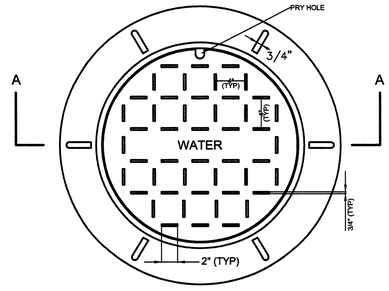


SECTION

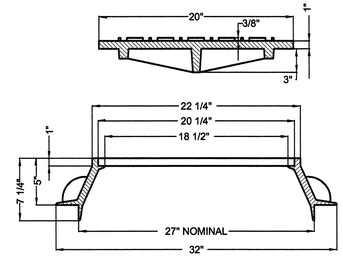


TRANSITION SECTION

B3 CU502 PIPED DRIVEWAY APPROACH
NTS



PLAN

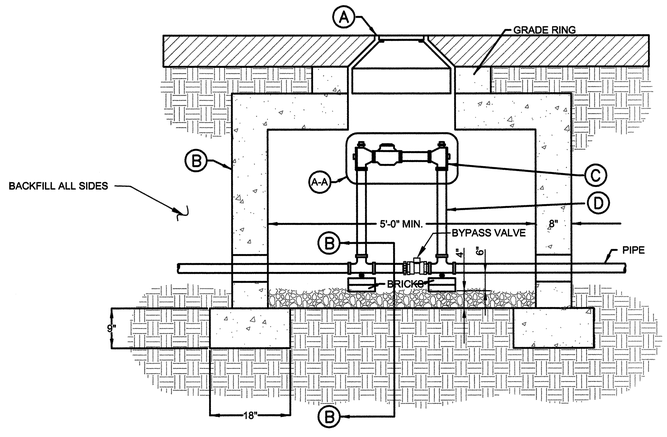


SECTION A-A

27" FRAME AND COVER

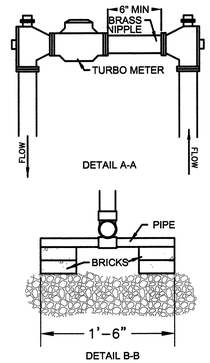
1. CASTINGS: GREY IRON CLASS 30 MINIMUM PER ASTM A 48.
2. COATINGS: EXCEPT MACHINED SURFACES, COAT ALL METAL PARTS WITH ASPHALTUM PAINT.
3. INSCRIPTIONS: CAST THE NAME OF THE AGENCY OR ITS ACRONYM AS THE FIRST LINE. CAST THE WORD "WATERWORKS" AS THE SECOND LINE. CAST THE WORD 'VALVE' (OR APPLICABLE WORD) AS THE THIRD LINE. CAST ALL LETTERS ON THE COVER IN UPPER CASE FLUSH WITH THE SURFACE FINISH.
4. HEAT NUMBER: PLACE FOUNDRY AND HEAT NUMBER ON THE INSIDE OF THE FRAME AND ON THE BOTTOM OF THE COVER.

B4 CU502 27" FRAME AND COVER
NTS



NOTES:

1. ALL WORK MUST BE INSPECTED BY ENGINEER PRIOR TO BACKFILL.
2. FROM THE TOP OF THE LID TO THE CENTER LINE OF THE METER TO BE 20"± 4".
3. CENTER MANHOLE OVER METER.
4. INSTALL BRICK BLOCKING UNDER METER.
5. BYPASS VALVE TO BE LEFT IN THE OFF POSITION.
6. ALLOW A 1" CLEARANCE AROUND WATER LINE WHERE PASSING THROUGH CONCRETE WALLS. SEAL HOLE WITH RAMNECK SEALANT.
7. MINIMUM DEPTH FOR WATER SERVICE TO BE 48 INCHES EXCEPT WHERE 60 INCHES IS DESIGNATED BY THE ENGINEER. 11. SUBSTITUTIONS: APWA 01630

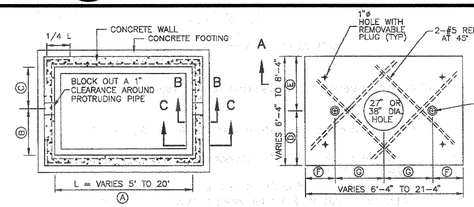


LEGEND

ITEM	DESCRIPTION	PART NUMBER
A	MANHOLE RING AND COVER	BY B4/CU502
B	CONCRETE BOX	BY B6/CU502
C	METER BOLTS	5/8"x2-3/4" BRASS
D	2" CUSTOM SETTER WITH BYPASS	SEE SPECS

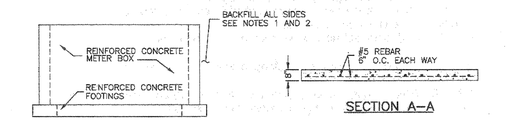
B5 CU502 2" METER
NTS

REGISTERED PROFESSIONAL ENGINEER
No. 189253
KIRK D. BAGLEY
STATE OF UTAH
12-2-2005



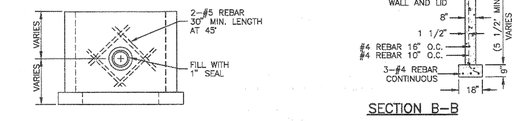
BOX PLAN

LID PLAN



FRONT ELEVATION

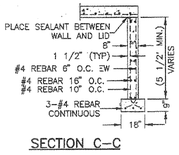
SECTION A-A



SIDE ELEVATION

SECTION B-B

TABLE OF DIMENSIONS						
	METER SIZES					
No.	10"	8"	6"	3" & 4"	1 1/2" & 2"	
A	20'-0"	11'-0"	11'-0"	9'-0"	5'-0"	
B	3'-10"	3'-9"	3'-5"	3'-0"	2'-6"	
C	6'-2"	3'-9"	3'-5"	2'-0"	2'-6"	
D	4'-6"	4'-1"	4'-1"	3'-8"	2'-6"	
E	6'-10"	4'-1"	4'-1"	2'-8"	2'-6"	
C	4'-8"	3'-5 3/4"	3'-10 1/4"	3'-5 1/2"	CENTER	
G	5'-4"	2'-8 1/4"	2'-3 3/4"	1'-8 1/2"		



SECTION C-C

B6 CU502 CONCRETE BOX
NTS

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Salt Lake City, Utah 84114
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CAMP WG WILLIAMS
RIVERTON, UTAH

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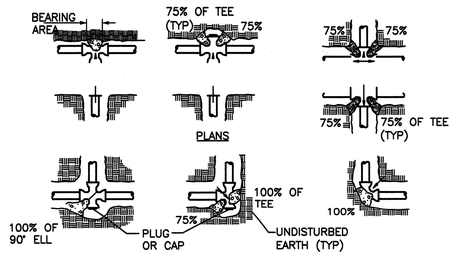


NOTE:
ARROWS INDICATE THRUST DIRECTION

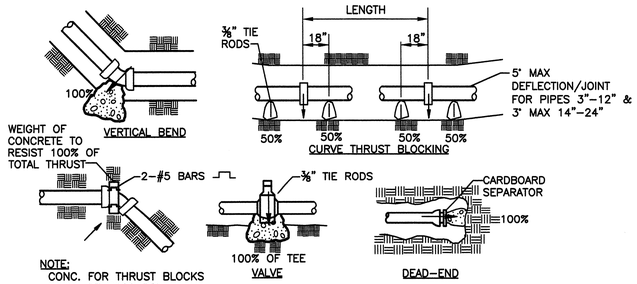
FIGURE (100%) AT THRUST BLOCK INDICATES PERCENT OF TOTAL THRUST TO BE APPLIED FOR BEARING AREA.
NOTE:

THRUST PER PSI OF WATER PRESSURE AT VARIOUS FITTINGS				
PIPE SIZE	DEAD END OR TEE	90° ELBOW	45° ELBOW	22½° ELBOW
4	19	27	15	7
6	39	55	30	15
8	67	94	51	26
10	109	154	84	43
12	155	218	119	61
14	210	296	161	82
16	275	383	209	106
18	351	494	269	137
20	434	611	333	169
24	623	878	478	244

EXAMPLE:
8-INCH 90° ELBOW, PRESSURE = 200lb./SQ. IN. FROM TABLE: THRUST=94 X 200 = 18,800 lb. ASSUME BEARING STRENGTH OF SOIL = 2000 lb./SQ. FT.

$\frac{18,800}{2000}$ =9.4 SQ. FT. =AREA OF BEARING REQUIRED FOR THRUST BLOCK.

B7 / CU502 CONCRETE THRUST BLOCKS
NTS

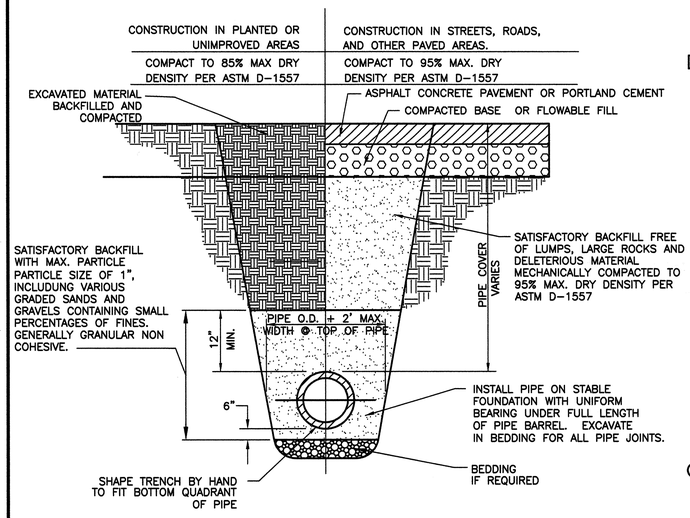


NOTE:
CONC. FOR THRUST BLOCKS SHALL BE 2000 PSI

THRUST PER PSI OF WATER PRESSURE AT VARIOUS FITTINGS			
PIPE SIZE	SIDE THRUST–LB	PIPE SIZE	SIDE THRUST–LB
4	35	14	377
6	72	16	486
8	122	18	665
10	197	20	790
12	278	24	1150

MULTIPLY THRUST BY DEGREE OF DEFLECTION TO OBTAIN TOTAL THRUST

NOTES:
1. IN USING THE ABOVE TABLES, USE THE MAXIMUM INTERNAL PRESSURE ANTICIPATED (i.e. HYDROSTATIC TEST PRESSURE, POSSIBLE SURGE PRESSURE DUE TO PUMP SHUTOFF, ETC.)
2. SEE SOILS REPORT FOR BEARING STRENGTH OF SOIL, IN THE ABSENCE OF A SOILS REPORT, AN AVERAGE SOIL (SPENDABLE MEDIUM CLAY) CAN BE ASSUMED TO HAVE A BEARING STRENGTH OF 2000 PSF
3. THRUST BLOCKS ARE NOT REQUIRED ON PVC PIPE WITH SOLVENT WELDED JOINTS.
4. CONCRETE FOR THRUST BLOCKS TO BE 2000 PSI.



B8 / CU502 TYPICAL TRENCH EXCAVATION SECTION
NTS

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REGISTERED PROFESSIONAL ENGINEER
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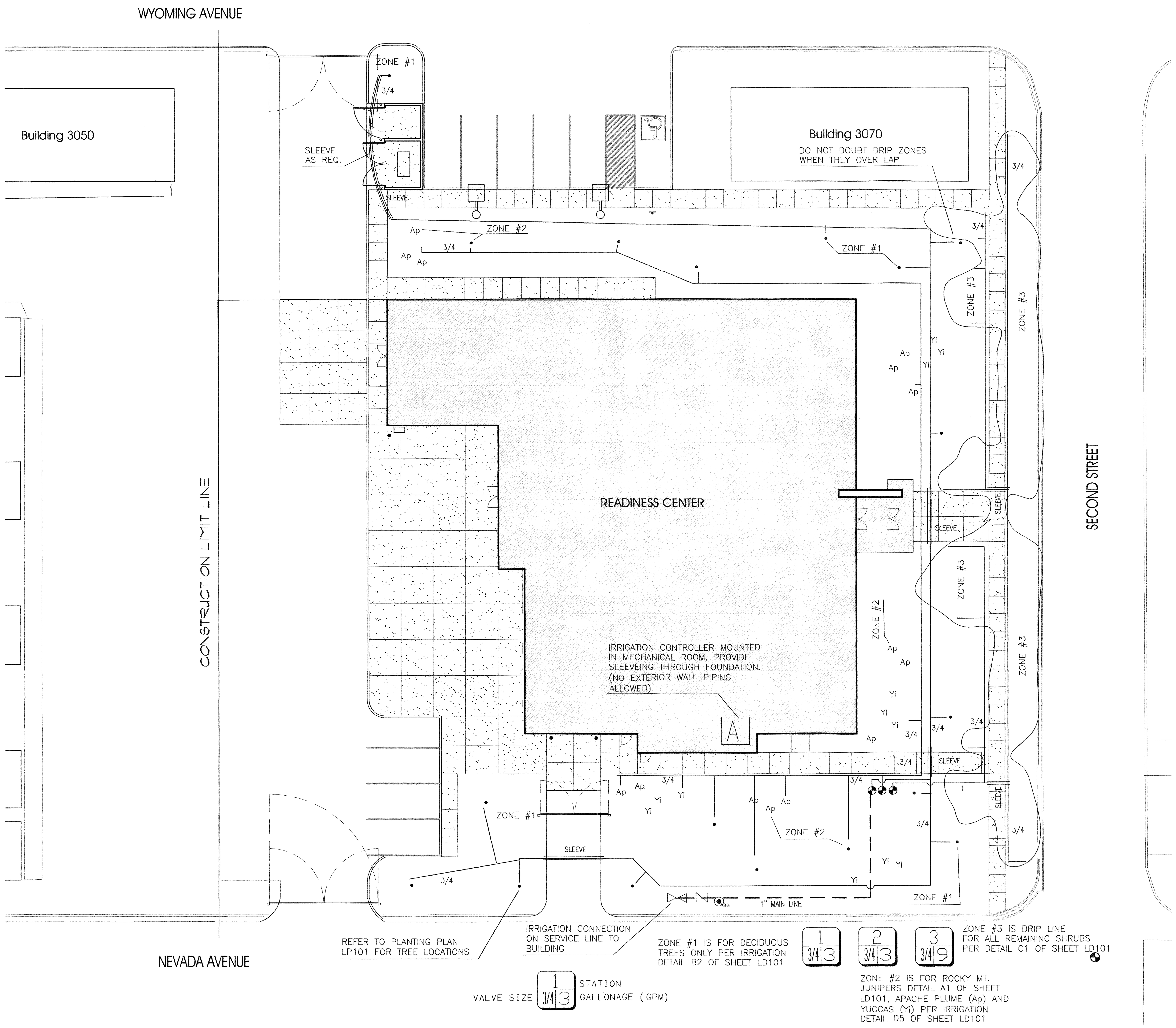

16' 0' 16' 32'
scale = 1" = 16'-0"

LANDSCAPE IRRIGATION PLAN (A3 / LI101)

GENERAL NOTES AND LEGEND:
*FOR SHEET LI101 ONLY.

- SEE SHEET AE001 FOR GENERAL NOTES.
- SEE COVER SHEET FOR DRAWING INDEX.
- SEE LANDSCAPE IRRIGAITON SHEET LPI01 FOR IRRIGATION SYSTEM
- DO NOT SCALE DRAWINGS.

SITE AREA LEGEND:
*FOR SHEET LI101 ONLY.

- IMPERVIOUS SURFACE AREA (CONCRETE)
- IMPERVIOUS SURFACE AREA (ASPHALT)

IRRIGATION LEGEND:
*FOR SHEET LI101 ONLY.

SYMBOL	DESCRIPTION
(valve symbol)	RAINBIRD DV 3/4" VALVES FOR DRIP ZONES WITH FILTER INSTALLED IN FRONT OF VALVE
(double check symbol)	WILKINS 3/4" 950XLTU DOUBLE CHECK VALVE WITH UNIONS AND "Y" STRAINER INSTALLED BELOW GRADE IN OVERSIZED VALVE BOX PER CODE.
(stop & waste symbol)	1" ORSEAL STOP & WASTE WITH GRAVEL SUMP, CURB BOX & GATE KEY.
qc	RAINBIRD 33DLRC QUICK COUPLER AND VALVE KEY 33K
A	RAINBIRD ESP-MC 8 STATION CONTROLLER MOUNTED ITH ENTERIOR MECHANICAL ROOM. SLEEVE UNDER FOOTINGS AS REQUIRED FOR WIRES. COORDINATE AND PROVIDE POWER TO AND GROUND CONTROLLER.
———	LATERAL LINE SCH 40 PVC WITH A 8" BURY PER PLAN, SIZED AS NOTED, 8 GPM 3/4", 12 GPM 1", 22 GPM 1 1/4"
— — —	MAIN LINE 1 1/4" SCH 40 PVC BURIED A MINIMUM OF 18". ALL MAIN LINE FITTINGS UP TO VALVE TO BE SCH 80.
═══	SLEEVES OF GREEN SEWER PIPE AND OR SCH 40 PVC PIPE A MINIMUM OF TWO SIZES LARGER THAN PIPE. PROVIDE SLEEVING AS REQUIRED TO ROUTE PIPING. ALL WIRE TO BE ENCLOSED IN SEPARATE CONDUIT OF SUFFICIENT SIZE. SLEEVE ALL MAIN LINE AND LATERAL LINES UNDER PAVED AREAS.

NETAFIM DRIP IRRIGATION PRODUCTS

NETAFIM CV-.4-18 PRESSURE COMPENSATING DRIP LINE WITH CHECK VALVE SPACED 18" O.C. WITH SCH 40 PVC HEADERS

PRESSURE REGULATING VALVES (PRV) INSTALLED AFTER ZONE VALVE & FILTER
PRV075LF20V42K FOR FLOWS FROM .25 TO 4.4 GPM
PRV075HF45V2K FOR FLOWS FROM 3.5 TO 17.6 GPM
PRV15045K FOR FLOW FROM 7 TO 35 GPM

FILTERS PLACE BEFORE OR AFTER THE ZONE VALVE
¾" FILTER FOR FLOWS UP TO 13 GPM
1" FILTER FOR FLOWS UP TO 26 GPM
1 ½" FILTER FOR FLOWS UP TO 35 GPM
WITH 140 MESH SCREEN

LINE FLUSH VALVE
TLSOV INSTALLED IN ROUND VALVE BOX AND GRAVEL SUMP

IRRIGATION NOTES:
*FOR SHEET LI101 ONLY.

IRRIGATION CONTRACTOR IS RESPONSIBLE FOR COMPLETE COVERAGE. CONTRACTOR TO UNDERSTAND DRIP IRRIGATION INSTALLATION AND SHALL PROVIDE QUALIFIED WORKERS TO INSTALL THE IRRIGATION SYSTEM. IRRIGATION DESIGN IS A DRIP SYSTEM. DRAWINGS ARE SCHEMATIC, LOCATE PIPING AS REQUIRED. PIPING SHOWN AS PARALLEL MAY BE INSTALLED IN COMMON TRENCH. VERIFY WATER CONNECTION LOCATION AND PRESSURE. SYSTEM IS DESIGNED WITH A QUICK COUPLER AT CONNECTION LOCATION FOR COMPRESSED AIR FOR WINTERIZATION.

SLEEVE MAIN AND LATERALS IN GREEN 4" SEWER PIPE OR IN SCH 40 PVC PIPE TWO SIZES LARGER THAN IRRIGATION LINE OR AS REQUIRED FOR MULTIPLE LINES UNDER PAVED AREAS. SLEEVE WIRE IN SEPARATE SCH 40 PIPE SIZED FOR THE NUMBER OF WIRES. SOIL AROUND SLEEVE TO BE COMPACTED TO 98%.

EACH AUTOMATIC CONTROL VALVE SHALL BE PROVIDED WITH A SEPARATE CONTROL WIRE RUNNING FROM THE VALVE TO THE PROPER STATION ON CONTROLLER CONTROLLING VALVE. ALL CONTROL VALVES OPERATED BY ONE CONTROLLER SHALL HAVE COMMON GROUND WIRE RUNNING FROM THE CONTROL VALVES TO CONTROLLER. ALL WIRE TO BE DIRECT BURIAL P.E. AND U.L. APPROVED, RED-LAWN, WHITE-GROUND, BLUE-SHRUBS. RUN ONE EXTRA WIRE TO FARTHEST VALVE BOX AND CLOCK BOTH DIRECTIONS, COLOR YELLOW. ELECTRICAL WIRE TO BE LOCATED ON UNDERSIDE OF MAIN LINE PIPE.

ALL AUTOMATIC VALVES SHALL BE IN A POLY PLASTIC VALVE BOXES WITH LOCKING LIDS. BOXES TO BE OF ADEQUATE SIZE FOR THE AMOUNT OF VALVES. EACH VALVE SHALL BE ACCESSIBLE FOR MANUAL OPERATION AND MAINTENANCE. VALVE BOXES SHALL HAVE A 6" GRAVEL SUMP AT BOTTOM OF VALVES. PROVIDE VALVE BOXES THAT MATCH THE COLOR OF THE GRAVEL AREA THEY WILL BE INSTALLED IN.

IRRIGATION PIPE SHALL BE SCH 40 PVC SOLVENT WELDED FOR MAIN LINE AND LATERAL LINES. MAIN LINES SHALL BE BURIED A MINIMUM OF 18" AND LATERAL LINES A MINIMUM OF 8" AS NOTED ON DETAILS (TO TOP OF PIPE, TYPICAL). COMPACTION OF ALL TRENCHES IS THE IRRIGATION CONTRACTORS'S RESPONSIBILITY.

SUPPLY PRODUCTS AS SPECIFIED, NO SUBSTITUTIONS ALLOWED UNLESS PRE-APPROVED IN WRITING BY OWNER OR LA. A ONE YEAR WRITTEN GUARANTEE SHALL BE SUPPLIED BY IRRIGATION CONTRACTOR TO OWNER ON ALL PARTS AND WORKMANSHIP. TWO SETS OF AS-BUILT DRAWINGS SHALL BE SUPPLIED TO OWNER ALONG WITH WINTER SHUT-DOWN AND SPRING START-UP INSTRUCTIONS. CONTRACTOR IS RESPONSIBLE FOR WINTERIZATION THE FIRST YEAR AND START THE FOLLOWING SPRING. A REDUCED DRAWING OF THE IRRIGATION PLAN SHOWING THE VALVE ZONES SHALL BE MOUNTED IN THE CONTROLLER.

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SHEET NAME:
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REVISIONS

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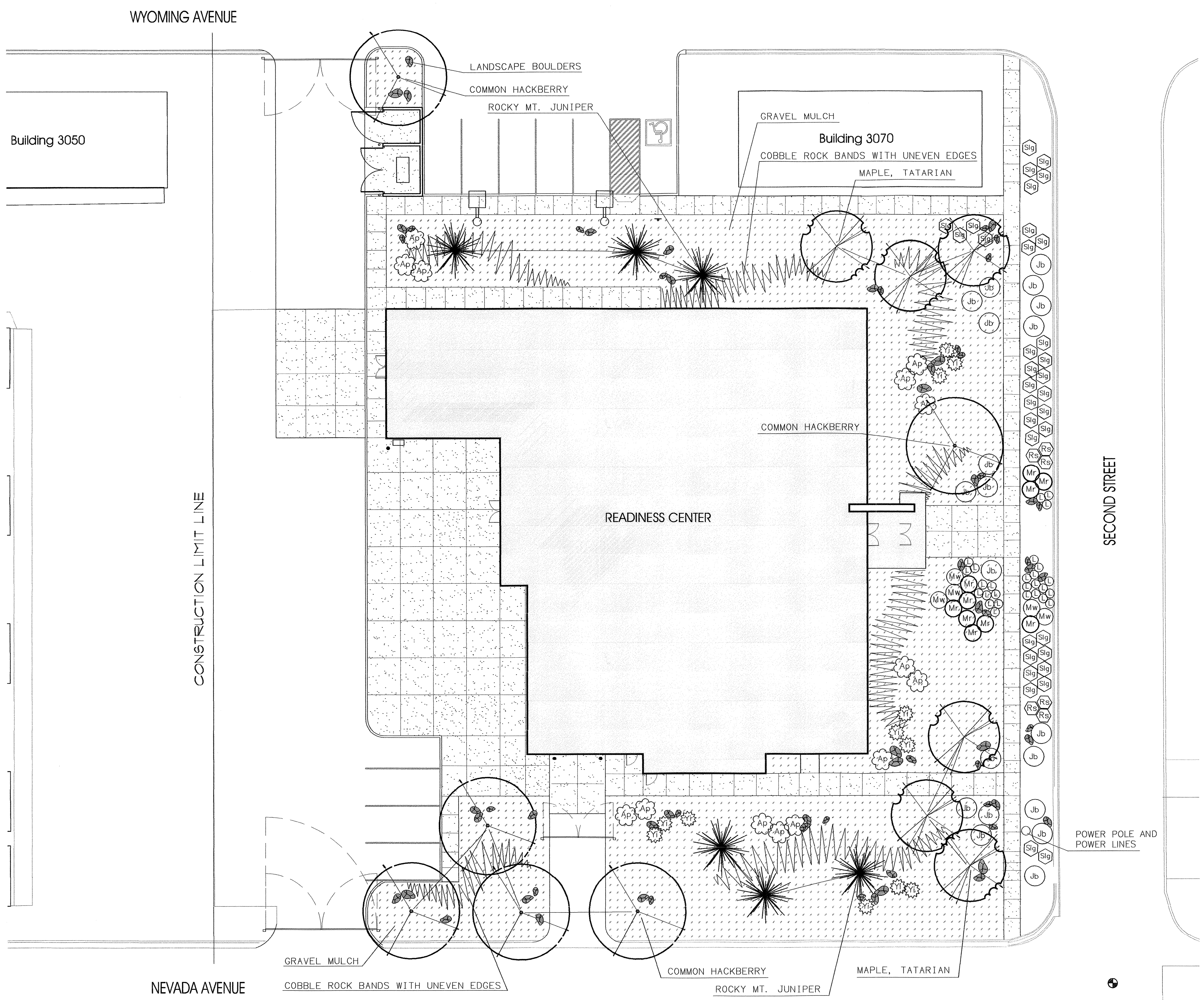

16' 0' 16' 32'
scale = 1" = 16'-0"

LANDSCAPE PLANTING PLAN (A3 / LP101)

GENERAL NOTES AND LEGEND:
*FOR SHEET LP101 ONLY.

- SEE SHEET AE001 FOR GENERAL NOTES.
- SEE COVER SHEET FOR DRAWING INDEX.
- SEE LANDSCAPE IRRIGAITON SHEET LI101 FOR IRRIGATION SYSTEM
- DO NOT SCALE DRAWINGS.

SITE AREA LEGEND:
*FOR SHEET LP101 ONLY.

IMPERVIOUS SURFACE AREA (CONCRETE)

IMPERVIOUS SURFACE AREA (ASPHALT)

½ TO 1" WASHED STAKE/PARSON COMPANIES 'ROUND COBBLE ROCK' MULCH AT 3" DEEP OVER DEWITT PRO 5 WEED BARRIER FABRIC. SHAPE ROUND, COLOR RED, BEIGE, BROWN, ETC. COLOR TO MATCH/COMPLEMENT BUILDING. CONTRACTOR TO SUBMIT SAMPLE AND ADDITIONAL SAMPLES AS REQUIRED TO ARCHITECT AND OR LANDSCAPE ARCHITECT FOR REVIEW AND APPROVAL.

STAKE/PARSON COMPANIES 2 TO 4" WASHED 'ROUND COBBLE ROCK' BANDS OVER THE ROCK MULCH, COBBLE ROCK COLOR THE SAME AS THE ½ TO 1" ROCK MULCH. CONTRACTOR TO SUBMIT SAMPLE AND ADDITIONAL SAMPLES AS REQUIRED TO ARCHITECT AND OR LANDSCAPE ARCHITECT FOR REVIEW AND APPROVAL.

68 LANDSCAPE BOULDERS TO BE THE SAME TYPE AND COLOR AS THE BOULDERS INSTALLED IN THE LANDSCAPE TO THE WEST OF THIS PROJECT. PROVIDE BOULDERS IN THE FOLLOWING SIZES IN FEET 3X2X2, 2.5X2X1.5, 2X2X1.5, 1.5X1.5X1, 1X1X1 EIN EQUAL NUMBERS

STAKER/PARSON COMPANIES
1670 N. BECK STREET
SALT LAKE CITY, UTAH 84116
(801)363-6478
OR APPROVED EQUAL

PLANT LEGEND:
*FOR SHEET LP101 ONLY.

PLANT LIST -- TREES

COMMON NAME	BOTANICAL NAME	SIZE	NUM
COMMON HACKBERRY	CELTIS OCCIDENTALIS	1.5" CAL	6
ROCKY MT. JUNIPER	JUNIPERUS SCOPULORUM	5 GAL	6
MAPLE, TATARIAN	ACER TATARICUM	1.5" CAL	6

PLANT LIST -- SHRUBS

SYM	COMMON NAME	BOTANICAL NAME	SIZE	NUM
Ap	APACHE PLUME	FALLUGIA PARADOXA	5 GAL	14
Jb	JUNIPER, BROADMOOR	JUNIPERUS BROADMOOR	5 GAL	20
L	DAY LILIES, STELLA-DE-OR	HEMEROCALLIS, STELLE-DE	1 GAL	27
Mr	MEIDILAND ROSE RED FUCHSIA	ROSA X MEIDILAND FUCHSIA	2 GAL	10
Mw	MEIDILAND ROSE WHITE	ROSA X MEIDLAND WHITE	2 GAL	5
Rs	RUSSIAN SAGE	PEROVSKIA ATRIPLICIFOLIA	1 GAL	6
Slg	GROW LOW SUMAC	RHUS AROMATICA	2 GAL	34
Yi	YUCCA, IVORY TOWER	YUCCA F. IVORY TOWER	5 GAL	12

VERIFY TOTALS

LANDSCAPE NOTES:
*FOR SHEET LP101 ONLY.

THE NEW BUILDING WILL BE DEVELOPED ON A SITE OF EXISTING BUILDINGS AND PARKING LOTS. EXISTING TOPSOIL MAY BE STRIPPED AND STOCK PILLED FOR LANDSCAPE USE. (VERIFY POTENTIAL OF ON-SITE STRIPPED TOPSOIL FOR LANDSCAPE USE WITH GENERAL CONTRACTOR.) PROVIDE NEW TOPSOIL AS REQUIRED. CONTRACTOR TO TILL SUB-GRADE, HAUL AND PLACE TOPSOIL, PROVIDE SHRUBS, TREES, GRAVEL MULCH, COBBLE ROCK, BOULDERS AND MAINTENANCE, ETC.

SHRUB PLANTING BEDS TO RECEIVE A MINIMUM OF 12 INCHES OF TOPSOIL. DIG SUB-GRADE IN SHRUB BEDS DOWN AS REQUIRED BEFORE PLACING TOPSOIL. THE MAJORITY OF THE SITE THAT IS COVERED WITH GRAVEL MULCH IS NOT REQUIRED TO HAVE QUALITY TOPSOIL INSTALLED. HOWEVER, WHERE THE SHRUBS ARE CLUSTER, PROVIDE TOPSOIL TO THE DEPTH OF 12 INCHES AND EXTENDED OUT PAST THE LAST SHRUB 2 FEET. WHERE INDIVIDUAL SHRUBS ARE ISOLATED FROM PLANTING BEDS PROVIDE A 12 INCH DEEP BY 2 FOOT WIDE PLANTING PIT BACKFILLED WITH ONE HALF TOPSOIL AND SOIL EXCAVATED FROM PLANT PIT MIXED.

SHRUB PITS TO BE EXCAVATED A MINIMUM OF 6 INCHES WIDER THAN BALL OF PLANT, TREES TO HAVE TREE PIT SOIL AT A 5 FOOT DIAMETER LOOSENED TO THE DEPTH OF THE BALL OF THE TREE. THE TREE WILL BE PLANTED IN LOOSENED EXISTING DIRT AND TREE HOLES TWO TIMES THE SIZE OF THE BALL. BACK FILL SHRUBS AND TREES WITH ONE HALF SOIL FROM PLANT PIT AND TOPSOIL MIXED. ALL SHRUBS AND TREES TO RECEIVE AGRIFORM 21 GRAM FERTILIZER TABLETS PLACED WITH PLANTS. ONE TABLET PER 1 GALLON PLANT, TWO TABLETS PER 5 GALLON PLANT, SIX TABLETS PER 15 GALLON TREE, ETC. TREE STAKING AND GUYING REQUIRED TO SUPPORT AND STRAIGHTEN TREES.

ALL SHRUB BEDS TO BE TOP DRESSED WITH 3 INCHES DEEP OF GRAVEL MULCH. FINISH GRADE OF GRAVEL MULCH NEXT TO WALKS AND CURBS TO BE 1 TO 1.5 INCHES BELOW TOP OF CONCRETE.

MAINTENANCE OF THE LANDSCAPE WILL BE THE RESPONSIBILITY OF THE LANDSCAPE CONTRACTOR UNTIL FINAL ACCEPTANCE. THE LANDSCAPE INSTALLATION SHALL BE WEED FREE AND THE SITE SHALL BE CLEANED UP WITH SIDE WALKS AND PARKING LOTS SWEPT AT TIME OF ACCEPTANCE.

ALL PRODUCTS, PLANTS, WORKMANSHIP, ETC., SUBJECT TO OWNERS OR OWNERS AGENT FOR APPROVAL OF QUALITY. GUARANTEE MATERIALS, PLANTS, AND WORKMANSHIP FOR ONE YEAR FROM DATE OF COMPLETION AND ACCEPTANCE OF THE LANDSCAPE. LANDSCAPE CONTRACTOR TO PROVIDE A WRITTEN GUARANTEE TO THE OWNER. ALL REPLACEMENTS OF DEAD OR UNHEALTHY PLANT MATERIAL TO BE MADE WITHIN TWO WEEKS AFTER NOTIFICATION FROM OWNER OR AGENT.

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UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

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LANDSCAPE PLANTING PLAN

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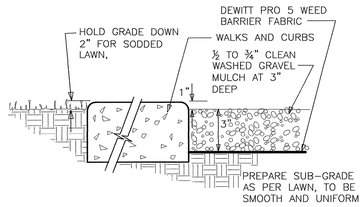
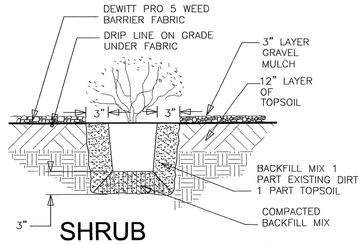


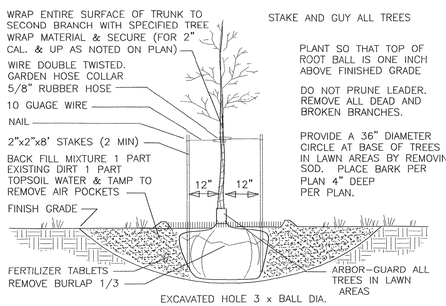
GRAVEL MULCH AND GRADES AT WALKS

D1 NO SCALE



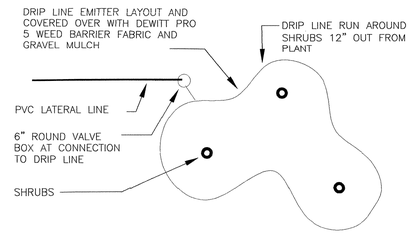
SHRUB PLANTING DETAIL

D2 NO SCALE



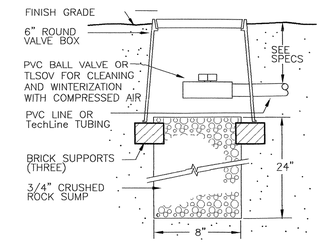
TREE PLANTING & STAKING DETAIL

D3 NO SCALE



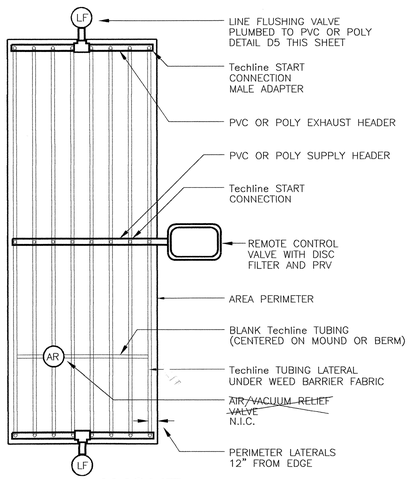
TREE PLANTING DRIP LINE DETAIL

D4 NO SCALE



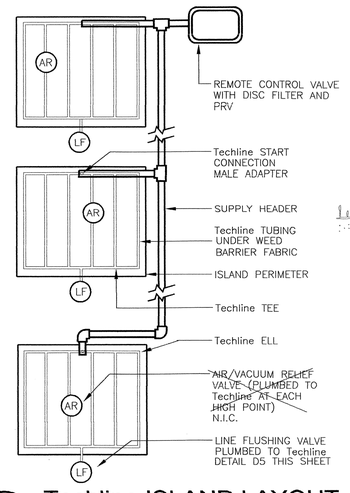
BALL VALVE FLUSHING

D5 NO SCALE



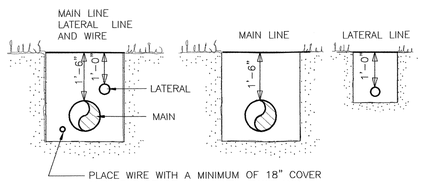
LAYOUT Techline CENTER FEED

C1 NO SCALE



Techline ISLAND LAYOUT

C2 NO SCALE

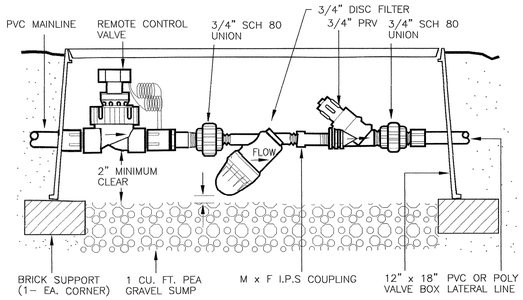


NOTE:

1. SETTING BED AND BACKFILL WITHIN 2" OF PIPE TO BE ROCK FREE (NO ROCKS OVER 1" IN SIZE). CONTRACTOR IS UNABLE TO SEPARATE ROCK FROM BACKFILL THEN CONTRACTOR SHALL PROVIDE A 2" LAYER OF SAND UNDER PIPE AND A MINIMUM OF 2" OF SAND OVER PIPE BEFORE PLACING EXCAVATED BACKFILLL.

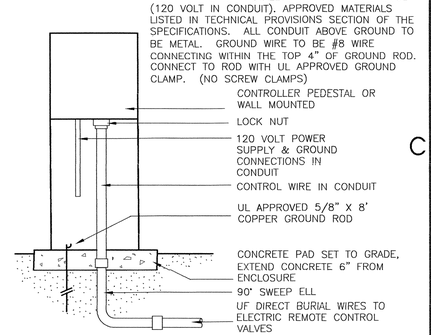
TYPICAL TRENCHING DETAIL

C3 NO SCALE



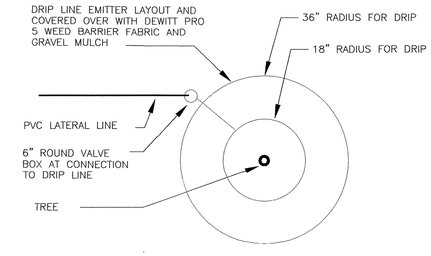
REMOTE CONTROL VALVE W/3/4" PRV & DISC FILTER

C4 NO SCALE



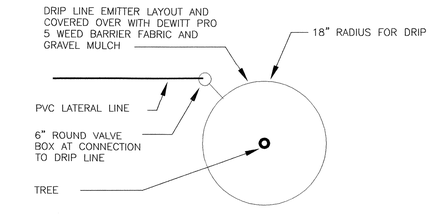
IRRIGATION CONTROLLER

C5 NO SCALE



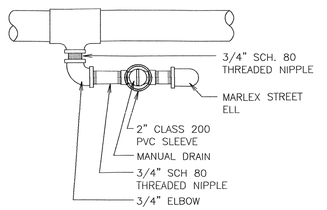
TREE PLANTING DRIP LINE DETAIL

B1 NO SCALE

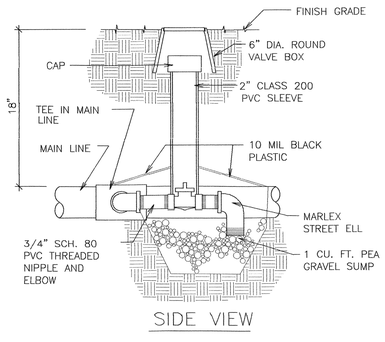


TREE PLANTING DRIP LINE DETAIL

A1 NO SCALE



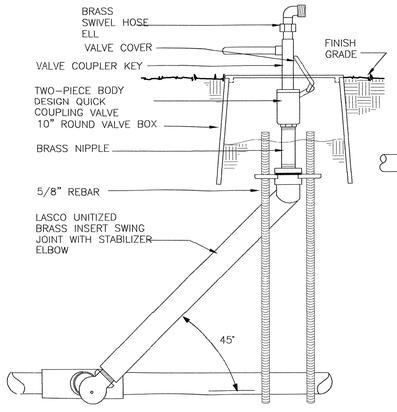
TOP VIEW



SIDE VIEW

MANUAL DRAIN VALVE DETAIL

A2 NO SCALE

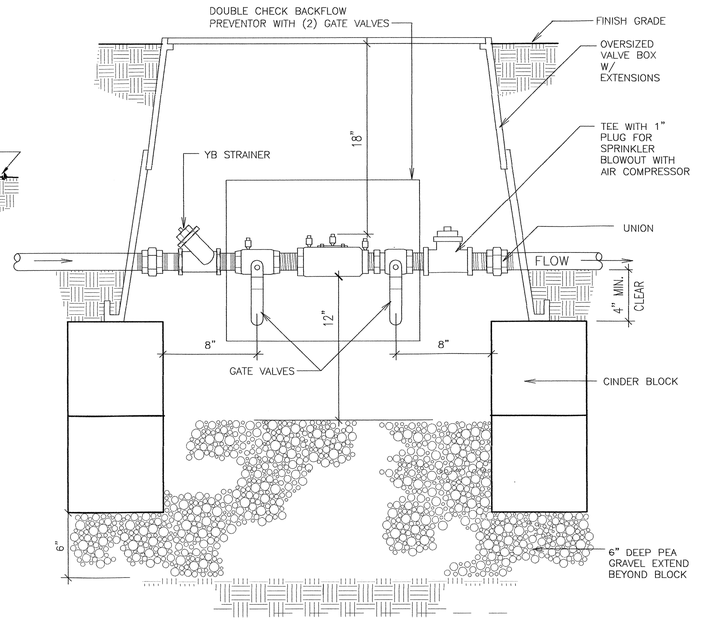


INSTALLATION NOTES:

1. ACME O-RING SEALED THREADS ARE TO BE FINGER TIGHTENED AND "BACKED OFF" ONE FULL ROTATION BEFORE INSTALLATION.
2. THE SWING JOINT LAY ARM IS TO BE INSTALLED AT AN ANGLE BETWEEN 30° AND 45° .

QUICK COUPLING VALVE DETAIL

A3 NO SCALE



DOUBLE CHECK BACKFLOW PREVENTER AND ENCLOSURE

A4 NO SCALE

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CAMP WG WILLIAMS
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SHEET NAME:

LANDSCAPE PLANTING & IRRIGATION DETAILS

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GENERAL STRUCTURAL NOTES

I. Design Criteria

A. Governing Building Code: 2003 International Building Code (IBC)

B. Floor Live Loading:
1. Office: 80 psf Live Load + 20 psf Partition Load
2. Exit Facilities & Corridors: 100 psf Live Load
3. Mechanical Rooms: 125 psf Live Load or actual weights, if larger

C. Roof Live Loading:
1. Roof Live Load: 20 psf
2. Roof Snow Load: 30 psf + Drift per IBC
   a. Ground Snow Load, P_g: 43 psf
   b. Snow Exposure Factor, C_e: 1.0
   c. Importance Factor, I_s: 1.0
   d. Thermal Factor, C_t: 1.0

D. Earthquake:
1. Seismic Use Group: I
2. Seismic Design Category: D
3. Spectral Response Accelerations:
   S_S = 1.196 g S_{DS} = 0.815 g
   S_1 = 0.458 g S_{D1} = 0.471 g
4. Soil Site Class: D
   F_a = 1.02 F_v = 1.54
5. Basic Seismic-Force-Resisting System: Special Reinforced Masonry Shear Walls
   R = 5 C_d = 3.5 Ω_0 = 2.5
6. Importance Factor, IE: 1.0
7. Design Base Shear: 216 kips
8. Analysis Procedure: Equivalent Lateral Force (Static)

E. Wind:
1. Basic Wind Speed (3-second gust): 90 mph
2. Importance Factor, I w: 1.0
3. Exposure: C
4. Internal Pressure Coefficient, GC_{pi}: 0.18

F. Foundation:
1. Subsurface Conditions:
   Soils report and log of borings was obtained by the Owner for the Engineer's use in the design of the foundation, and is not a part of the Contract Documents. This report and log of borings is available for the Contractor's information, but is not a warranty of the subsurface conditions. The Contractor may use the report at his own risk.
2. Soils Report by AMEC Earth & Environmental, dated February 22, 2005.
3. Soil Bearing Pressure: 3000 psf
4. Lateral Soil Pressure Fluid Equivalent Density.
   a. Passive: 300 pcf
5. Coefficient of Friction: 0.40

II. Earthwork

A. Clearing: The entire building area shall be scraped to remove the top 4 inches of soil, including all vegetation and debris.

B. Proof rolling: The natural undisturbed soil below all footings shall be proof rolled prior to placing concrete. Remove all soft spots and replace with compacted structural fill.

C. Compacted structural fill: All fill material shall be a well-graded granular material with a maximum size less than 4 inches and with not more than 18 percent passing a No. 200 sieve. It shall be compacted to 95 percent of the maximum laboratory density as determined by ASTM D1557. All fill shall be tested (See Specifications and the Quality Assurance section of the GSN).

D. Footings may not be placed on possible fills. Remove all possible fills to depth of undisturbed native soil & replace w/ compacted structural fill.

E. Slabs on grade may not be placed on possible fills. Remove and replace with compacted structural fill.

III. Concrete

A. Materials shall comply with the Standards specified in American Concrete Institute (ACI) 318-02, "Building Code Requirements for Structural Concrete."
1. Compressive strengths of concrete at 28 days shall be as follows:
   a. Footings: 3000 psi
   b. Slabs on Grade: 3000 psi
   c. Walls: 4000 psi
   d. Columns: 4000 psi
   e. Suspended Slab (@ vault) 4000 psi
   f. Lightweight concrete over Steel Deck: 3000 psi
   g. All other Site Cast Concrete: 4000 psi
2. Concrete Density (Maximum Air Dry Weight):
   a. Normal weight concrete shall be approximately 145 to 155 pounds per cubic foot.
   b. Lightweight concrete shall not exceed 110 pounds per cubic foot and shall be made of lightweight coarse aggregates and a blend of lightweight and normal weight fines.
3. Reinforcement steel:
   a. ASTM A615 Grade 60, fy = 60,000 psi min. unless noted otherwise.
   b. Reinforcement at concrete moment frames and shear wall jambs shall be ASTM A706 or ASTM A615 Grade 60, with the following properties:
      (1) Actual yield strength based on mill tests shall not exceed 78,000 psi.
      (2) Retest shall not exceed 81,000 psi.
      (3) Ratio of actual ultimate tensile stress to the actual yield strength shall not be less than 1.25.
      (4) Mill tests shall be submitted to the Engineer.
4. Admixtures:
   a. Air-entraining admixtures, comply with ASTM C 260 (when used).
      (1) When air content of a trowel finished floor slab exceeds 3%, there is an increased risk for delaminations and blistering to occur. When this situation is present, the contractor shall pay special attention to the finishing procedures to help minimize such risks. Refer to ACI 302.1R-96 "Guide for Concrete Floor and Slab Construction" for proper finishing guidelines.
   b. Calcium chloride shall not be added to the concrete mix.
5. Only one grade or type of concrete shall be poured on the site at any given time.
6. Plastic coated tie wires and chairs shall be used to support reinforcing bars, tie bars and tendons.

B. Formwork shall comply with ACI Standards Publication 347 and the project specifications. The contractor shall be responsible for the design, detailing, care, placement and removal of the formwork and shores.
1. Precamber forms and screeds with a camber of 1/4" per every 10'-0" of span to compensate for dead load deflection, unless noted otherwise. Post tensioned concrete slabs and beams do not require formwork to be pre-cambered.

C. Concrete cover requirements for deformed bar reinforcing steel shall comply with ACI 318, "Building Code Requirements for Structural Concrete".

1. Cast-in-place Concrete:	Clear Cover
a. Cast against and permanently exposed to earth:	3"
b. Formed concrete exposed to earth or weather:	
#6 thru #18 bars	2"
#5 and smaller bars	1.1/2"
c. Concrete not exposed to weather or in contact with ground:	
Slabs, Walls, Joists; #11 bars and smaller	3/4"
Beams, Columns: Primary Reinf., Ties, Stirrups, Spirals	1.1/2"
d. Concrete Tilt-Up Panels:	
#8 and smaller bars	1"
#9 thru #18 bars	2"
2. Pre-cast Concrete (manufactured under plant controlled conditions):	
Wall Panels #11 bars and smaller	3/4"
Other members #6 thru #11 bars	1.1/2"
Other members #5 bars and smaller	1.1/4"

3. Prestressed Concrete (Provide the following minimum cover for prestressed and non-prestressed reinforcements, ducts, and end fittings.):

a. Cast against and permanently exposed to earth	3"
b. Formed concrete exposed to earth or weather:	
Wall Panels, Slabs, Joists	1"
Other members	1.1/2"
c. Concrete not exposed to weather or in contact with ground:	
Slabs, Walls, Joists	3/4"
Beams, Columns: Primary Reinf.	1.1/2"
Ties, Stirrups, Spirals	1"

D. Construction Joints and Control Joints:
1. Provide a continuous 2 X 4 keyway or a surface intentionally roughened to a full amplitude of approximately ¼" in all wall footings. Adjust the keyway as necessary to provide for proper bar placement. A continuous keyway shall not be used for concrete shear wall to footing connections, unless specifically indicated. Refer to project plans, schedules and details for the shear wall to footing connection requirements.
2. All horizontal and vertical construction joints shall have a continuous 2 X 4 keyway along the joint or joints shall be intentionally roughened to a full amplitude of approximately 1/4", unless noted otherwise.
3. Provide reinforcement dowels to match the member reinforcement across the joint except for shear walls, unless noted otherwise. For dowels across construction joints and wall to footing connections of concrete shear walls, refer to specific project plans, schedules, and details.
4. Construction joints in suspended concrete pours shall be made at the center of spans.
5. Slabs on grade shall have construction or control joints spaced not to exceed 30 times the slab thickness in any direction. All discontinuous control or construction joints shall be reinforced with 2 - #4 x 4'-0". See structural details. Construction joints shall not exceed a distance of 125'-0" o.c. in any direction.
6. Control joints shall be installed in slabs on grade so the length to width ratio of the slab is no more than 1.25:1. Control joints shall be completed within 12 hours of concrete placement. Control joints may be installed by:
   a. Saw cut a depth of 1/4 the thickness of the slab
   b. Tooled joints a depth of 1/4 the thickness of the slab
7. Control joints in visually exposed walls, unless noted otherwise: (Joints shall line up with masonry and architectural joints, see drawings.)
   a. Vertical control joints at 10'-0" on center.
   b. Reinforcing shall be continuous through control and construction joints, unless noted otherwise.
   c. Control joints in concrete foundation walls shall line up with masonry control joints.
8. Control joints shall be installed in suspended slabs over steel decking by sawcutting along all interior grid lines. Joints centered above the purlins shall be 3/4" deep and shall have #4 X 5'-0" at 18" o.c. reinforcing, placed perpendicular (and centered) to the purlin. Joints centered above the girders shall be 3/4" deep and shall have #4 X 10'-0" at 16" o.c. reinforcing placed perpendicular (and centered) to the girder. The #4 bar reinforcing centered above the grid lines is in addition to the specified WWF continuous throughout the suspended slabs over steel decking. Reinforcing shall be placed 1" below the top of the slab.

E. Detailing: All reinforcing, including WWF, shall be detailed, bolstered & supported to comply with ACI 16, "Details and Detailing of Concrete Reinforcement" and the Concrete Reinforcing Steel Institute (CRSI) recommendations. Reinforcing bars shall not be welded unless specifically shown on drawings.
1. Lap splice lengths shall be detailed to comply with the "Reinforcing Bar Lap Splice Schedule" contained within the contract drawings. Splices may be made with mechanical splices capable of 125% tension capacity of the bar being spliced. Mechanical splices shall be the positive connecting type coupler. They shall be covered by a current ICBO Evaluation Report. Use "Cadweld" splice sleeves with ferrous filler, "Lenton" taper threaded rebar splices, "Bar-Lock" lockshear bolt coupling sleeves, or approved equivalent. If mechanical splices are used, splices or couplers on adjacent bars shall be staggered a minimum of 24" apart along the longitudinal axis of the reinforcing bars.
2. All embedments and dowels shall be securely tied to formwork or to adjacent reinforcing prior to the placement of concrete.
3. Use chairs or other support devices recommended by the CRSI to support and tie reinforcement bars and WWF prior to placing concrete. WWF shall be continuously supported at 36" o.c. maximum.
4. Provide corner bars at intersecting wall corners using the same bar size and spacing as the horizontal wall reinforcing. Unless noted otherwise, corner bar lap lengths shall conform with reinforcing bar lap splice lengths as noted above.
5. All vertical reinforcing shall be doweled to footings, or to the structure below. Dowels shall be the same size and at the same spacing as the vertical reinforcing scheduled (or detailed) for the element above. Lap splice lengths shall comply as noted above or as shown in the drawings. Dowels extending into footings shall terminate with a 90 degree standard ACI hook and shall extend to within 4" of the bottom of the footing. Footing dowels (#8 bars and smaller) with hooks need not extend more than 20" into footings.
6. Horizontal wall reinforcing shall terminate at ends of walls and openings into the far end of the jamb column with a 90-degree standard ACI hook, unless shown otherwise. Lap horizontal bar splices as noted above or as shown in the drawings. Horizontal wall reinforcing shall be continuous through construction and control joints. Splices in horizontal reinforcement shall be staggered, so the splice laps will not overlap. Splices in two curtains where used shall not occur in the same location, splice laps shall not overlap.
7. Wall Openings 8" to 36" wide: Place 2- #5 bars (or 1 - #7 bar in 10" walls and thinner) around all openings 8" or larger in any direction, and extend the reinforcing bars a minimum of 24" beyond the corner of the openings, unless noted otherwise. Where 24" is not available, extend bars as far beyond the opening as possible and terminate them with a 90 degree standard ACI hook.
8. Wall Openings 36" wide and wider: Provide reinforced concrete lintels per Concrete Lintel Schedule over the top of, and 2-#5 bars (or 1 - #7 bar in 10" walls and thinner) and on all sides and below every unscheduled opening, unless noted otherwise. Bars for all openings shall extend a minimum of 24" beyond the corners of the opening. Vertical bars shall extend from floor level below to the floor, or roof, level above. Where 24" extension is not possible, extend bars as far beyond the opening as possible and terminate them with a 90 degree standard ACI hook.
9. Provide 2-#5 X 4'-0" diagonal bars (or 1 - #7 x 4'-0" bar in 10" walls and thinner) at the corners of all openings. Diagonal bars shall be centered on the corner of the opening. All recesses in concrete walls that interrupt reinforcing steel shall be reinforced the same as an opening.
10. Contractor shall coordinate placement of all openings, curbs, dowels, sleeves, conduits, bolts, inserts and other embedded items prior to concrete placement.
11. All tied columns shall have ties spaced at one-half the required tie spacing for a distance of one-sixth of the column height above and below all floor (or beam) and roof (or beam) levels or any other point of lateral support, unless noted or detailed otherwise on the structural drawings.
12. Column cross-ties shall have a 135 degree hook at one end and a 90 degree hook at the other. The hooks shall engage the vertical column reinforcement. The 135 degree hooks of consecutive cross-ties engaging the same vertical bars shall engage alternate vertical bars
13. Splices in vertical column reinforcing will be permitted at floor levels only, unless shown otherwise. Where changes in the cross section of the column occur, the longitudinal bars shall be offset in a region where lateral support is afforded. Where offset, the slope of the inclined portion of the bar shall not exceed 1 to 6 (horizontal to vertical). In the case of tied columns, the ties shall be spaced not over three inches on center for a distance of one foot above and one foot below the point of offset.
14. All reinforcement shall be bent cold, and shall be bent only once at the same location. All reinforcement shall be shop bent, unless otherwise permitted by the engineer.

F. Minimum Reinforcing: Wall reinforcing shall be as follows, unless noted otherwise:

Wall Thickness	Horizontal Reinf.	Vertical Reinf.
6"	#4 @ 13" o.c.	#4 @ 18" o.c.
8"	#5 @ 15" o.c.	#4 @ 16" o.c.
10"	#5 @ 12" o.c.	#4 @ 13" o.c.
12"	#4 @ 13" o.c. Each Face	#4 @ 18" o.c. Each Face
Others	0.25% of Wall Area	0.15% of Wall Area

Place steel in the center of the wall (except in walls thicker than 10" and where shown otherwise). Walls thicker than 10" shall have two curtains of reinforcing (placed near each face of the wall), unless otherwise shown on the structural drawings. Spacing shall not exceed three times the wall thickness nor 18". In addition to the above reinforcing, 2 - #5 (or 1 - #7 in 10" walls and thinner) x continuous horizontal bars shall be placed at the bottom of the wall (near the footing) and at each floor level, at the roof level and at the top of wall.

G. No aluminum conduit or product containing aluminum or any other material injurious to concrete shall be embedded in concrete.

H. Unless otherwise noted, all slabs on grade shall be 4" thick.

IV. Masonry

A. Materials, unless noted otherwise:
1. Concrete Masonry Units: Lightweight Grade N, Type 1 (minimum unit strength of 1900 psi) or better. ($f'm$ = 1500 psi)
2. Hollow Clay Units: Hollow Brick, Grade I (minimum unit strength of 6400 psi average or better. ($f'm$ = 2000 psi)
3. Solid Clay Units: Grade SW (minimum unit strength of 4400 psi average or better. ($f'm$ = 1500 psi)
4. Mortar: Use Type "S" according to IBC Section 2103.7, and tested according to ASTM C270. Admixtures shall not be added to the mortar mix. (1500 psi minimum compressive strength).
5. Grout: Conform to IBC Table 2103.10 or ASTM C476. Proportioned according to IBC Section 2103.10 and tested according to ASTM C1019. Grout shall attain a minimum compressive strength of 2000 psi at 28 days.
6. Reinforcing: Grade 60 reinforcing steel shall comply with ASTM A615. Wire joint reinforcing shall comply with ASTM A951.
7. Deformed Bar Anchors (DBA): All DBAs shall comply with ASTM A496.
8. Anchor Bolts (AB): ASTM A307 with ASTM A563 heavy hex nuts and hardened washers, Grade A, unless noted otherwise.
9. Headed Stud Anchors (HSA): Manufacture all HSAs in conformance with ASTM A108 with dimensions complying with AISC specifications.

B. Construction Requirements:
1. Mortar Joints: Joints shall be "concave", "V-joint" or "weathered raked" for structural members unless noted otherwise on architectural drawings.
2. Masonry walls, beams and columns shall be constructed with running bond, unless noted otherwise.
3. Grouting Requirements: Comply with IBC Section 2104 and ACI 530.1/ASCE 6/TMS 602. Grout shall be mechanically consolidated and mechanically reconsolidated according to ACI 530.1/ASCE 6/TMS 602 Section 3.5 E.
4. Reinforcing Bars shall not be welded unless specifically shown on drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars for DBAs or HSAs.
5. Control Joints: Spacing shall not exceed 40'-0". See architectural drawings for locations.
6. Grout all beam and joist pockets solid after installation of beams and joists.
7. Masonry Veneer Attachment and Reinforcing:
   a. To steel stud and wood stud walls: Veneer shall be attached to the studs Dur-O-Wal D/A 213 S seismic veneer anchors or Hohmann & Barnard DW-10 or DW-10HS seismic veneer anchors (or equal) spaced at 16" o.c. Veneer anchors shall be attached to studs with #10 corrosion resistant self drilling screws. Attach the veneer to the anchors with Dur-O-Wal Seismic Steel Pintles or Hohmann & Barnard 3/16" Byna-Tie with Seismiclips (or equal) spaced at a maximum of 16" o.c. in both directions. Anchor ties shall engage to a galvanized No. 9 gauge horizontal joint reinforcement wire in the veneer which shall be continuous and shall be placed at 16" o.c. maximum at the center of the veneer.
   b. To concrete walls: 22 gauge galvanized dovetail slots shall be installed vertically in concrete at 16" o.c. Attach the veneer to dovetail slots with Dur-O-Wal 16 gage seismic dovetail anchor ties or Hohmann & Barnard 3/16" Byna-Tie with Seismiclips (or equal) spaced at a maximum of 16" o.c. in both vertical and horizontal directions. Anchor ties shall engage to a galvanized No. 9 gauge hortzontal joint reinforcement wire in the veneer which shall be continuous and shall be placed at 16" o.c. maximum at the center of the veneer. Dovetail slots and anchor ties shall be galvanized.
   c. To reinforced masonry walls: Veneer shall be attached with tri-rod laddur type reinforcement spaced at a maximum of 16" o.c. vertically consisting of 3 - #9 gauge, galvanized, corrugated, wires. Veneer may also be attached with Dur-O-Wal D/A 360 S Seismic Ladur-Eye spaced at 16" o.c. maximum in both vertical and horizontal directions. . Anchor ties shall engage to a galvanized No. 9 gauge horizontal joint reinforcement wire in the veneer which shall be continuous and shall be placed at 16" o.c. maximum at the center of the veneer. Anchors shall extend to the galvanized ladur type (2 - #9 wires) joint reinforcing in the masonry wall spaced at 16" o.c. maximum.
   d. Other methods of attachment may be used after written acceptance by the architect and structural engineer.
   e. Steel Lintels: Provide steel angle lintels at all openings through the masonry veneer. Provide one inch of bearing for each foot of width of opening, with a minimum bearing of six inches. See the Steel Angle Lintel Schedule for size.

C. Detailing Requirements:
1. Standards: Reinforcing detailing shall comply with American Concrete Institute (ACI) Standard 315, "Details and Detailing of Concrete Reinforcement."
2. Reinforcement Protection (cover):
   a. Joint reinforcement shall have not less than 5/8" mortar coverage from the exposed face.
   b. Other reinforcement shall have a minimum coverage of one bar diameter over all the bars, but not less than 3/4". When masonry is exposed to soil, minimum coverage shall be 1.5".
3. Vertical steel reinforcement shall be placed and secured against displacement prior to grouting by wire positioners or other suitable devices: at intervals not exceeding 112 bar diameters, at the grout lift heights, or at bar splice locations, whichever is less. Vertical reinforcing shall be located at the center of the wall, unless noted otherwise.
4. Lap Splice Lengths: Lap all masonry reinforcing bars per the "Masonry Reinforcing Bar Lap Splice Schedule." Joint reinforcement shall lap a minimum of 6".
5. Corner Bars: Horizontal reinforcement shall be continuous at all corners and at intersecting walls. Provide corner bars with the required lap splice length.
6. Dowels: All vertical reinforcing shall be doweled to the foundation wall, footing (structure below) and to the structure above with the same size dowel, spacing (and in the same core) as the vertical wall reinforcing unless noted otherwise.
7. Wall Openings 24" wide and wider: Provide reinforced masonry lintels per Masonry Lintel Schedule over the top of, and 2 - #5 bars, in grouted spaces, on all sides and adjacent to every unscheduled opening, unless noted otherwise. Bars for all openings shall extend a minimum of 48 bar diameters beyond the corners of the opening. Vertical bars shall extend from floor level below to the floor , or roof, level above. Where a 48 bar diameter extension is not possible, extend bars as far beyond the opening as possible and terminate them with a 90 degree standard ACI hook.
8. Horizontal wall reinforcing shall be continuous through joining concrete walls, masonry walls, columns, and pilasters. Provide a key between the wall and the column or pilaster. Horizontal wall reinforcing shall be placed inside the column vertical reinforcing.
9. Anchor bolts and headed stud anchors shall be set in a grouted cell. Anchor bolts and headed stud anchors shall have 1" grout surrounding the shank at its penetration. Grout shall be flush with the face or top of the masonry.
10. All masonry column ties shall terminate with 135 degree hooks plus a 6 bar diameter extension (4" minimum).
11. The exposed face of all embed plates shall be set flush with the face of masonry wall or column.

D. Minimum Reinforcing:
All masonry walls shall be reinforced as follows, unless shown otherwise on the drawings. Reinforcing shall be placed in grouted cells.

Wall Thickness	Horizontal Reinf.	Vertical Reinf.
6"	#4 @ 48" o.c.	#5 @ 32" o.c.
8"	#5 @ 48" o.c.	#5 @ 32" o.c.
10"	#6 @ 48" o.c.	#6 @ 32" o.c.
12"	2 - #5 @ 48" o.c.	#6 @ 32" o.c.

V. Structural Steel

A. Material:
1. W-Shapes: ASTM A992, (Fy = 50 ksi), except as noted otherwise.
2. All Other Shapes and Plates: ASTM A36 (Fy = 36 ksi), except as noted otherwise.
3. Rectangular and Square Hollow Structural Sections (HSS): ASTM A500, Grade B (Fy = 46 ksi).
4. Deformed Bar Anchors (DBA): ASTM A496.
5. Headed Stud Anchors (HSA): ASTM A108, with dimensions complying with AISC specifications.
6. Anchor Rods: ASTM F1554, Grade 36 with ASTM A563 heavy hex nuts and ASTM F436 hardened washers, unless noted otherwise.
7. All steel shapes and plates which are a part of the seismic force resisting system shall have Charpy V-Notch absorbed energy of 20 ft-lbs. or greater at 70 degrees Fahrenheit, as indicated below:
   a. Shapes: Group 3 (with flanges 1.1/2" or thicker), Groups 4 and 5.
   b. Plates: 2" and thicker.
   c. Test Frequency: heat.

B. Fabrication and construction shall comply with the following Codes and Standards:
1. American Institute of Steel Construction (AISC) 335-89, "Specification for Structural Steel Buildings - Allowable Stress Design and Plastic Design," including Supplement No. 1, 2001.
2. AISC LRFD 1999, "Load and Resistance Factor Design Specification for Structural Steel Buildings."
3. AISC 341-02, "Seismic Provisions for Structural Steel Buildings," dated May 21, 2002.
4. AISC HSS 2000, "Load and Resistance Factor Design Specification for Steel Hollow Structural Sections."
5. AISC 2000, "Code of Standard Practice" excluding the following: Section 3.3 (last sentence), Section 3.4, Section 4.4, Section 4.4.1, Section 4.4.2, Section 4.5, and Section 7.13.3.
   a. The architectural drawings are the prime contract drawings. Consultants' drawings by other disciplines are supplementary to the architectural drawings. The structural drawings shall be used in conjunction with the architectural drawings. Detailing and shop drawing production for structural elements will require information (including dimensions) contained in architectural, structural, and/or other consultants' drawings. Refer to VII. Special Instructions, notes VII.B and VII.C on this sheet.
6. AISC/RCSC 2000, "Specification for Structural Joints Using ASTM A325 or A490 Bolts"
7. American Welding Society (AWS) D1.1-00, "Structural Welding Code - Steel" (specific items do not apply when they conflict with the AISC requirements).
8. Steel Joist Institute (SJI) 1994, "Standard Specification, Load Tables and Weight Tables for Steel Joists and Joist Girders," and "Recommended Code of Standard Practice."
9. American Iron and Steel Institute (AISI) 20011996, "North American Specification for the Design of Cold-Formed Steel Structural Members."
10. Federal Emergency Management Agency (FEMA) 350, July 2000, "Recommended Seismic Design Criteria for New Steel Moment-Frame Buildings."
11. FEMA 353, July 2000, "Recommended Specifications and Quality Assurance Guidelines for Steel Moment-Frame Construction for Seismic Applications."

C. Structural shapes and plates shall be fabricated from newly rolled (milled) one-piece sections without splices, unless specifically noted otherwise on the structural drawings. Connections for structural steel shall comply with the structural drawings, unless written approval is given by the structural engineer.

D. Welding:
1. Certification of Welders: All shop and field welding shall be executed by AWS certified welders who have been specifically certified for the type of work to be performed. Certification shall be considered current if dated within the past 12 months. Welders will be considered certified if they have been certified under AWS and their work records are current within every six-month period thereafter as required by AWS. Certification and records must comply with AWS Standards. Certification and appropriate records must be provided to the architect prior to beginning work.
2. Electrodes: E-70 XX or as noted otherwise. E60 XX may be used for welding steel floor and roof decks.
3. Minimum Welds: All intersecting steel shapes that are not bolted shall be connected by a fillet weld all around, unless noted otherwise. Fillet weld sizes that are not shown shall be 1/16" less than the thinnest of the connected parts for thicknesses 1/4" and larger. Fillet welds on plates less than 1/4" shall be of the same size as the thinnest of the connected parts.
4. Reinforcing Bars: Do not weld rebar except as specifically detailed in the drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars for deformed bar anchors (DBAs), machine bolts, or headed stud anchors (HSAs).
5. Bolts: Do not apply any welds, including "tack" welds to bolts, including anchor bolts, except as specifically detailed in the drawings.
6. It is recommended the steel erection contractor and steel fabricator contact the Quality Assurance Agency prior to beginning any of the above welds. A program of joint preparation and welding procedures should be worked out between the two parties before the welding is started so that correct welds will be made from the beginning.
7. Headed Stud Anchor (HSA) welding and Deformed Bar Anchor (DBA) welding shall conform to the manufacturer's specifications. Welding shall comply with AWS D1.1 Section 7.6 through 7.8 and Annex IX.
8. Special Provisions for full penetration welds used in moment frames: Welding methods, procedures and quality control shall comply with AWS D1.1 and the following.
   a. Tack weld quality shall comply with Section 5.18.
   b. Arc Strikes, gouges and other imperfections within or adjacent to the joint, shall be repaired or removed.
   c. Preheat, and interpass requirements as outlined in Section 3.5.
   d. Use weld tabs at beam flange connections; after welding, remove the weld tabs and finish to a smooth contour per Section 5.31.3.
   e. Backer bars shall be removed from the beam bottom flange connections to columns. The root of the weld shall be back gouged to sound metal to remove all slag and cracks. Weld the back gouged region and finish welding using a reinforcing fillet weld, according to Section 5.10.4.
   f. Backer bars need not be removed from the beam top flange connections to columns provided that the backer bars are welded to the column flange with a continuous fillet weld for the entire length of the backer bar.
   g. Unrepaired cracks, gouges, grooves and notches will not be permitted in the joint area.
   h. Use electrodes with Charpy V-notch absorbed energy equal to or greater than 20 ft-lbs at 0 degrees Fahrenheit under AWS A5 classification test methods, and 40 ft-lbs at 70 degrees Fahrenheit using test procedures prescribed in Appendix A of FEMA 353. Acceptable electrodes include E70TG-K2, E71T-8, and E71T-1.
9. Braced Frame welding: lengths shown for fillet welds for brace-to-gusset, gusset-to-baseplate, and column-to-gusset connections are minimums, intended for establishing gusset plate dimensions. Weld entire contact length at these joints, typical.

E. Bolted Connections:
1. Use ASTM A325N bolts for steel to steel connections, as noted herein or as noted on the drawings. A325N bolts shall be used in connections for simple span framing and beam (or girder) to bearing plate connections. Tighten bolts to a snug tight condition. A snug tight condition is usually attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. Bolts shall be tightened until all plies of the joint are in firm contact.
2. Use ASTM A325N bolts for all steel to steel connections, unless noted otherwise. Tighten bolts by the turn of the nut, calibrated wrench, or direct tension indicator method. Alternate fastener designs as defined by AISC shall be submitted to the engineer for review and acceptability prior to installation. Provide hardened washers beneath turned element.
3. Use ASTM A325X bolts for all steel to steel connections, unless noted otherwise. Tighten bolts by the turn of the nut, calibrated wrench, or direct tension indicator method. Alternate fastener designs as defined by AISC shall be submitted to the engineer for review and acceptability prior to installation. Provide hardened washers beneath turned element.
4. Use ASTM A325SC bolts for all steel to steel connections in moment frames and connections subject to vibrations, unless noted otherwise. Tighten bolts by the turn of the nut, calibrated wrench, or direct tension indicator method. Alternate fastener designs as defined by AISC shall be submitted to the engineer for review and acceptability prior to installation.
5. Provide hardened washers beneath the turned element of all bolts or nuts. Provide hardened beveled washers, to compensate for the lack of parallelism, where the outer face of the bolted parts has a slope greater than one in twenty with respect to the plane normal to the bolt axis. Hardened washers or plates installed over oversized holes or slotted holes shall be at least 5/16" thick and shall conform to ASTM F436. Plates or bars installed at slotted holes shall have a size sufficient to completely cover the slot after installation.
6. Where a steel to steel beam connection is not detailed in the drawings, provide a standard AISC framed connection with the capacity to support one half of the total uniform load capacity of the given shape for the span and for the steel specified.
7. Bolts, nuts and washers shall not be reused.

F. Steel Lintels:
Provide steel angle lintels at all openings through the masonry veneer. Provide one inch of bearing for each foot of width of opening, with a minimum bearing of six inches. See the Steel Angle Lintel Schedule for size.

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V. Structural Steel cont.

G. Beam Web Stiffener Plates:
Provide full-height web stiffener plates to each side of all beams above all bearing points. Stiffener plates shall be the thickness noted below unless noted otherwise and shall be welded on both sides of the stiffener plate with fillet welds (noted below) all around.

Beam Web stiffener thickness	For beams with flange widths between	Weld Size
1/4 inch thick	Greater than 0" and less than 8 1/4"	3/16"
3/8 inch thick	Greater than 8 1/4" and less than 12 1/4"	1/4"
1/2 inch thick	Greater than 12 1/4" and less than 16 1/2"	5/16"
5/8 inch thick	Greater than 16 1/2" and less than 20 3/4"	3/8"

H. Open Web Steel Joists and Girders:
1. The steel joist supplier shall be responsible for the design of all open web steel joists and girders. Joists or girders with slopes greater than 1/2 inch per foot shall be designed to meet or exceed the load capacities, listed in the SJI load tables, of the joist or girder sizes indicated on the framing plan, as if the joists or girders were installed level.
2. Open web joists and girders shall be designed with deflection limits of L/240 for total load and L/360 for live load, where L is the joist span.
3. Where uplift loads due to wind are indicated, rows of bottom chord bridging shall be provided at the first bottom chord panel points per SJI Specifications. This is in addition to the bridging shown on the framing plans.
4. Provide special bearing ends to accommodate slopes from sloped joists, sloped girders or sloped bearing conditions.
5. Camber: All joists shall be cambered as specified in the SJI specifications, unless noted otherwise.
6. Field Modifications: Do not modify any joist or girder, including holes through the top and bottom chords, without the written consent and direction from the manufacturer.
7. Shop Drawings and Design Calculations: Shop drawings for all joists and girders used in the project shall be submitted for review. Prior to the fabrication of joists and girders, the open web steel joist and girder manufacturer shall prepare complete joist and girder calculations under the direct supervision of a professional engineer licensed in the State of Utah. Calculations shall be submitted for review for joists and girders designated as SPECIAL or SP and for all joists or girders with axial loads or additional concentrated loads (as noted on the drawings). Submitted calculations shall bear the seal of a professional engineer licensed in the State of Utah.
8. Stabilizer Plates: Provide steel joist and joist girder stabilizer plates as indicated. Stabilizer plates shall be 6" x 6" with a 13/16" diameter hole with 1.1/2" minimum edge distance, and shall extend 3" minimum below the bottom chord of the joist or girder. Plate thickness shall be equal to the chord gap minus 1/4", or 3/4", whichever is less.
9. Verify size, weight, location and configuration of all roof top equipment with architect and mechanical engineer. Provide steel frames for support of roof top equipment as indicated on structural details in the structural drawings. Coordinate openings with the mechanical and general contractor.
10. All concentrated loads greater than 100 pounds supported by open web steel joists and girders shall be located within 6 inches of joist or girder panel points or the joist or girder shall be reinforced with an additional web member. Refer to the "TYPICAL DETAIL AT ADDITIONAL CONCENTRATED POINT LOAD" in the structural drawings.
11. Concentrated point loads, single or multiple, totaling 100 pounds or less can be located at any point along the top or bottom chord of an open web steel joist or girder between adjacent panel points without meeting the requirements above. A limit of four concentrated 100# maximum point loads per joist or girder will be permitted on spans of 12' or greater, one concentrated 100# max. load on spans less than 12', unless specifically noted otherwise on the structural drawings.
12. Joist bridging shall never be used to support hanging loads.
13. Bracing of miscellaneous items (mechanical, electrical, plumbing, etc.) to the bottom chord of joists or girders will not be allowed in any instance. All lateral braces must connect to the top flange/top chord of the framing member above unless noted otherwise on the structural drawings.

I. Steel Floor Deck:
1. Steel floor deck shall comply with the latest requirements of the Steel Deck Institute (SDI). Submit ICBO report with load and lateral shear capacities with shop drawings.
2. Steel floor deck shall be 2" deep X 20 gauge minimum phosphatized/painted composite (deck shall be galvanized (G60) when used above or below mechanical equipment rooms), type "W" deck with interlocking side seams with the following properties:

	20 Gauge	18 Gauge
Minimum S (in3/ft) =	0.361	0.510
Minimum I (in4/ft) =	0.423	0.555

Painted deck shall be coated with special paint to receive sprayed-on fire proofing, where required.
3. A 3 1/2" thick (5 1/2" overall) lightweight concrete (f'c = 3,000 psi @ 28 days unless noted otherwise) slab shall be poured over the steel deck. Reinforce slab with 6" x 6" - W2.1/W2.1 welded wire fabric (minimum) or reinforce slab with 2 pounds per cubic yard minimum polypropylene fibrillated fiber reinforcement, as per specifications. Welded Wire Fabric shall be placed 1" to 1 1/2" below the top of the slab.
4. Steel deck with 3 1/2" thick (5 1/2" overall) lightweight concrete slab shall have a minimum diaphragm shear capacity of 1695 lbs/ft. for a 8'-0" deck span.
5. Weld deck to supporting framing members with 3/4" diameter puddle welds at the following spacing (Closer spacings may be used to develop minimum shear requirements.):
a. 12" o.c. to supports perpendicular to deck corrugations (4 welds per 36" wide sheet).
b. 12" o.c. to all supports parallel to deck corrugations.
c. All welded surfaces shall be dry before welding deck to supports.
6. Attach interlocking seams with 3/16" button punch at 18" o.c. or 1 1/2" top seam weld at 36" o.c. between adjacent pieces of deck. Crimp seams before button punching or welding interlocking seams. Closer spacings may be used to develop minimum shear requirements.
7. All deck shall be 3-span continuous minimum where possible. In areas where 3-span conditions are not possible, the deck shall meet the above loading criteria for the span condition. The contractor shall provide heavier gauge deck or provide shoring as required for one or two span conditions to meet the equivalent loading of the above deck under a three span condition.
8. Deck shall have a minimum bearing length of 2".

K. Steel Roof Deck:
1. Steel roof deck shall comply with the latest requirements of the Steel Deck Institute (SDI). Submit ICBO report with load and lateral shear capacities with shop drawings.
2. Steel roof deck shall be 1 1/2" deep X 20 Gauge minimum galvanized (G60), type "B" wide rib deck with interlocking side seams with the following properties:

	20 Gauge	18 Gauge
Minimum S (in3/ft)=	0.234	0.322
Minimum I (in4/ft)=	0.213	0.296

Painted roof deck shall be coated with special paint to receive sprayed-on fire proofing, where required.
3. Minimum allowable deck diaphragm shear values shall be 400 lb/ft. for a 9'-0" deck span.
4. Weld steel roof deck to supporting framing members with 3/4" diameter puddle welds at the following spacings (Closer spacings may be used to develop minimum shear requirements.):
a. 6" o.c. to all supports perpendicular to deck corrugations (7 welds per 36" sheet).
b. 6" o.c. to the following supports parallel to deck corrugations: All perimeter supports, all concrete or masonry walls, all joists or joist girders with a "T/C" designation, and all steel beams which are on a grid line with braced frames or moment frames.
c. 12" o.c. to all other supports parallel to deck corrugations.
d. All welded surfaces shall be dry before welding deck to supports.
5. Attach interlocking seams with 1 1/2" long top seam welds at 12" o.c. minimum between adjacent pieces of decking. Crimp side seams before welding. Closer spacings may be used to develop minimum shear requirements.
6. Alternate means of deck attachment and fastening using screws, pins or other mechanical fastening methods are permitted with approval of the engineer. The contractor shall submit the proposed attachment system and the code evaluation report demonstrating the system has the strength to meet the required deck shears specified above (in item 3) and on the plans. The flexibility factor of the proposed deck attachment system shall be equal to or less than the flexibility factor for the specified welded deck attachment system. If the alternate attachment method is approved, it is the responsibility of the contractor to ensure that the deck type and profile is compatible with the fastening system being used.
7. Provide a 2" minimum bearing and a 4-inch lap at the splice points of all pieces of deck.
8. All deck shall be 3-span continuous minimum where possible. The contractor shall provide heavier gauge deck as required for one or two span conditions to meet equivalent loading of the above deck under a three span condition.
9. Steel roof deck shall not be used to support loads from plumbing, HVAC ducts, light fixtures, architectural elements or equipment of any kind, unless specifically noted otherwise. Lightweight suspended acoustical ceilings with a total weight per wire not exceeding 50# may be hung from the steel roof deck. The hangers should be staggered to distribute the load over multiple deck flutes.

L. Cold-Formed Steel
1. Light Gauge Steel Framing:
a. Where steel framing size designators are used in the drawings, they follow the convention established by the Steel Stud Manufacturers' Association (SSMA) and the North American Steel Framing Alliance (NASFA). Framing members provided shall comply with the designations according to this convention.
b. All load bearing stud (and/or) joist framing members along with all runner, bridging, and end track shall be of the designation shown on the plans. All studs with base metal thickness of 54 mil and 68 mil, and joists with base metal thickness of 54 mil, 68 mil and 97 mil, shall be formed from steel meeting the requirements of ASTM A570 except that the steel shall have a 50,000 psi yield stress. All track and end closures with base metal thickness of 54 mil and 68 mil, bridging with base metal thickness of 54 mil, and studs and track with base metal thickness of 43 mil and 33 mil, shall be formed from steel with a minimum yield of 33,000 psi. All components shall be galvanized.
c. Follow all manufacturers' recommendations for the use of these products.
d. Unless noted otherwise, all welded connections shall be done using 1/8" AWS type 6013 or 7014 rod with a welding heat of 60-110 amperes depending on the gauge of material and the fit of the parts. Wire tying of framing components shall not be permitted.
e. All interior non-bearing steel stud walls that extend above the ceiling but do not attach to the floor or roof diaphragm (above) shall have diagonal braces at 45 degrees (+/-). The KL/r ratio of the brace shall not exceed 200 and shall not be spaced further apart than 10'-0". Connect diagonal braces to the top of the steel stud walls and to the underside of the top flange of the steel beams, open web joists or girders, etc. with 1/8" fillet welds all around. Diagonal angle braces may be connected to 8" x 8" x 1/4" steel plates which shall be anchored to the floor or roof decks which have concrete fill above with 2- 3/8" dia. expansion anchors for each plate. Connect angle to plate with a 1/4" fillet weld all around. Connect diagonal braces to roof decking which does not have concrete fill above per the non-bearing wall brace connection details shown on the structural drawings. Diagonal braces may be constructed from cold-formed light gauge steel studs but must conform to the kl/r ratio of less than 200 requirement. When diagonal brace lengths exceed 10'-0" (+/-), cold-formed box sections made from two 600S137-54 steel studs will likely be required.
2. Prefabricated Systems: Submit complete shop drawings and calculations of all elements for review. Submittals shall bear the stamp of a Professional Engineer registered in the State of Utah.

VII. Special Instructions

A. The project specifications are not superseded by the General Structural Notes but are intended to be complementary to them. Consult the specifications for additional requirements in each section. Notes and specific details on the drawings shall take precedence over General Structural Notes and typical details.

B. The architectural drawings are the prime contract drawings. Consultant drawings by other disciplines are supplementary to the architectural drawings. All omissions or conflicts, including dimensions, between the various elements of the consultants' drawings and/or specifications shall be brought to the attention of the Architect before proceeding with any work involved. In case of conflict, follow the most stringent requirement as directed by the Architect without additional cost to the owner. Any work done by the contractor after discovery of such discrepancy shall be done at the contractor's risk.

C. The structural drawings shall be used in conjunction with the architectural drawings. Primary structural elements and overall structural layout are indicated within the structural plans and details. Some secondary elements, architectural layouts, alcoves, elevations, slopes, depressions, curbs, mechanical equipment and electrical equipment, are not indicated within the structural drawings. Detailing and shop drawing production for structural elements will require information (including dimensions) contained in the architectural, structural and/or other consultants' drawings.

D. Shoring and Bracing Requirements:
1. Floor and Roof Structures – The General Contractor is responsible for the method and sequence of all structural erection. He shall provide temporary shoring and bracing as his method of erection requires to provide adequate vertical and lateral support. Shoring and bracing shall remain in place as the chosen method requires until all permanent members are in place and all final connections are completed, including all roof and floor attachments. The building shall not be considered stable until all connections are complete.
2. Foundation walls must be braced until the complete floor or roof systems is completed. Do not backfill until floor or roof systems are in place.
3. Walls above grade shall be braced until the structural system is complete. Walls shall not be considered to be self supporting.

E. Submittals: A copy of all shop drawings that have been submitted for review must be kept at the construction site for reference. These drawings must bear the appropriate review stamps. The shop drawing review shall not relieve the contractor of the responsibility of completing the project according to the contract documents. The general contractor shall review and mark all shop drawings prior to submitting them to the Architect for his review. Shop Drawings made from reproductions of (these) contract drawings will be rejected.

F. Project Coordination: It shall be the responsibility of the general contractor to coordinate with all trades any and all items that are to be integrated into the structural system. Openings or penetrations through, or attachments to the structural system that are not indicated on these drawings shall be the responsibility of the general contractor and shall be coordinated with the Architect/Engineers. The order of construction is the responsibility of the general contractor. It is the contractor's obligation to provide all items necessary for his chosen procedure.

G. Contractor shall field verify all dimensions, and conditions. If the contract drawings do not represent actual conditions, contractor shall notify architect/engineer prior to fabrication or construction within that area.

H. Notice of Copyright: The structural drawings, plans, schedules, notes and details are hereby copyrighted by Reaveley Engineers and Associates, Inc., All Rights reserved. Submission or distribution of documents to meet official regulatory requirements or for similar purposes in connection with the project is not to be construed as publication in derogation of Reaveley Engineers & Associates, Inc.'s reserved rights. The documents defining the structure are instruments of service prepared by Reaveley Engineers and Associates, Inc. for one use only. Furthermore, these documents shall not be reproduced, or copied, in whole or in part by the contractor or his subcontractors for preparation of shop drawings or other submittals.

VIII. Quality Assurance

A. Quality Assurance Agency Requirements:
1. The owner shall engage a qualified Quality Assurance Agency (QAA) to provide all special inspection and quality assurance testing for the project. All quality assurance personnel assigned to the project shall demonstrate competence, to the satisfaction of the building official, for inspection of the particular type of construction or operation requiring special inspection.
2. Prior to construction, the QAA shall prepare a written Quality Assurance Implementation Plan (QAIP) for the project. The QAIP shall include a list of personnel assigned to the project including management personnel, inspection procedures and frequency, proposed testing methods and frequency of testing, and reporting procedures. The QAIP shall also outline methods of documenting deficiencies and reporting corrections. A copy of the QAIP shall be given to the contractor for review and coordination with subcontractors.
3. Prior to construction, the QAA shall submit the following information to the Architect and Engineer of Record for approval:
a. A copy of the Quality Assurance Implementation Plan for the project.
b. A copy of the appropriate certification and training records for each individual performing inspections or testing.
c. A list of the testing equipment designated for the project and recent calibration records for the equipment.
d. Sample inspection and testing reports and the distribution list for the reports.
4. The special inspector shall inspect the work per Chapter 17 of the IBC for conformance with the contract documents. The special inspector shall send reports to the owner, building official, architect, engineer, and contractor. All discrepancies shall be brought to the immediate attention of the contractor for correction. The QAA shall submit a final signed report stating that the special inspection work was, to the best of their knowledge, in conformance with the plans, specifications and applicable workmanship provisions of the IBC.

B. Seismic Force Resisting Systems
1. Elements that are a part of the Main Seismic Force Resisting System for the structure may require increased quality assurance inspection and testing. The Main Seismic Force Resisting system for the structure includes the following elements:
a. Masonry walls.
b. Footings and foundation systems that directly support walls.
c. Roof decking.
d. Floor decking and/or slab systems.
e. All elements labeled as "drag struts" or "chords."
f. Connections between the elements referenced above.

C. Special Inspection: Special Inspection shall be provided for the following elements per IBC sections 1704 and 1707:
1. Concrete and elements embedded in concrete shall be special inspected prior to and during placement of concrete. Special inspection of concrete shall include the following:
a. Reinforcing steel size and placement.
b. Surface preparation at cold joints including placement of keyways.
c. Bolt and embed size, configuration and placement.
d. Concrete shall receive continuous special inspection during placement, and periodic inspection after placement to ensure proper curing and weather protection procedures.
2. Structural steel fabrication and erection shall be special inspected, including the following:
a. Fillet welds smaller than 5/16" per AWS D1.1.
b. Fillet welds larger than 5/16", multiple pass welds, and all groove welds shall receive continuous special inspection during weld placement per AWS D1.1.
c. Welding of Headed Stud Anchors (HSA) and Deformed Bar Anchors (DBA) shall be inspected to comply with AWS D1.1 Section 7.6 through 7.8 and Annex IX.
d. Welding or fastening of floor or roof deck per AWS D1.3. or per the code evaluation report for the fastening method.
3. Structural Masonry: Special inspection shall be provided, as follows:
a. As masonry construction begins, and periodically during construction, verify the following are in compliance:
(1) Proportions of site-mixed mortar and grout.
(2) Placement of masonry units and construction of mortar joints.
(3) Locations of reinforcement, connectors and embeds.
(4) Protection of masonry during cold or hot weather.
b. Prior to grouting, verify the following are in compliance:
(1) Grout space.
(2) Size and location of structural elements.
(3) Grade, size and placement of reinforcement, connectors and embeds.
(4) Construction of all mortar joints.
c. Continuous inspection is required during placement of grout and during preparation of grout specimens, mortar specimens, and/or prisms for testing.
4. Post-installed anchors, including but not limited to expansion anchors, adhesive anchors and rebar dowels, and low velocity fasteners, shall receive special inspection per the code evaluation reports for the anchors.
a. Continuous special inspection is required during the installation of all adhesive anchors and rebar dowels. Special inspector shall verify the following:
(1) Anchor size and steel grade.
(2) Hole diameter, location, and type of drill bit.
(3) Cleanliness of hole and anchor.
(4) Adhesive application.
(5) Anchor embedment.

D. Structural Testing: The following materials shall be tested per IBC sections 1704 and 1708. The owner reserves the right to test any and all materials using any appropriate non-destructive procedure. Any items found to be deficient shall be corrected and retested at no additional cost to the owner.
1. Earthwork: All compacted structural fill shall be tested to verify soil gradation, lift thickness, and compaction requirements. See the specifications and earthwork section of the GSN for testing frequency and acceptability criteria.
2. Concrete Strength Verification and Testing. All concrete shall be tested to verify strength, slump, unit weight, air content, and temperature. See the specifications for testing criteria, testing frequency and acceptability criteria.
3. Masonry Strength Verification:
a. Masonry strength, f'm shall be verified using the "Unit Strength Method" per IBC Section 2105.2.2.1 and as follows: Prior to construction, the masonry units and grout shall be tested for compressive strength. Certificates of compliance with strength requirements of the masonry units and grout shall be submitted by the masonry and grout suppliers. During construction, the masonry units and grout shall be tested for every 5,000 square feet of masonry constructed per GSN IV.A, above. Proportions of materials in mortar and grout delivered to the site shall be verified.
b. The contractor has the option of using the "Masonry Prism Test Method" per IBC Section 2105.2.2.2, in lieu of the "Unit Strength Method."
4. Welded connections shall be tested for compliance according to IBC Table 1704.3, AWS D1.1 and the contract specifications and plans. As a minimum, the testing shall include the following:
a. All complete penetration groove welds shall be tested 100 percent either by ultrasonic testing or by radiography.
b. Partial penetration groove welds shall be tested either by ultrasonic testing or radiography. A minimum of 50% of these welds shall be tested.
c. Any material discontinuities shall be accepted or rejected on the basis of the defect rating in accordance with the testing in AWS D1.1 Chapter 6, excluding Sections 6.1 through and including 6.6. All deficient welds shall be corrected and retested at no additional cost to the owner.
5. Post-installed anchors, including but not limited to expansion anchors, adhesive anchors, and low velocity fasteners, shall be tested per the code evaluation reports for the anchors.

E. Structural Observations by the Engineer of Record.
1. The Engineer of Record may perform structural observations at critical phases of the project. Copies of the engineer's report will be distributed to the architect, contractor, owner, and QAA.
2. Observation visits to the site by the Engineer's field representatives shall not be construed as inspection or approval of construction.
3. Notification of Engineer: The contractor shall notify the engineer twenty-four hours prior to:
a. Placing concrete in any footing.
b. Closing any wall forms.
c. Placing concrete in any column, beam or suspended slab.
d. Grouting any masonry.
e. Completing the welding of major sections of steel decking.

F. Contractor Responsibility: The contractor shall prepare and submit a written statement of responsibility to the building official and the owner prior to commencement of work on the project. As a minimum the statement shall contain the following information:
1. Acknowledgement of the quality assurance requirements for the structure.
2. Acknowledgement of receipt of the Quality Assurance Implementation Plan (QAIP) from the testing agency.
3. Acknowledgement that control will be exercised to obtain conformance to the Contract Documents and the QAIP.
4. Quality control procedures within the contractors organization, methods and frequency of reporting, and distribution of the reports.
5. Identification and qualifications of the person(s) responsible for quality control and their position(s) in the organization.

ABBREVIATIONS

AB	ANCHOR BOLT (S)	F.D	FLOOR DRAIN	NIC	NOT IN CONTRACT
ABV	ABOVE	FDTN	FOUNDATION	NTS	NOT TO SCALE
@	AT	F.F.	FINISH FLOOR		
ALT	ALTERNATE	FIN	FINISH	OPNG	OPENING
APPROX	APPROXIMATE	FL	FLOOR	OPP	OPPOSITE
ARCH	ARCHITECT(URAL)	FT	FOOT	O.C.	ON CENTER
		FTG	FOOTING	O.F.	OUTSIDE FACE
BM	BEAM	FV	FIELD VERIFY	OWSJ	OPEN WEB STEEL JOIST
BLW	BELOW				
		GA	GAUGE	PCF	POUNDS/CUBIC FOOT
BRG	BEARING	GALV	GALVANIZED	PL	PLATE
BTWN	BETWEEN	GLB	GLU-LAMINATED BEAM	PLF	POUNDS/LINEAL FOOT
BLDG	BUILDING	GR	GRADE	PNL	PANEL
BOT	BOTTOM	GSN	GENERAL STRUCTURAL NOTES	PSF	POUNDS/SQ FOOT
				PSI	POUNDS/SQ INCH
C.J.	CONSTRUCTION JOINT OR CONTROL JOINT	HB	HORIZONTAL BRIDGING	PT	POINT
CJP	COMPLETE JOINT PENETRATION	HT	HEIGHT	REINF	REINFORCING
		HORIZ	HORIZONTAL	R.D.	ROOF DRAIN
CMU	CONC MASONRY UNIT	HSA	HEADED STUD ANCHORS	REQ'D	REQUIRED
COL	COLUMN				
CONC	CONCRETE	IBC	INTERNATIONAL BUILDING CODE	SHT	SHEET
CONST	CONSTRUCTION			SI	SPECIAL INSPECTION
CONT	CONTINUOUS	ICBO	INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS	SOG	SLAB ON GRADE
CONTR	CONTRACTOR			STD	STANDARD
CTR	CENTER	ICC	INTERNATIONAL CODE COUNCIL	STIFF	STIFFENER
				STL	STEEL
DB	DECK BEARING	IN.	INCH	SQ	SQUARE
DBA	DEFORMED BAR ANCHORS	INSUL	INSULATION	SIM	SIMILAR
		INT	INTERIOR	STR	STRUCTURAL
DBL	DOUBLE	I.F.	INSIDE FACE	STAG	STAGGERED
DET	DETAIL				
DIA	DIAMETER	JT	JOINT	T&B	TOP AND BOTTOM
DIM	DIMENSION	JST	JOIST	TEMP	TEMPERATURE
DN	DOWN	KLF	KIPS PER LINEAL FOOT	THDS	THREADS
DWG	DRAWING	KSF	KIPS PER SQUARE FOOT	T.O.	TOP OF
DWL	DOWEL	KSI	KIPS PER SQUARE INCH	TOC	TOP OF CONCRETE
		K	KIPS – 1,000 POUNDS	TOF	TOP OF FOOTING
EA	EACH			TOS	TOP OF SLAB
E.J.	EXPANSION JOINT (SEISMIC SEPARATION JOINT)	LF	LINEAL FOOT	TOW	TOP OF WALL
		LBS	POUNDS	TYP	TYPICAL
ELEV	ELEVATION	LLH	LONG LEG HORIZONTAL		
ELEC	ELECTRICAL	LLV	LONG LEG VERTICAL	UNO	UNLESS NOTED OTHERWISE
EQUIP	EQUIPMENT				
EQ	EQUAL	MAS	MASONRY	VERT	VERTICAL
EXIST	EXISTING	MAX	MAXIMUM	W/	WITH
EXP	EXPANSION / EXPOSED	MCJ	MASONRY C.J.	WWF	WELDED WIRE FABRIC
EXT	EXTERIOR	MECH	MECHANICAL		
E.F.	EACH FACE	MFR	MANUFACTURER		
E.W.	EACH WAY	MIN	MINIMUM		
		MISC	MISCELLANEOUS		

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UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:
GENERAL STRUCTURAL NOTES

REVISIONS

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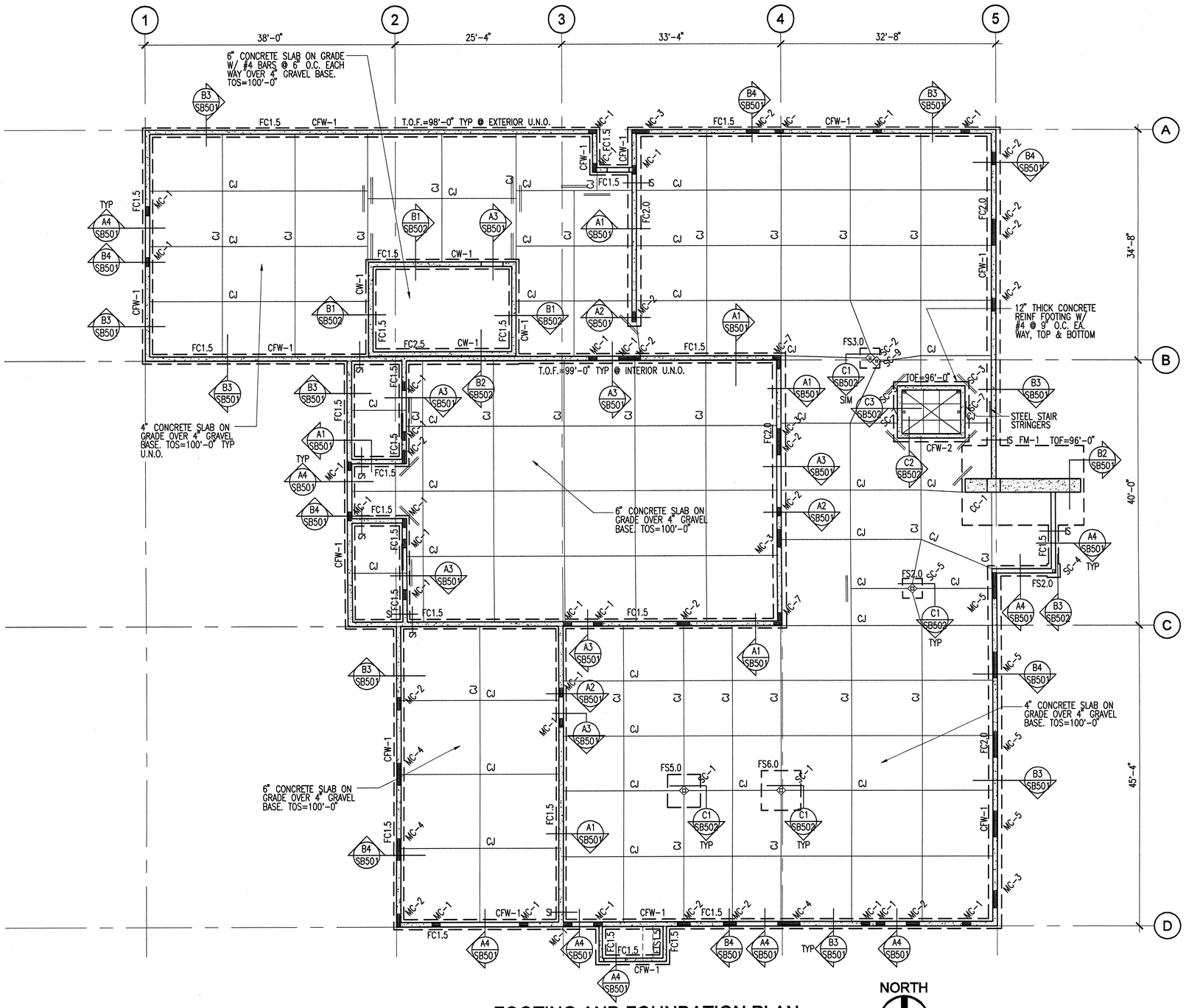

FOOTING AND FOUNDATION PLAN

SCALE: 1/8"=1'-0"

NORTH

FOOTING & FOUNDATION PLAN LEGEND

LEG-FOOTING

- IS FOOTING STEP
- FOOTING – CONTINUOUS
- FOOTING – THICKENED SLAB
- FOOTING – SQUARE FOOTING – RECTANGULAR FOOTING – MAT FOOTING
- CONCRETE WALL, CONCRETE FOUNDATION WALL, OR CONCRETE RETAINING WALL
- CONCRETE FOUNDATION WALL – RECESSED
- CONCRETE COLUMN
- CONCRETE JAMB COLUMN POURED MONOLITHIC WITH CONCRETE WALL
- MASONRY WALL
- STEEL COLUMN – TUBE
- CHANGE IN ELEVATION
- SLAB BLOCK-OUT AT COLUMN
- CJ — SLAB CONTROL/CONSTRUCTION JOINT
- SPECIAL SLAB AREA
- RECESSED/DEPRESSED SLAB
- OPENING

FOOTING & FOUNDATION PLAN NOTES

NOTE-FOOTING

1. SEE ARCHITECTURAL, CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS ETC.
2. SEE ARCHITECTURAL DRAWINGS AND FINISH SCHEDULE FOR SLAB AREAS TO RECEIVE FLOOR TILE.
3. SEE ARCHITECTURAL DRAWINGS FOR SLAB DEPRESSIONS AND SLOPES TO DRAINS, ETC.
4. SEE ARCHITECTURAL, CIVIL AND LANDSCAPE DRAWINGS FOR ADDITIONAL EXTERIOR CONCRETE RETAINING AND / OR SITE WALLS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
5. SEE C1/SB501 FOR TYPICAL EXTERIOR CONCRETE FOOTING STEP DETAIL.
6. SEE C2/SB501 FOR TYPICAL REINFORCEMENT AT WALL CORNERS AND INTERSECTIONS.
7. SEE C3/SB501 FOR TYPICAL DETAIL AT SLAB JOINTS IN CONCRETE SLABS ON GRADE.
8. SEE C4/SB501 FOR REINFORCEMENT AT DISCONTINUOUS CONSTRUCTION / CONTROL JOINTS IN CONCRETE SLAB ON GRADE.
9. REFER TO GENERAL STRUCTURAL NOTES AND SEE B1/SB501 FOR COMPACTED STRUCTURAL FILL REQUIREMENTS BELOW FOOTINGS & SLAB ON GRADE.

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FOOTING AND FOUNDATION PLAN

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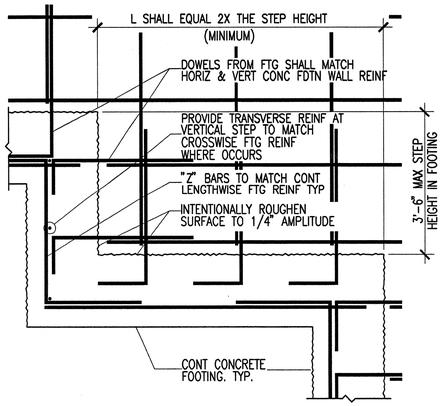
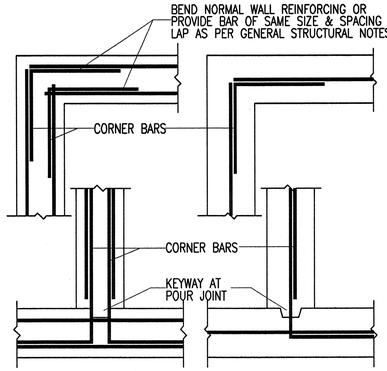
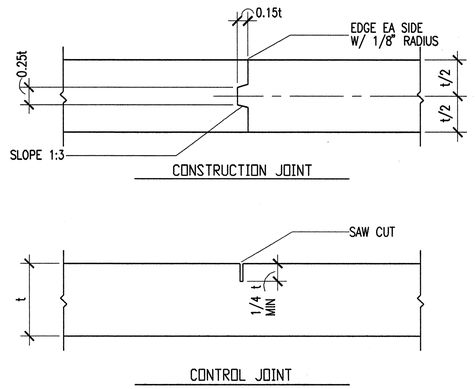


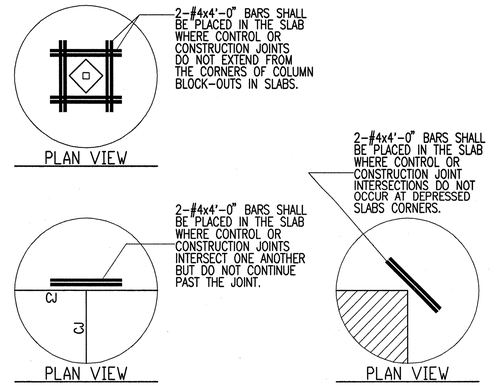
C1 SB501 TYPICAL STEP FOOTING DETAIL
NO SCALE
CF-TYP01



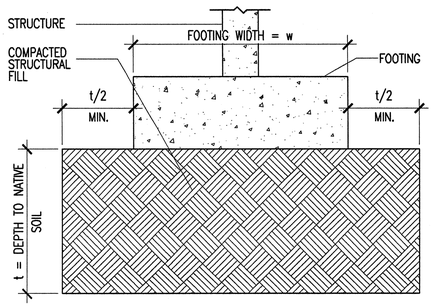
C2 SB501 PLAN VIEW - TYPICAL WALL CORNERS & INTERSECTION
NO SCALE
CF-TYP02



C3 SB501 TYPICAL SLAB JOINTS
NO SCALE
CF-TYP03

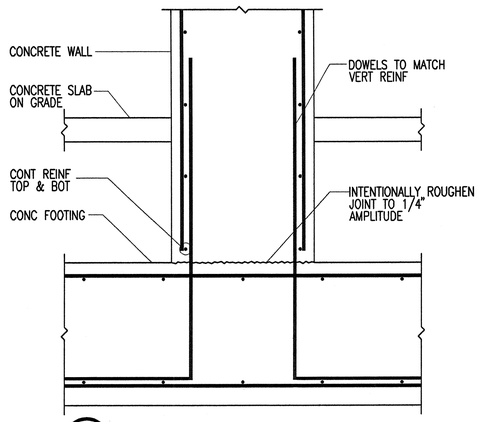


C4 SB501 TYPICAL SLAB REINFORCING AT DISCONTINUOUS SLAB JOINTS
NO SCALE
CF-TYP10

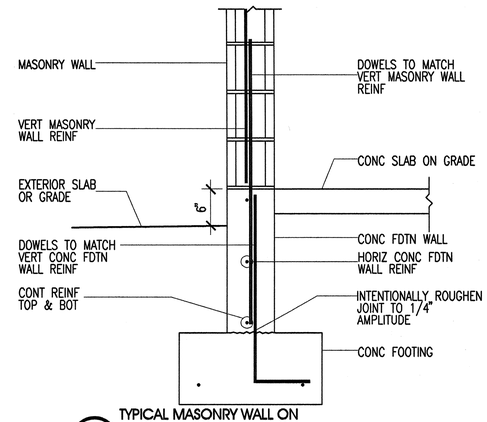


COMPACTED STRUCTURAL FILL:
ALL FILL MATERIAL SHALL BE A WELL-GRADED GRANULAR MATERIAL WITH A MAXIMUM SIZE LESS THAN 4 INCHES AND WITH NOT MORE THAN 18 PERCENT PASSING A NO. 200 SIEVE. IT SHALL BE COMPACTED TO 95 PERCENT OF THE MAXIMUM LABORATORY DENSITY AS DETERMINED BY ASTM D-1557. ALL FILL SHALL BE TESTED (SEE SPECIFICATIONS).

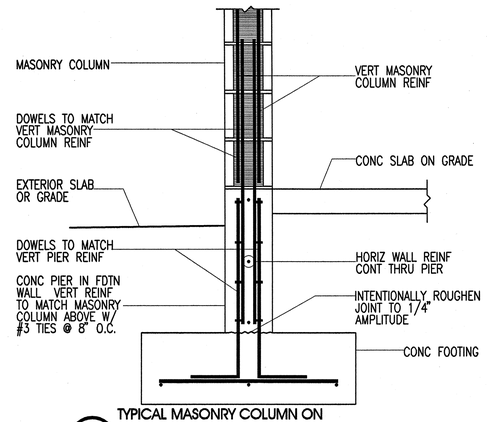
B1 SB501 TYP COMPACTED STRUCTURAL FILL DETAIL
NO SCALE
CF-TYP09



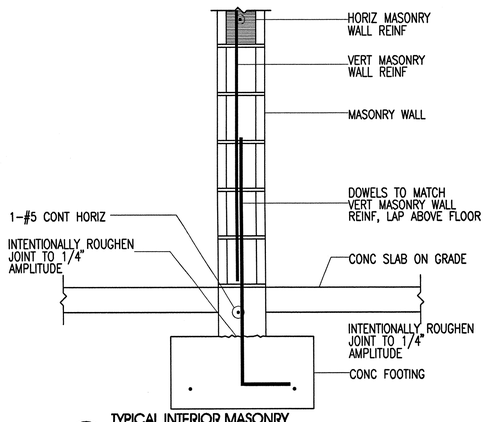
B2 SB501 CONCRETE WALL ON CONCRETE FOOTING
NO SCALE
2004-210-SB501/B2



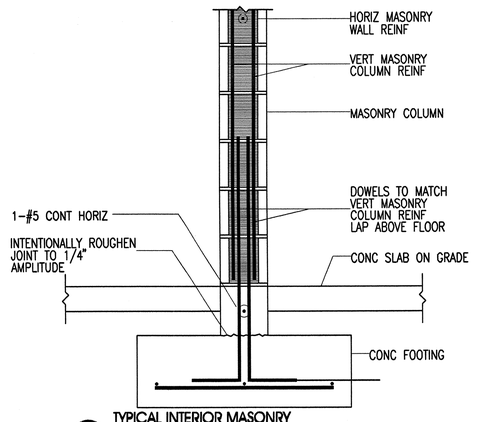
B3 SB501 TYPICAL MASONRY WALL ON FOUNDATION WALL
NO SCALE
MW1CFW01



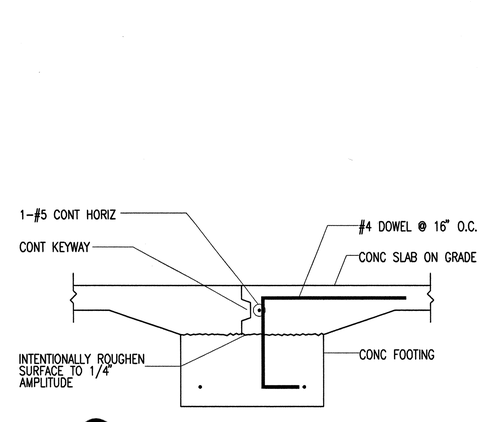
B4 SB501 TYPICAL MASONRY COLUMN ON CONCRETE PIER IN FOUNDATION WALL
NO SCALE
MW1CFW02



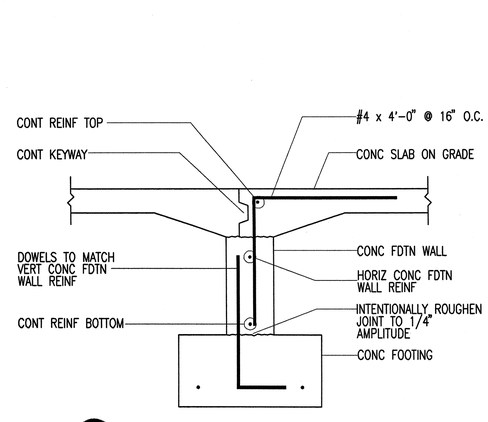
A1 SB501 TYPICAL INTERIOR MASONRY WALL ON CONCRETE CURB WALL
NO SCALE
MW4CFW06



A2 SB501 TYPICAL INTERIOR MASONRY COLUMN ON CONCRETE CURB
NO SCALE
MW4CFW07



A3 SB501 TYPICAL DOOR OPENING AT INTERIOR
NO SCALE
MW4CFW08



A4 SB501 TYPICAL SLAB AT DOOR OPENING
NO SCALE
MW4CFW09

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UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:
FOOTING AND FOUNDATION DETAILS

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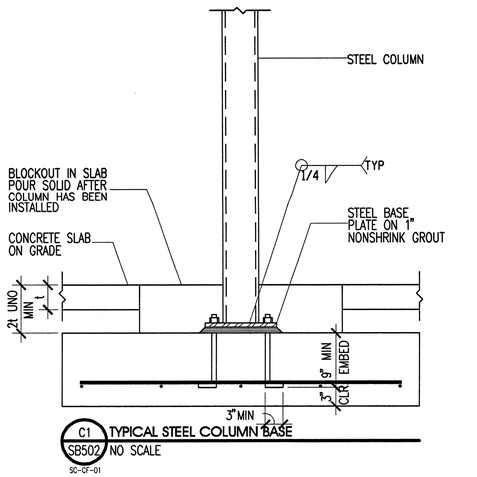
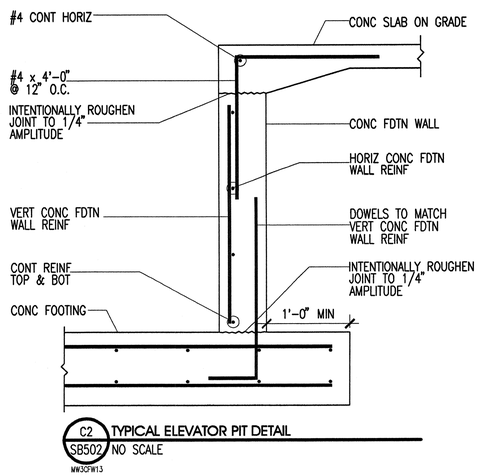


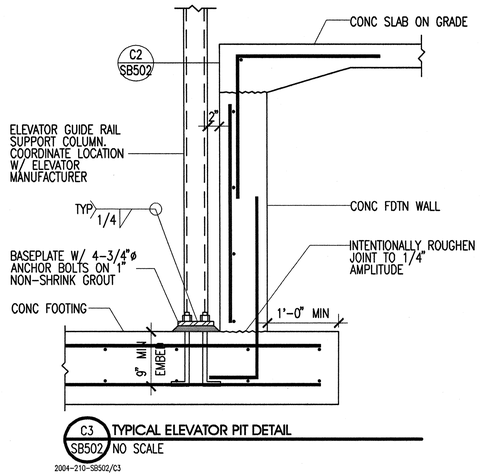
C1 SB502 TYPICAL STEEL COLUMN BASE
NO SCALE

SC-CF-01



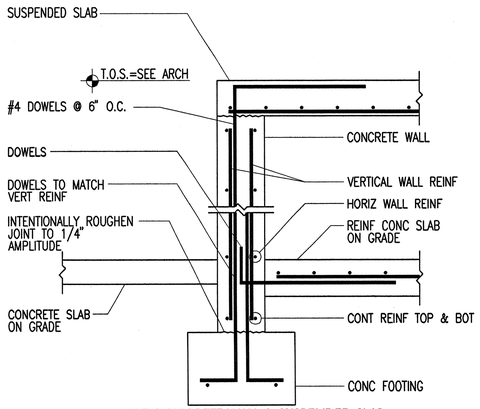
C2 SB502 TYPICAL ELEVATOR PIT DETAIL
NO SCALE

MW3CFW13



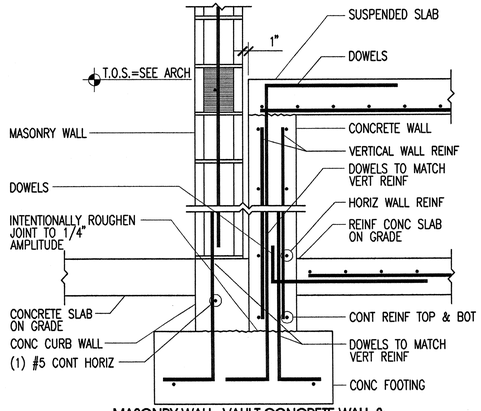
C3 SB502 TYPICAL ELEVATOR PIT DETAIL
NO SCALE

2004-210-SB502/C3



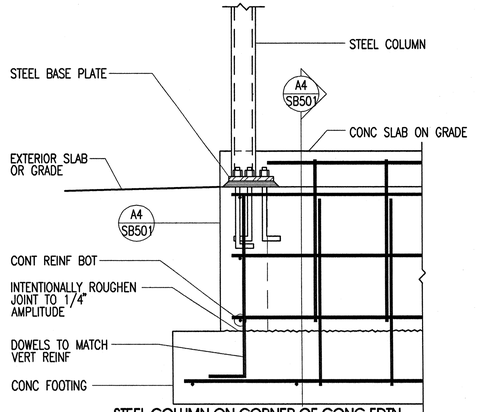
B1 SB502 VAULT CONCRETE WALL & SUSPENDED SLAB ON CONCRETE FOOTING
NO SCALE

SC-CF-01



B2 SB502 MASONRY WALL, VAULT CONCRETE WALL & SUSPENDED SLAB ON CONCRETE FOOTING
NO SCALE

2004-210-SB502/B2



B3 SB502 STEEL COLUMN ON CORNER OF CONC FDTN WALL AT GLAZING SYSTEM
NO SCALE

2004-210-SB502/B3

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CONCRETE FOOTING SCHEDULE

CF-3000

MARK	WIDTH	LENGTH	THICK	CROSSWISE REINFORCING				LENGTHWISE REINFORCING				REMARKS
				NO.	SIZE	LENGTH	SPACE	NO.	SIZE	LENGTH	SPACE	
FTS1.5	1'-6"	CONT.	12"	--	NONE	REQ'D	--	3	#4	CONT.	6"	
FC1.5	1'-6"	CONT.	12"	--	NONE	REQ'D	--	3	#4	CONT.	6"	
FC2.0	2'-0"	CONT.	12"	--	NONE	REQ'D	--	3	#4	CONT.	9"	
FC2.5	2'-6"	CONT.	12"	--	#5	2'-0"	14"	3	#5	CONT.	12"	
FS2.0	2'-0"	2'-0"	12"	2	#5	1'-6"	18"	2	#5	1'-6"	6"	
FS3.0	3'-0"	3'-0"	12"	3	#5	2'-6"	15"	3	#5	2'-6"	15"	
FS3.5	3'-6"	3'-6"	12"	3	#5	3'-0"	18"	3	#5	3'-0"	18"	
FS4.0	4'-0"	4'-0"	12"	4	#5	3'-6"	14"	4	#5	3'-6"	14"	
FS4.5	4'-6"	4'-6"	12"	4	#5	4'-0"	16"	4	#5	4'-0"	16"	
FS5.0	5'-0"	5'-0"	14"	5	#5	4'-6"	13.5"	5	#5	4'-6"	13.5"	
FS5.5	5'-6"	5'-6"	15"	6	#5	5'-0"	12"	6	#5	5'-0"	12"	
FS6.0	6'-0"	6'-0"	16"	8	#5	5'-6"	9.4"	8	#5	5'-6"	9.4"	
FM-1	12'-0"	18'-6"	27"	18	#7	11'-6"	12.8"	12	#7	18'-0"	12.6"	TOP & BOTTOM

NOTES:
1. PLACE ALL FOOTING REINFORCING IN BOTTOM OF FOOTING WITH 3" CLEAR CONCRETE COVER UNLESS NOTED OTHERWISE.
2. TOP REINFORCING, WHERE SPECIFIED, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" CLEAR CONCRETE COVER.
3. SPOT FOOTINGS SHALL BE CENTERED UNDER COLUMNS AND CONTINUOUS FOOTINGS SHALL BE CENTERED UNDER WALLS, UNLESS NOTED OTHERWISE.
4. ALL FOOTINGS SHALL BE FORMED. FOOTINGS SHALL NOT BE EARTH FORMED OR OVERSIZED WITHOUT WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER.

CONCRETE COLUMN SCHEDULE

CC-1

MARK	SIZE	REINFORCING		REMARKS
		VERTICAL	TIES	
CC-1	24"X24"	(8) #8	#3 @ 18" O.C.	

CONCRETE REINFORCING BAR LAP SPLICE SCHEDULE

RBLS-1

BAR SIZE Fy = 60 KSI	TENSION BARS																COMP. BARS
	f'c = 3000 PSI				f'c = 4000 PSI				f'c = 5000 PSI				f'c = 6000 PSI				f'c = ALL
	REGULAR CLASS		TOP CLASS		REGULAR CLASS		TOP CLASS		REGULAR CLASS		TOP CLASS		REGULAR CLASS		TOP CLASS		
	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	
#3	17"	22"	22"	28"	15"	19"	19"	25"	13"	17"	17"	22"	12"	16"	16"	20"	12"
#4	22"	29"	29"	38"	19"	25"	25"	33"	17"	23"	23"	29"	16"	21"	21"	27"	15"
#5	28"	36"	36"	47"	24"	31"	31"	41"	22"	28"	28"	36"	20"	26"	26"	33"	19"
#6	33"	43"	43"	56"	29"	37"	37"	49"	26"	34"	34"	44"	24"	31"	31"	40"	23"
#7	48"	63"	63"	81"	42"	54"	54"	71"	38"	49"	49"	63"	34"	45"	45"	58"	27"

NOTES: THESE NOTES SHALL BE USED FOR ALL SPLICES, UNLESS NOTED OTHERWISE ON DRAWINGS.

1. TOP BARS ARE HORIZONTAL BARS, SPLICED SO THAT 12" OR MORE OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE REINFORCING BAR.
2. CLASS A SPLICES MAY BE USED ONLY WHEN 50% OR LESS OF THE BARS ARE SPLICED WITHIN THE LAP SPLICE LENGTH.
3. CLASS B SPLICES SHALL BE USED FOR ALL SPLICES IN SLABS, BEAMS, JOISTS, WALLS, MOMENT RESISTING COLUMNS, AND JAMB COLUMNS, UNLESS THEY MEET THE REQUIREMENTS OF NOTE #2 ABOVE.
4. TIES AND STIRRUPS SHALL NOT BE SPLICED.
5. A. FOR BUNDLED BARS OF THREE OR LESS, LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.2.
   B. FOR BUNDLED BARS OF FOUR OR MORE, LAP LENGTHS SHALL BE MULTIPLIED BY 1.33.
   C. INDIVIDUAL BAR SPLICES WITHIN A BUNDLE SHALL NOT OVERLAP. ENTIRE BUNDLES SHALL NOT BE LAP SPLICED.
6. FOR ALL LIGHTWEIGHT CONCRETE, LAP LENGTHS SHALL BE MULTIPLIED BY 1.3.
7. FOR ALL EPOXY COATED BARS WITH COVER LESS THAN 3 BAR DIAMETERS OF CLEAR SPACING LESS THAN 6 BAR DIAMETERS THE LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.5. FOR ALL OTHER EPOXY BARS THE SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.2
8. THE BAR LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.5 WHEN EITHER OF THE FOLLOWING IS TRUE:
   A. CLEAR SPACING OF BARS BEING DEVELOPED IS LESS THAN ONE BAR DIAMETER, CLEAR COVER IS LESS THAN ONE BAR DIAMETER AND STIRRUPS OR TIES ALONG THE LENGTH OF THE SPLICE ARE LESS THAN THE CODE MINIMUM.
   B. CLEAR SPACING OF BARS BEING DEVELOPED IS LESS THAN 2 BAR DIAMETERS AND CLEAR COVER IS LESS THAN ONE BAR DIAMETER.

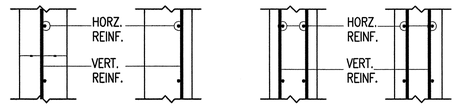
AS PER ACI 318-02

CONCRETE FOUNDATION WALL SCHEDULE

CFW-1

MARK	THICK	HORIZONTAL REINFORCING	VERTICAL REINFORCING	TOP & BOTTOM HORIZONTAL BARS	PLACEMENT
CFW-1	8"	#5@15" O.C.	#4@16" O.C.	2-#5	TYPE A
CFW-2	8"	#5@15 O.C.	#4@16" O.C.	2-#5	TYPE B

PLACEMENT TYPE



TYPE A TYPE B TYPE C TYPE D

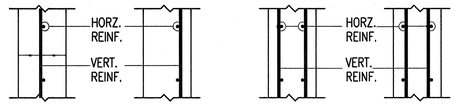
E.F. = EACH FACE
O.F. = OUTSIDE FACE (AGAINST SOIL)
I.F. = INSIDE FACE
3L = THREE LAYERS

CONCRETE WALL SCHEDULE

CW-1

MARK	THICK	HORIZONTAL REINFORCING	VERTICAL REINFORCING	TOP & BOTTOM HORIZONTAL BARS	NOTES
CW-1	8"	#4 @ 9" O.C.	#4 @ 9" O.C.	2-#5	TYPE A
CW-2	24"	#7 @ 18" O.C. E.F.	#6 @ 18" O.C. E.F.	2-#8	TYPE C

PLACEMENT TYPE



TYPE A TYPE B TYPE C TYPE D

E.F. = EACH FACE
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I.F. = INSIDE FACE
3L = THREE LAYERS

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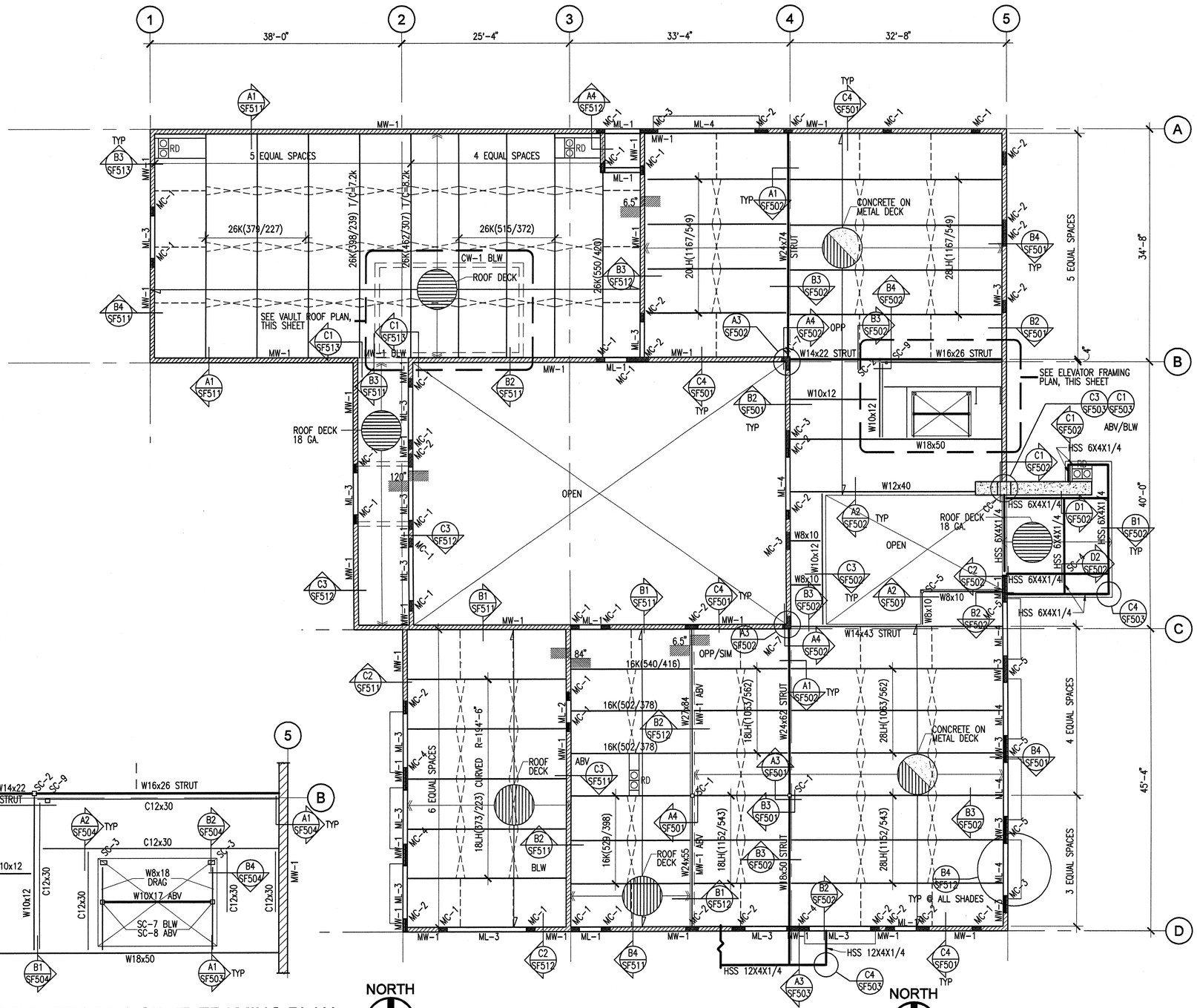

ELEVATOR SUPPORT & STAIR FRAMING PLAN

SCALE: 1/4"=1'-0"

LOW ROOF / FLOOR FRAMING PLAN

SCALE: 1/8"=1'-0"

VAULT ROOF PLAN

SCALE: 1/8"=1'-0"

NOTE:
DESIGN OPEN WEB STEEL JOISTS AT LOW ROOF FOR 5 psf NET WIND UPLIFT LOAD.

FLOOR FRAMING PLAN LEGEND

LEG-FLOOR

- CONCRETE WALL
- CONCRETE COLUMN
- MASONRY WALL
- MASONRY LINTEL IN MASONRY WALL
- MASONRY COLUMN IN MASONRY WALL
- STEEL COLUMN – TUBE
- STEEL BEAM OR GIRDER
- STEEL JOIST OR PURLIN
- CROSS BRIDGING
- HORIZONTAL BRIDGING
- STEEL STUD WALL – STRUCTURAL
- 0" CHANGE IN ELEVATION
- CONCRETE ON METAL DECK
- ROOF DECK
- OPENING

FLOOR FRAMING PLAN NOTES

NOTE-FLOOR

1. SEE GENERAL STRUCTURAL NOTE (III.D.8) AND DETAILS B3/SF101 FOR CONTROL JOINTS IN SUSPENDED SLABS OVER STEEL DECK.
2. SEE DETAILS C1 & C2/SF501 FOR MISCELLANEOUS FLOOR OPENINGS.
3. SEE GENERAL STRUCTURAL NOTE (V.J.7) FOR STEEL DECK REQUIREMENTS WHERE 3-SPAN CONDITIONS ARE NOT POSSIBLE
4. FOR ROOF FRAMING, SEE PLAN NOTES ON SF102.

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SECOND LEVEL FLOOR FRAMING PLAN

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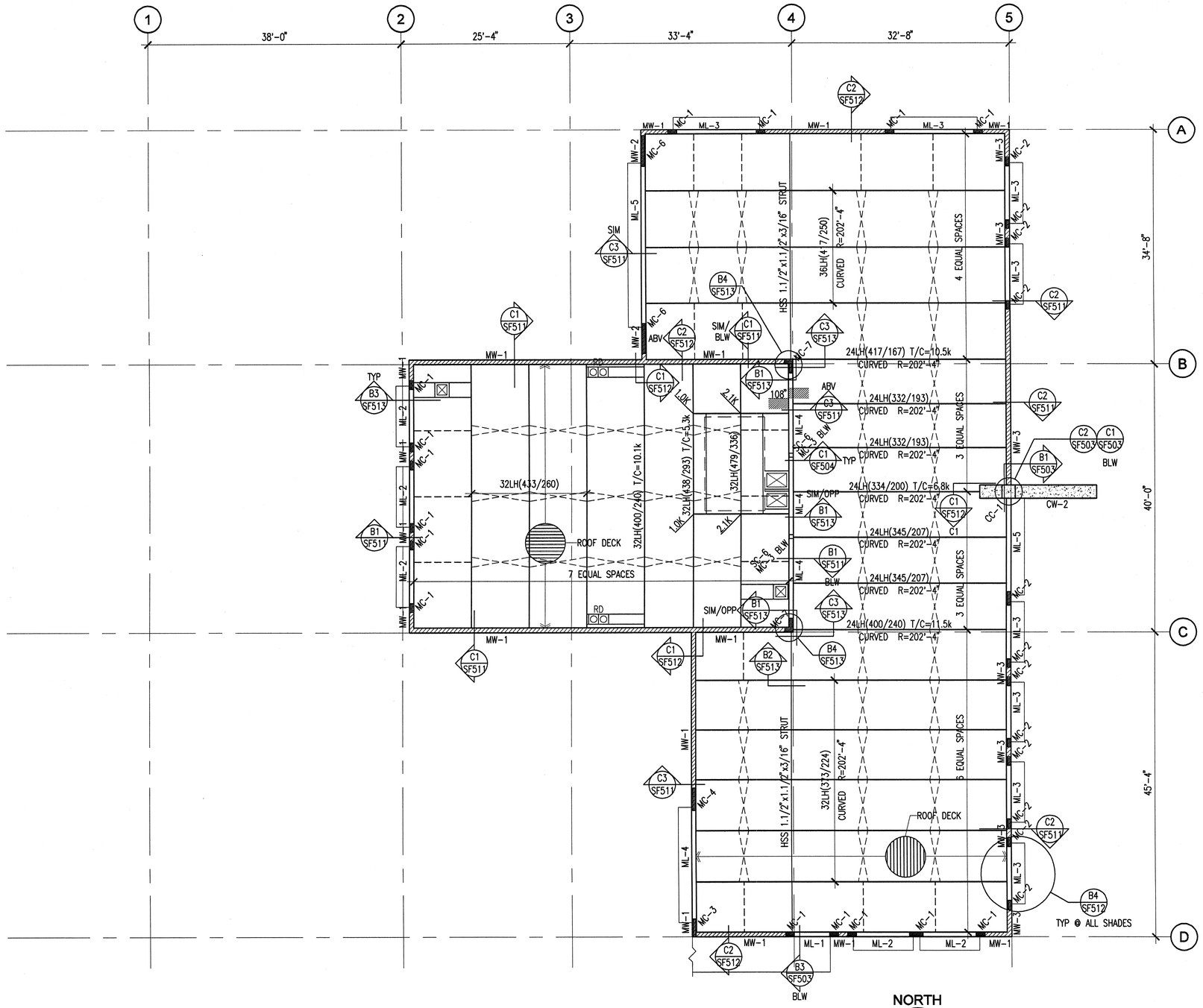

HIGH ROOF FRAMING PLAN

SCALE: 1/8"=1'-0"

NORTH

ROOF FRAMING PLAN LEGEND

LEG-ROOF

- CONCRETE COLUMN
- MASONRY WALL
- MASONRY LINTEL IN MASONRY WALL
- MASONRY COLUMN IN MASONRY WALL
- STEEL COLUMN – TUBE
- STEEL BEAM OR GIRDER
- STEEL JOIST OR PURLIN
- CROSS BRIDGING
- HORIZONTAL BRIDGING
- STEEL STUD WALL – STRUCTURAL
- CHANGE IN ELEVATION
- ROOF DECK
- SPECIAL DECK AREA
- OPENING

ROOF FRAMING PLAN NOTES

NOTE-ROOF

1. VERIFY SIZE, WEIGHT, LOCATION AND CONFIGURATION OF ALL ROOF TOP EQUIPMENT WITH ARCHITECT AND MECHANICAL ENGINEER. PROVIDE STEEL FRAMES FOR SUPPORT OF ROOF TOP EQUIPMENT AS INDICATED IN DETAIL A2/SF512 COORDINATE OPENINGS WITH MECHANICAL AND ELECTRICAL AND GENERAL CONTRACTORS.
2. ALL ROOF OPENINGS SHALL BE FRAMED AS INDICATED IN DETAIL A3/SF511. FOR ROUND OPENINGS WHICH ARE LESS THAN 12"ø SEE DETAIL A4/SF511.
3. SEE ARCHITECTURAL FOR ROOF SLOPES AND DRAINS. SEE A2/SF511 FOR ROOF DRAIN OPENING FRAME.
4. OPEN WEB STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL AND LATERAL LOADS SHOWN ON THE ROOF FRAMING PLANS IN ADDITION TO THE UNIFORM AND POINT LOADS SHOWN.
5. ±#.#K ADD – INDICATES ADDITIONAL UPLIFT / DOWNWARD FORCE ON STEEL JOIST IN ADDITION TO REGULAR LOADS.
6. T/C=##K – INDICATES ADDITIONAL TOP CHORD AXIAL FORCE ON STEEL JOIST OR GIRDER IN ADDITION TO REGULAR LOADS. THIS FORCE IS A SEISMIC LRFD LEVEL FORCE THAT SHALL BE CONSIDERED IN BOTH TENSION AND COMPRESSION. STEEL JOISTS AND GIRDERS WITH T/C FORCE SHALL BE DESIGNED AS COLLECTOR ELEMENTS PER IBC SECTION 1620.16 WITH STRENGTH TO RESIST THE SPECIAL LOAD COMBINATIONS OF IBC SECTION 1605.4.
7. ALL LOADS SUPPORTED BY OPEN WEB STEEL JOISTS AND GIRDERS SHALL BE LOCATED WITHIN 6" OF JOIST OR GIRDER PANEL POINT OR THE JOIST OR GIRDER SHALL BE REINFORCED PER DETAIL A1/SF512.
8. SEE DETAIL A3/SF512 FOR SUPPORT OF HANGING MECHANICAL UNITS.
9. HORIZONTAL AND CROSS BRIDGING SHALL BE SIZED AND SUPPLIED BY THE JOIST MANUFACTURER. CONNECT TO WALLS AS INDICATED IN DETAILS.
10. WHERE SKYLIGHTS OR MECHANICAL UNITS INTERRUPT HORIZONTAL BRIDGING PROVIDE CROSS BRIDGING AT JOIST SPACES ON EACH SIDE. TYPICAL.
11. R=#'-#" INDICATES TOP CHORD RADIUS OF CURVED JOIST.

NOTE:
DESIGN OPEN WEB STEEL JOISTS AT LOW ROOF FOR 8 psf NET WIND UPLIFT LOAD.

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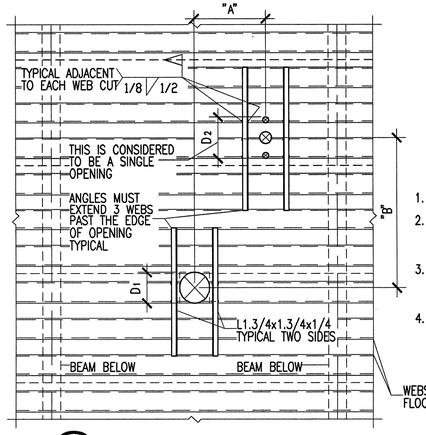


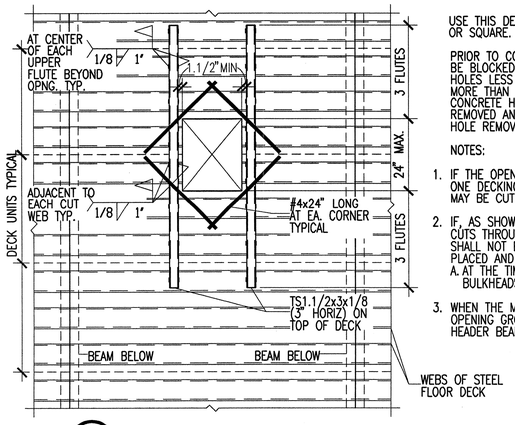
USE THIS DETAIL FOR HOLES CUTTING NO MORE THAN: 3 ADJACENT WEBS FOR 6" AND 8" MODULE DECK, 2 ADJACENT WEBS FOR 12" MODULE DECK

PRIOR TO CONCRETE POUR, SMALL OPENINGS SHOULD BE BLOCKED OUT AND FLOOR DECK LEFT INTACT. HOLES LESS THAN 6" IN DIAMETER AND CUTTING NO MORE THAN 1 WEB NEED NO REINFORCING. AFTER THE CONCRETE HAS CURED, THE BLOCKOUT CAN BE REMOVED AND THE FLOOR DECK IN THE AREA OF THE HOLE REMOVED.

NOTES:

1. ANGLES SHALL BE PLACED ON TOP OF THE DECK.
2. IF DIMENSION "A" IS GREATER THAN $4D_1$, $4D_2$, OR 32" WHICHEVER IS LARGER, THEN THERE IS NO RESTRICTION ON DIMENSION "B".
3. IF DIMENSION "B" IS GREATER THAN $4D_1$, $4D_2$, OR 32" WHICHEVER IS LARGER, THEN THERE IS NO RESTRICTION ON DIMENSION "A".
4. IF DIMENSIONS "A" AND "B" ARE LESS THAN $4D_1$, $4D_2$, OR 32" WHICHEVER IS LARGER, THE OPENING GROUP WILL BE CONSIDERED AS A SINGLE HOLE, AND MUST BE REINFORCED AS REQUIRED FOR THE LARGER OPENING.

C1 SF501 TYPICAL MISCELLANEOUS FLOOR OPENING REINFORCING DETAIL
NO SCALE
FLROPNG1



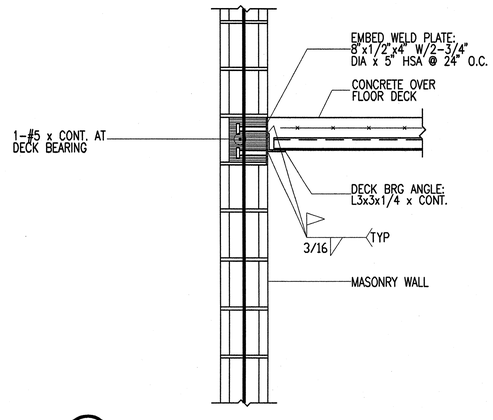
USE THIS DETAIL FOR LARGER HOLES, RECTANGULAR OR SQUARE.

PRIOR TO CONCRETE POUR, SMALL OPENINGS SHOULD BE BLOCKED OUT AND FLOOR DECK LEFT INTACT. HOLES LESS THAN 6" IN DIAMETER AND CUTTING NO MORE THAN 1 WEB NEED NO REINFORCING. AFTER THE CONCRETE HAS CURED, THE BLOCKOUT CAN BE REMOVED AND THE FLOOR DECK IN THE AREA OF THE HOLE REMOVED.

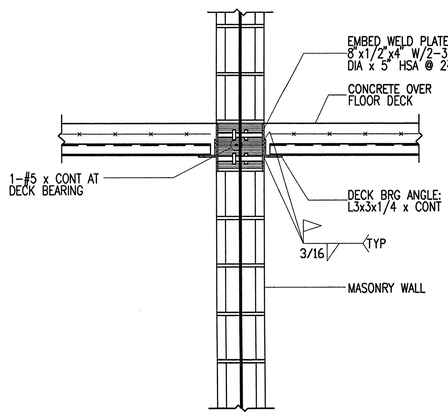
NOTES:

1. IF THE OPENING OR GROUP OF OPENINGS OCCURS IN ONE DECKING UNIT, THE OPENING OR OPENING GROUP MAY BE CUT PRIOR TO POURING OF CONCRETE.
2. IF, AS SHOWN, THE OPENING OR OPENING GROUP CUTS THROUGH TWO DECKING UNITS, THE DECKING SHALL NOT BE CUT UNTIL CONCRETE HAS BEEN PLACED AND CURED.
   A. AT THE TIME OF POURING, SUITABLE SLEEVES OR BULKHEADS SHALL BE PLACED AROUND THE OPENING.
3. WHEN THE MAXIMUM DIMENSION OF AN OPENING OR OPENING GROUP EXCEEDS 24", PROVIDE W8x10 HEADER BEAMS BELOW THE DECK AT ALL SIDES.

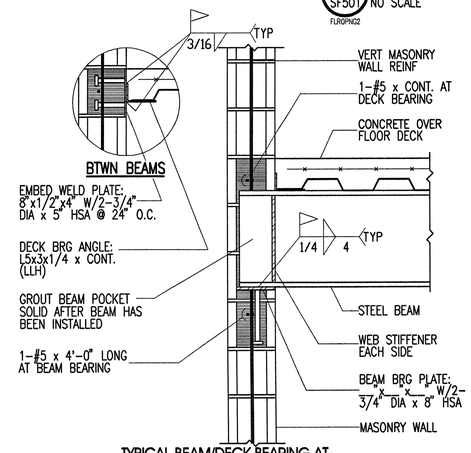
C3 SF501 TYPICAL MISCELLANEOUS FLOOR OPENING REINFORCING DETAIL
NO SCALE
FLROPNG2



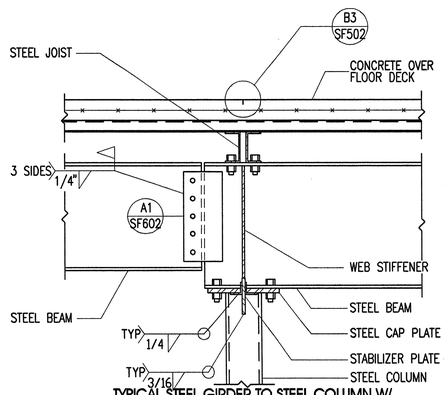
C4 SF501 TYPICAL DECK BEARING AT MASONRY WALL
NO SCALE
SB1MW101



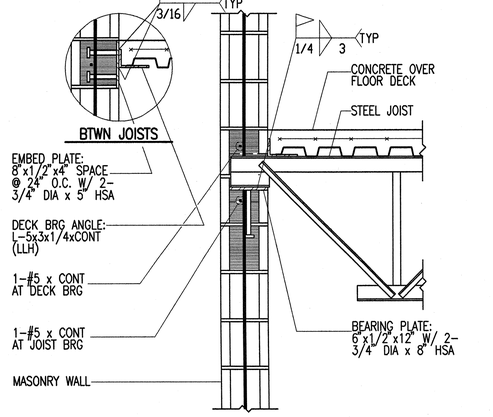
B1 SF501 TYPICAL DECK BEARING AT MASONRY WALL
NO SCALE
SB1MW102



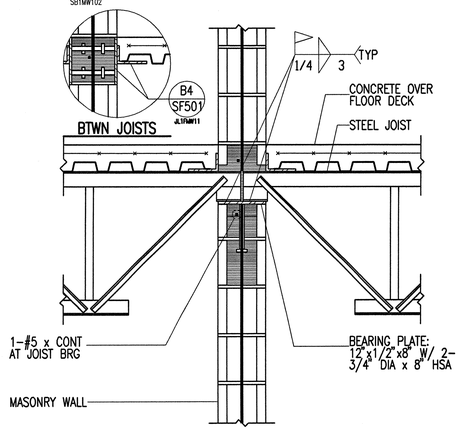
B2 SF501 TYPICAL BEAM/DECK BEARING AT MASONRY WALL
NO SCALE
SB1MW106



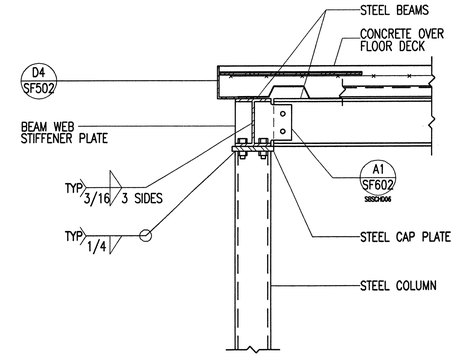
B3 SF501 TYPICAL STEEL GIRDER TO STEEL COLUMN W/ DRAG CONNECTION
NO SCALE
2004-210-SF501/B3



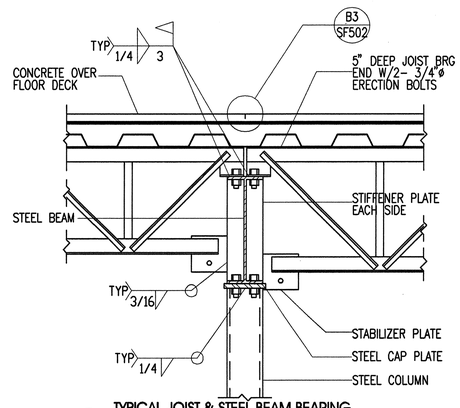
B4 SF501 TYP JOIST/DECK BEARING AT MASONRY WALL
NO SCALE
JL1FMW11



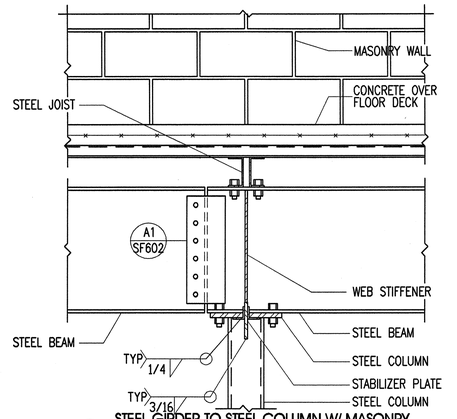
A1 SF501 TYP JOIST/DECK BEARING AT MASONRY WALL
NO SCALE
JL1FMW16



A2 SF501 TYPICAL STEEL BEAM TO STEEL COLUMN
NO SCALE
SB2FLR01



A3 SF501 TYPICAL JOIST & STEEL BEAM BEARING ON STEEL COLUMN
NO SCALE
2004-210-SF501/A3



A4 SF501 STEEL GIRDER TO STEEL COLUMN W/ MASONRY WALL ABOVE
NO SCALE
2004-210-SF501/A4

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UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:
FLOOR FRAMING DETAILS

REVISIONS

MARK	DATE	DESCRIPTION

ISSUE DATA

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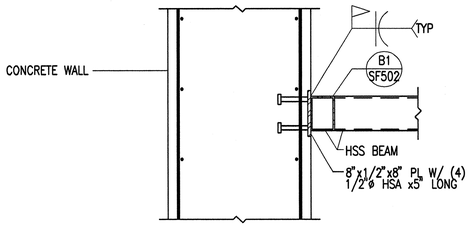
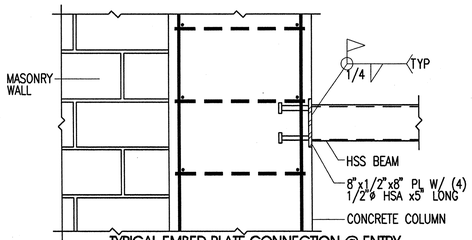
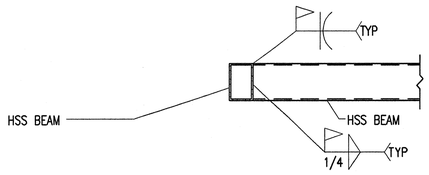


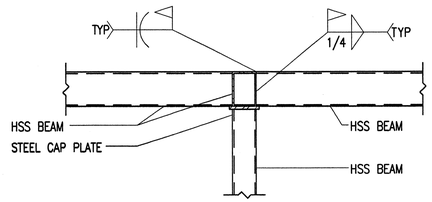
D1 SF502 — FLUSH BEAM CONNECTION TO CONCRETE WALL
NO SCALE
2004-210-SF502/D1



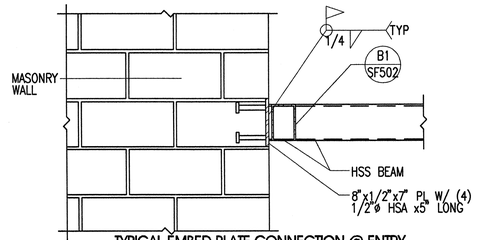
C1 SF502 — TYPICAL EMBED PLATE CONNECTION @ ENTRY ROOF FRAMING TO CONCRETE COLUMN
NO SCALE
2004-210-SF502/C1



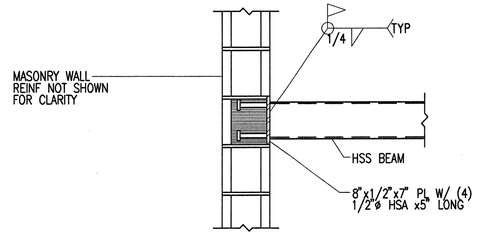
B1 SF502 — TYPICAL HSS TO HSS CONNECTION
NO SCALE
2004-210-SF502/B1



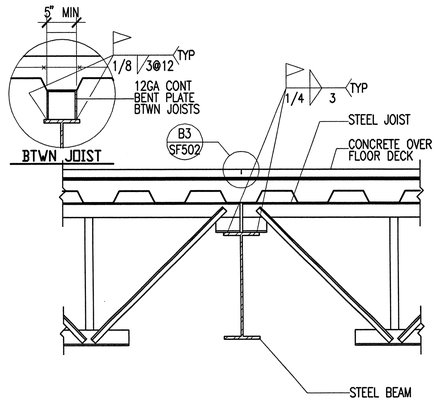
D2 SF502 — TYPICAL HSS TO HSS CONNECTION
NO SCALE
2004-210-SF502/D2



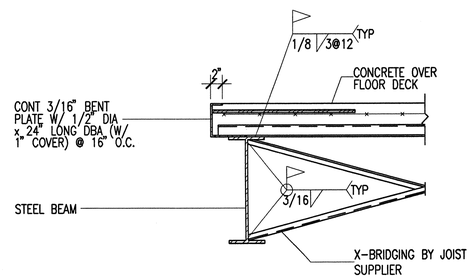
C2 SF502 — TYPICAL EMBED PLATE CONNECTION @ ENTRY ROOF FRAMING TO MASONRY WALL
NO SCALE
2004-210-SF502/C2



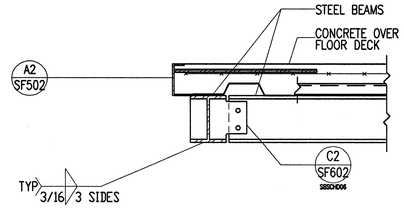
B2 SF502 — EMBED PLATE DETAIL PERP TO MASONRY WALL
NO SCALE
2004-210-SF502/B2



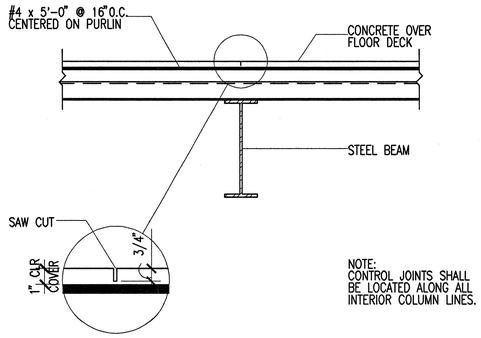
A1 SF502 — TYPICAL JOIST BEARING AT STEEL BEAM CHORD TIE/DRAG STRUT
NO SCALE
JL3FTS06



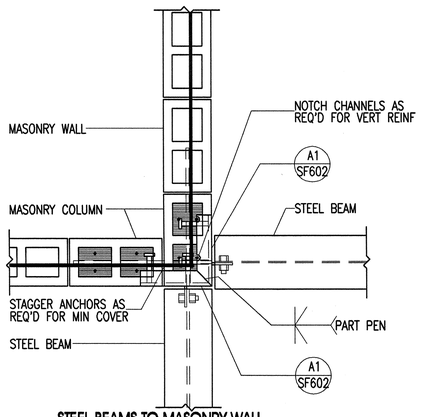
A2 SF502 — TYPICAL EDGE BEAM W/ BRIDGING
NO SCALE
JL3FTS11



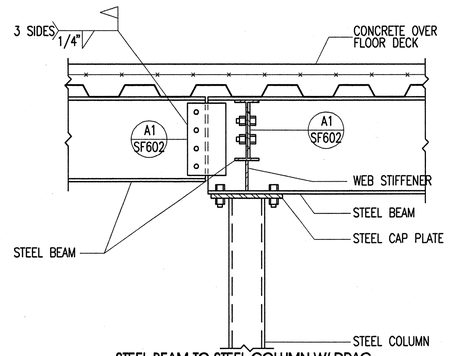
C3 SF502 — TYPICAL STEEL BEAM TO STEEL BEAM
NO SCALE
2004-210-SF502/C3



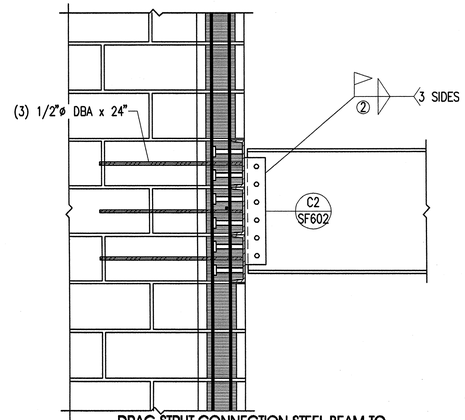
B3 SF502 — TYPICAL CONCRETE DECK CONTROL JOINT AT STEEL PURLIN
NO SCALE
2004-210-SF502/B3



A3 SF502 — STEEL BEAMS TO MASONRY WALL CONNECTIONS
NO SCALE



B4 SF502 — STEEL BEAM TO STEEL COLUMN W/ DRAG CONNECTION
NO SCALE
2004-210-SF502/B4



A4 SF502 — DRAG STRUT CONNECTION STEEL BEAM TO MASONRY WALL
NO SCALE
2004-210-SF502/A4

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CAMP WG WILLIAMS
RIVERTON, UTAH

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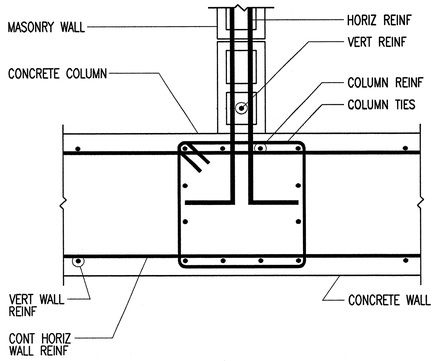
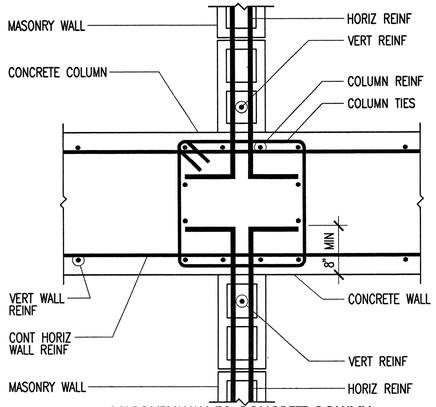


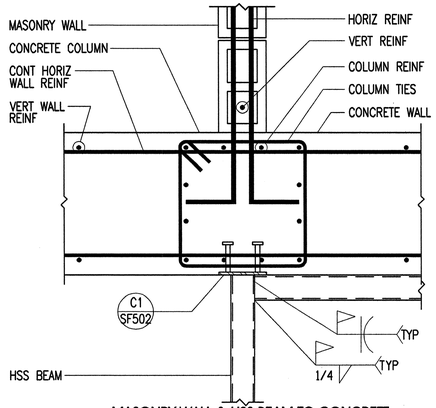
C1 SF503 MASONRY WALL TO CONCRETE COLUMN CONNECTION — NO SCALE

2004-210-SF503/C1



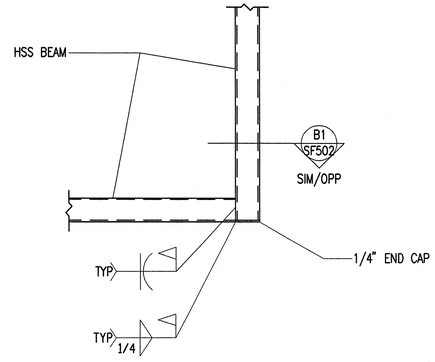
C2 SF503 MASONRY WALL TO CONCRETE COLUMN CONNECTION — NO SCALE

2004-210-SF503/C2



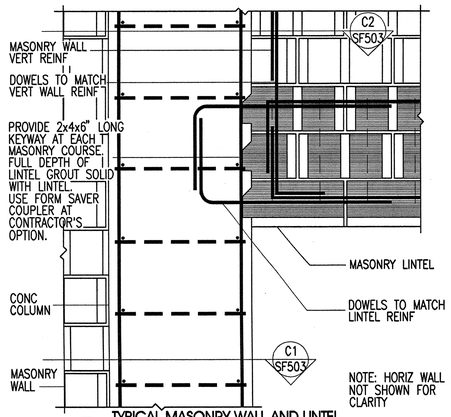
C3 SF503 MASONRY WALL & HSS BEAM TO CONCRETE COLUMN CONNECTION — NO SCALE

2004-210-SF503/C3



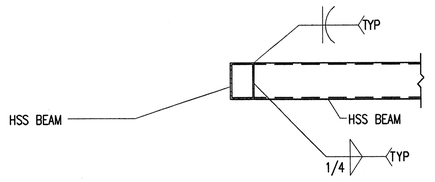
C4 SF503 TYPICAL HSS TO HSS CONNECTION — NO SCALE

2004-210-SF503/C4



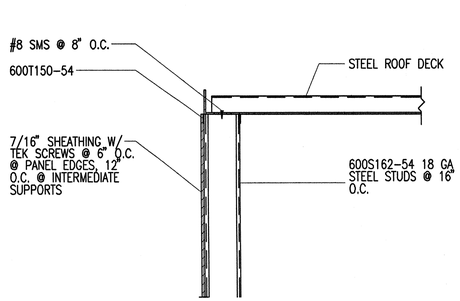
B1 SF503 TYPICAL MASONRY WALL AND LINTEL DOWELS TO CONCRETE WALL OR COLUMN — NO SCALE

2004-210-SF503/B1



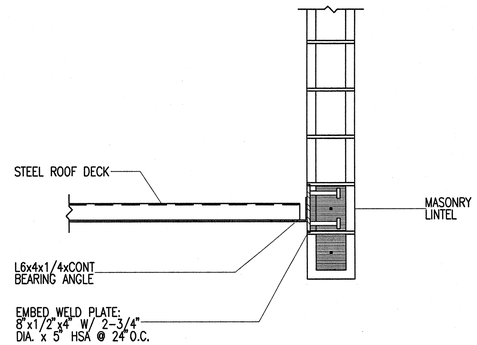
B2 SF503 TYPICAL HSS TO HSS CONNECTION - SHOP WELD — NO SCALE

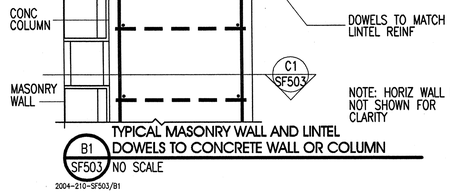
2004-210-SF503/B2



B3 SF503 SOUTH STAIR SECTION — NO SCALE

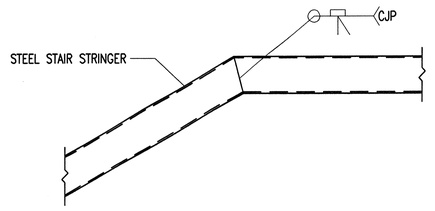
FILENAME





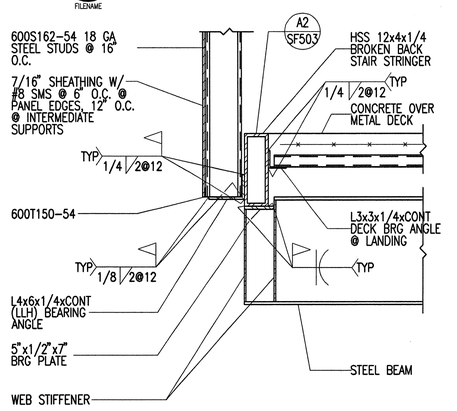
A1 SF503 ELEVATOR SUPPORT MEMBER DRAG CONNECTION TO FLOOR BEAM — NO SCALE

2004-210-SF503/A1



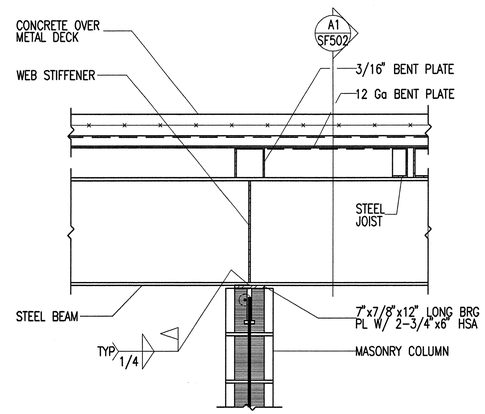
A2 SF503 TYPICAL BROKEN BACK STEEL STAIR STRINGER DETAIL — NO SCALE

2004-210-SF503/A2



A3 SF503 SOUTH STAIR SECTION — NO SCALE

FILENAME



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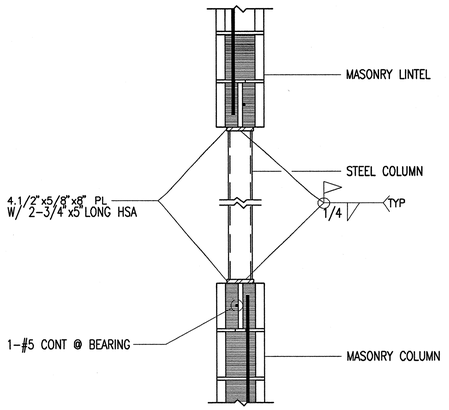
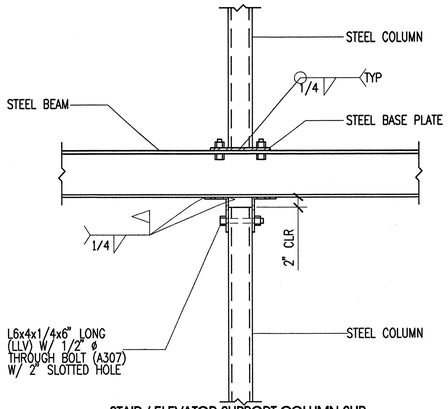
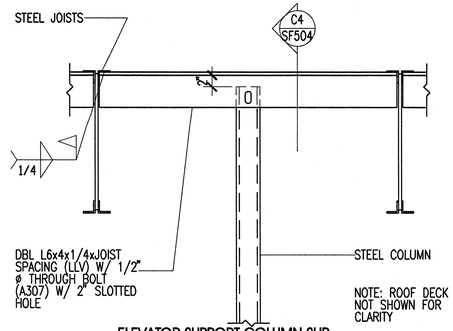


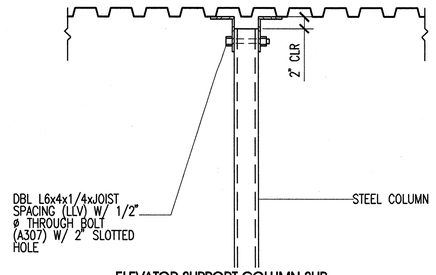
C1 SF504 STEEL COLUMN SUPPORT
NO SCALE
2004-210-SF504/C1



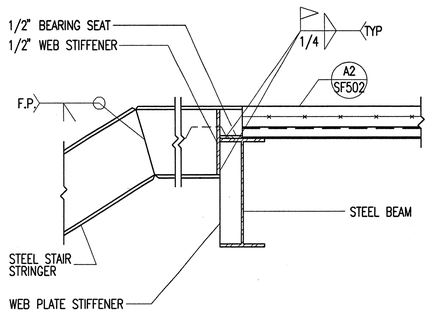
C2 SF504 STAIR / ELEVATOR SUPPORT COLUMN SLIP CONNECTION @ STEEL BEAM
NO SCALE
2004-210-SF504/C2



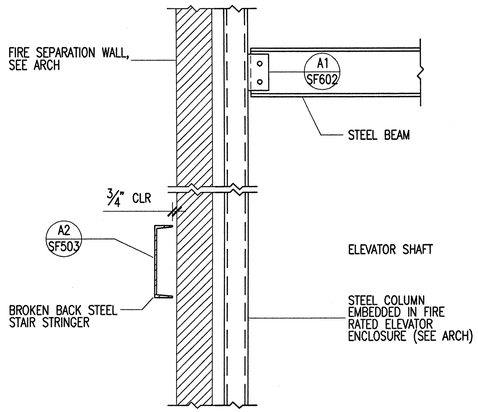
C3 SF504 ELEVATOR SUPPORT COLUMN SLIP CONNECTION BETWEEN STEEL JOISTS
NO SCALE
2004-210-SF504/C3



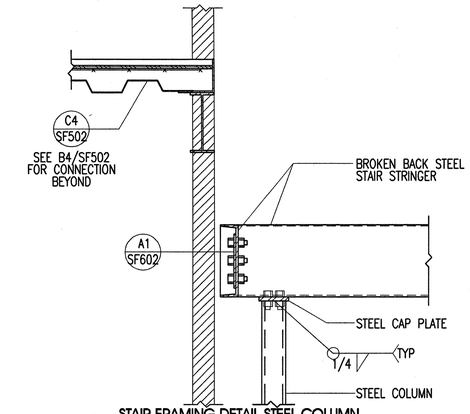
C4 SF504 ELEVATOR SUPPORT COLUMN SLIP CONNECTION BETWEEN STEEL JOISTS
NO SCALE
2004-210-SF504/C4



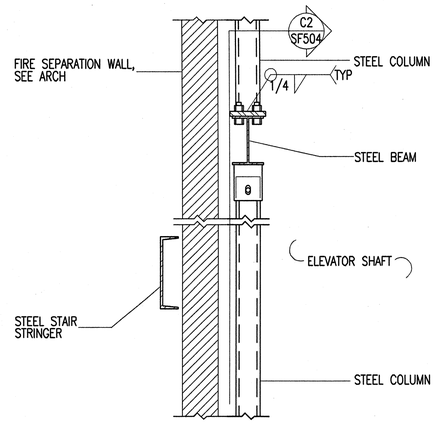
B1 SF504 STEEL STAIR CONNECTION TO STEEL BEAM
NO SCALE
2004-210-SF504/B1



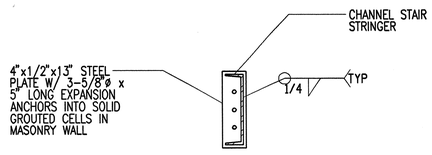
B2 SF504 STAIR STRINGER & ELEVATOR SUPPORT DETAIL
NO SCALE
2004-210-SF504/B2



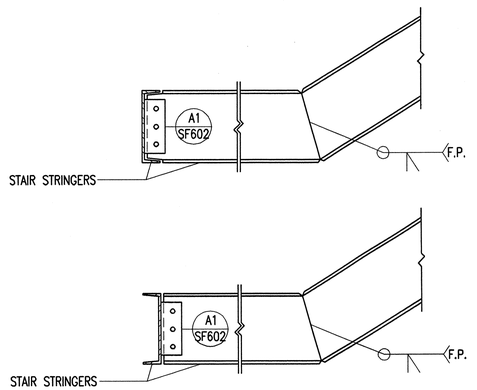
B3 SF504 STAIR FRAMING DETAIL STEEL COLUMN SUPPORTING STAIR STRINGERS
NO SCALE
2004-210-SF504/B3



B4 SF504 STAIR STRINGER & ELEVATOR SUPPORT DETAIL
NO SCALE
2004-210-SF504/B4



A1 SF504 CHANNEL STAIR STRINGER CONNECTION TO MASONRY WALL
NO SCALE
2004-210-SF504/A1



A2 SF504 CHANNEL STAIR STRINGER CONNECTION TO CHANNEL STAIR STRINGER
NO SCALE
2004-210-SF504/A1

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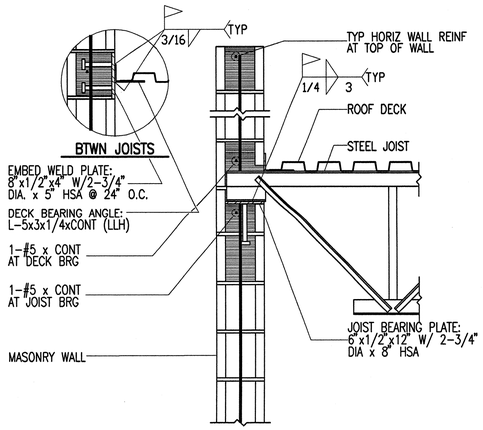
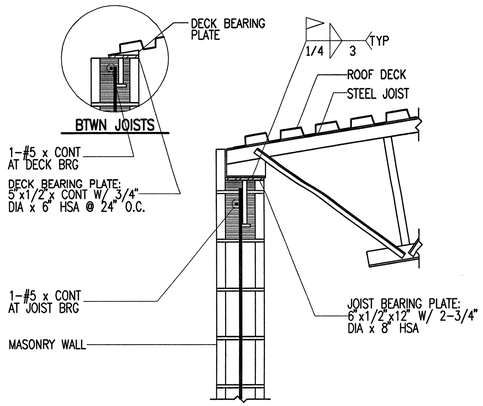
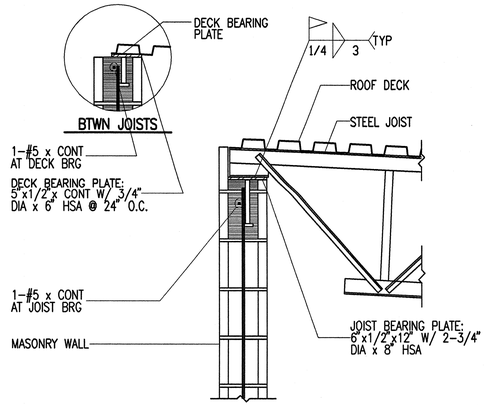


C1 TYP JOIST/DECK BEARING AT MASONRY WALL
SF511 NO SCALE
2004-210-SF511/C1



C2 TYP JOIST/DECK BEARING AT MASONRY WALL
SF511 NO SCALE
2004-210-SF511/C2



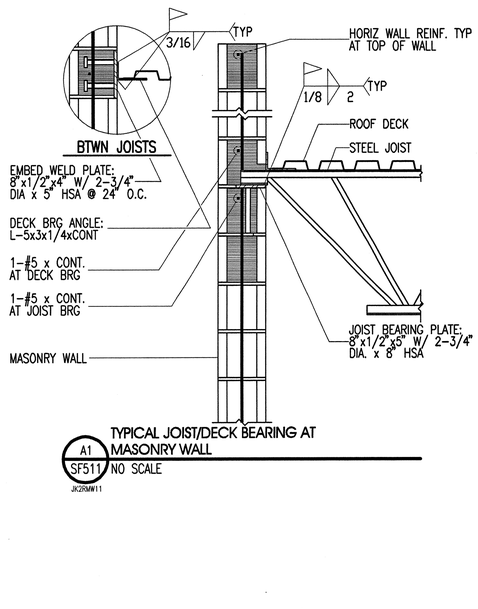
C3 TYP JOIST/DECK BEARING AT MASONRY WALL
SF511 NO SCALE
2004-210-SF511/C3

B1 TYPICAL DECK BEARING AT MASONRY WALL WITH BRIDGING ATTACHMENT
SF511 NO SCALE

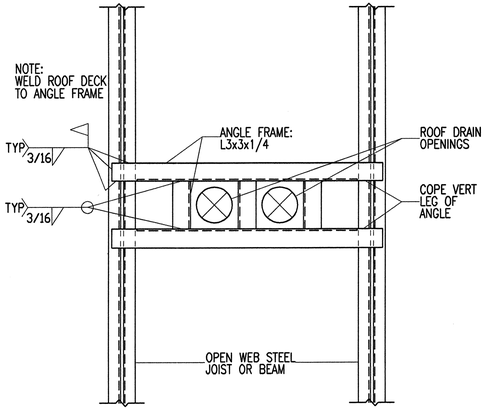
B2 TYPICAL JOIST/DECK BEARING AT MASONRY WALL
SF511 NO SCALE
JK2RMW09

B3 TYPICAL JOIST/DECK BEARING AT MASONRY WALL
SF511 NO SCALE
JK2RMW20

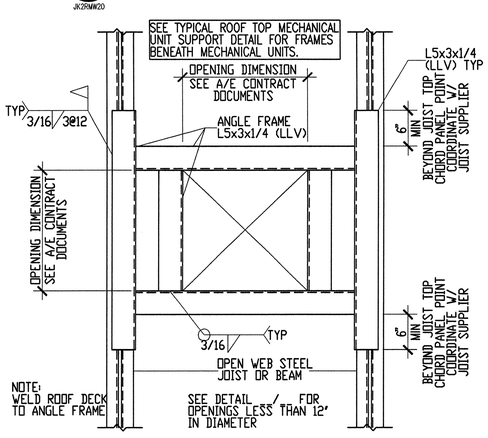
B4 TYPICAL DECK BEARING AT MASONRY WALL WITH BRIDGING ATTACHMENT
SF511 NO SCALE
JK2RMW12



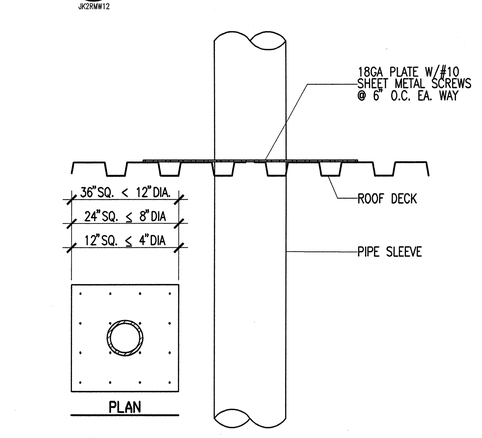
A1 TYPICAL JOIST/DECK BEARING AT MASONRY WALL
SF511 NO SCALE
JK2RMW11



A2 TYPICAL ROOF DRAIN OPENING (PLAN VIEW)
SF511 NO SCALE
JK2RTS15



A3 TYPICAL ROOF OPENING DETAIL (PLAN VIEW)
SF511 NO SCALE
JK2RTS07



A4 TYPICAL PIPE SLEEVE HOLE THRU ROOF DECK
SF511 NO SCALE
JK2RTS09

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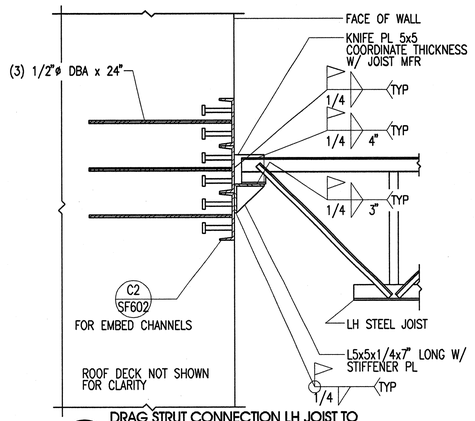
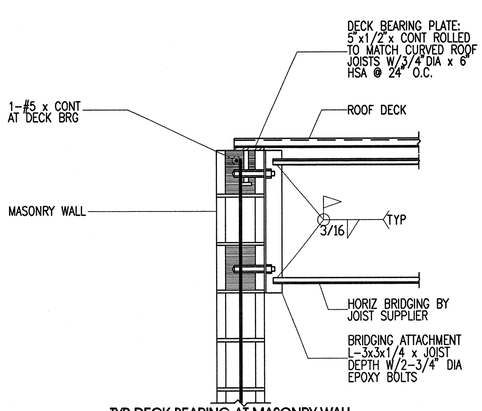


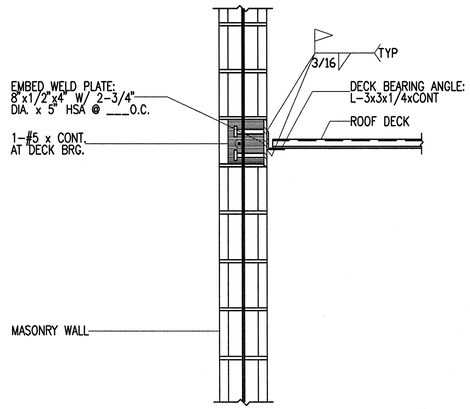
C1 SF512 — DRAG STRUT CONNECTION LH JOIST TO CONTINUOUS MASONRY OR CONCRETE WALL — NO SCALE

2004-210-SF512/C1



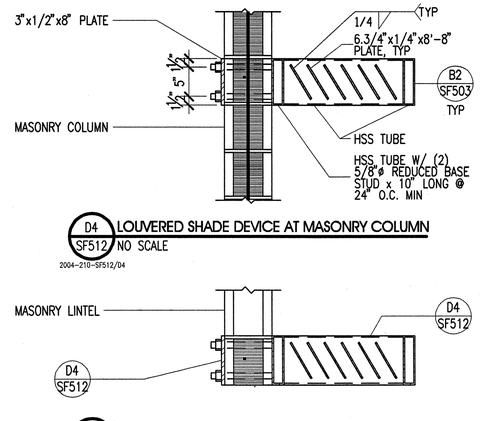
C2 SF512 — TYP DECK BEARING AT MASONRY WALL WITH BRIDGING ATTACHMENT — NO SCALE

2004-210-SF512/C2



C3 SF512 — TYPICAL DECK BEARING AT MASONRY WALL — NO SCALE

2004-210-SF512/C3

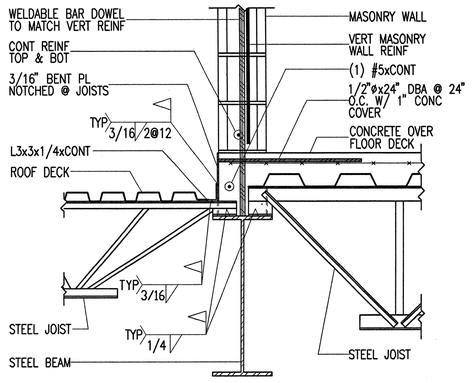


D4 SF512 — LOUVERED SHADE DEVICE AT MASONRY COLUMN — NO SCALE

2004-210-SF512/D4

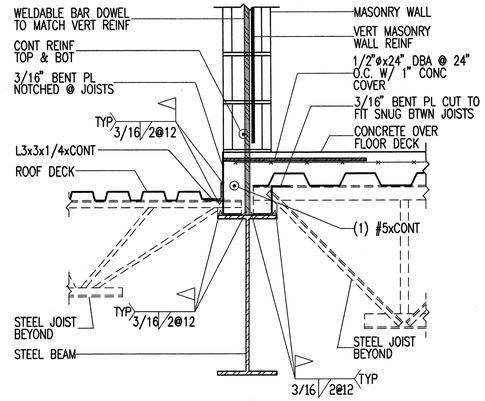
C4 SF512 — LOUVERED SHADE DEVICE AT MANSORY LINTEL — NO SCALE

2004-210-SF512/C4



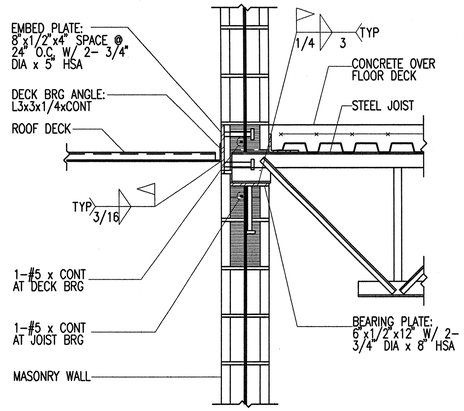
B1 SF512 — TYPICAL JOIST & STEEL BEAM BEARING ON STEEL COLUMN — NO SCALE

2004-210-SF512/B1



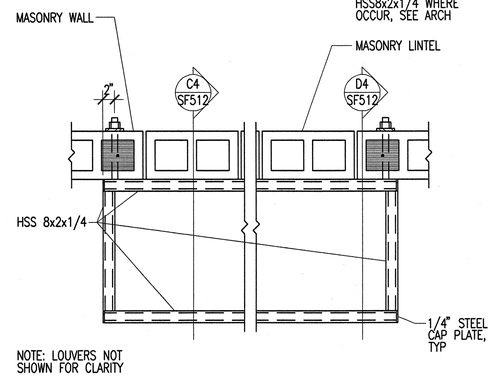
B2 SF512 — MASONRY WALL BEARING ON STEEL BEAM BETWEEN JOISTS — NO SCALE

2004-210-SF512/B2



B3 SF512 — JOIST/DECK BEARING AT MASONRY WALL — NO SCALE

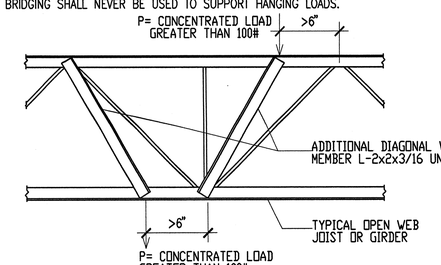
2004-210-SF512/B3



B4 SF512 — LOUVERED SHADE DEVICE CONNECTION - PLAN VIEW — NO SCALE

2004-210-SF512/B4

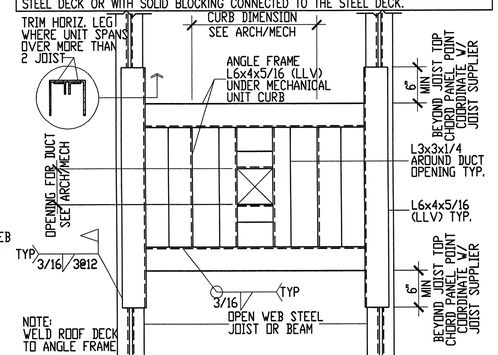
NOTE:
WHEN CONCENTRATED LOADS (GREATER THAN 100#) ON OPEN WEB JOISTS OR GIRDERS ARE LOCATED MORE THAN 6 INCHES FROM THE PANEL WORKPOINTS AT EITHER THE TOP OR BOTTOM CHORD ADDITIONAL DIAGONAL WEB MEMBERS SHALL BE FURNISHED AND INSTALLED AT THE LOCATION OF THE CONCENTRATED LOAD BY THE CONTRACTOR. CONCENTRATED POINT LOADS, SINGLE OR MULTIPLE, TOTALING 100# OR LESS CAN BE LOCATED AT ANY POINT ALONG THE TOP OR BOTTOM CHORD OF AN OPEN WEB JOIST OR GIRDER BETWEEN ADJACENT PANEL POINTS WITHOUT MEETING THESE REQUIREMENTS. A LIMIT OF FOUR CONCENTRATED 100# MAX. POINT LOADS PER JOIST OR GIRDER WILL BE PERMITTED ON SPANS OF 12' & GREATER, ONE CONCENTRATED 100# MAX. LOAD ON SPANS LESS THAN 12', UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS. JOIST BRIDGING SHALL NEVER BE USED TO SUPPORT HANGING LOADS.



A1 SF512 — TYPICAL DETAIL AT ADDITIONAL CONCENTRATED POINT LOAD — NO SCALE

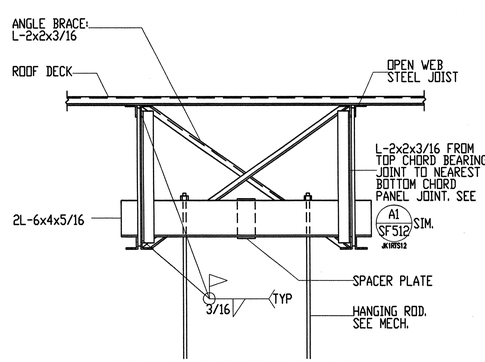
JK1RTS12

WHERE THE MECHANICAL UNIT WEIGHS LESS THAN 600#, THE L6x4x5/16 SUPPORT ANGLES MAY CHANGED TO L5x3x1/4 (LLV). SOLID BLOCK THE FLUTES OF THE STEEL DECK BENEATH THE CURB OF THE MECHANICAL UNIT. THIS MAY BE DONE WITH A HSS1.1/2x1.1/2x1/4 x6" LONG TACK WELDED TO THE STEEL DECK OR WITH SOLID BLOCKING CONNECTED TO THE STEEL DECK.



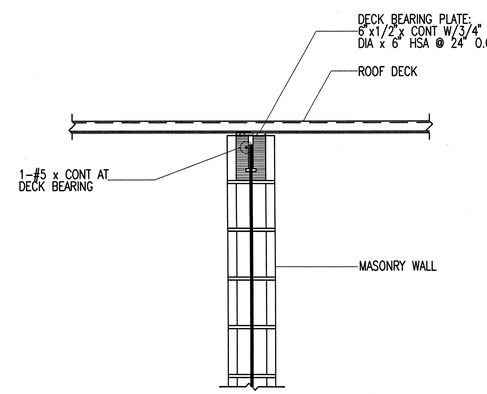
A2 SF512 — TYPICAL ROOF TOP MECHANICAL UNIT SUPPORT DETAIL (PLAN VIEW) — NO SCALE

JK2RTS08



A3 SF512 — TYPICAL HANGING MECHANICAL UNIT SUPPORT DETAIL — NO SCALE

JK1RTS15



A4 SF512 — TYPICAL DECK BEARING AT MASONRY WALL — NO SCALE

2004-210/SF512/A4

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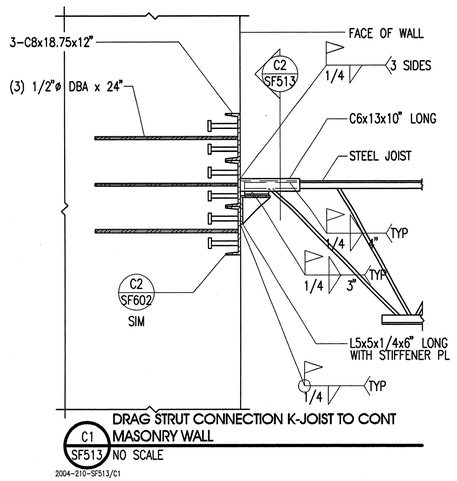
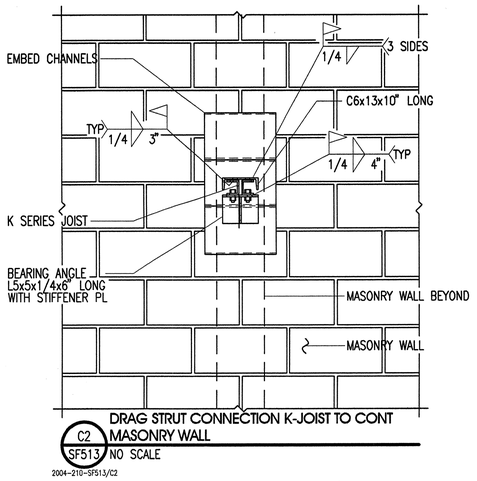


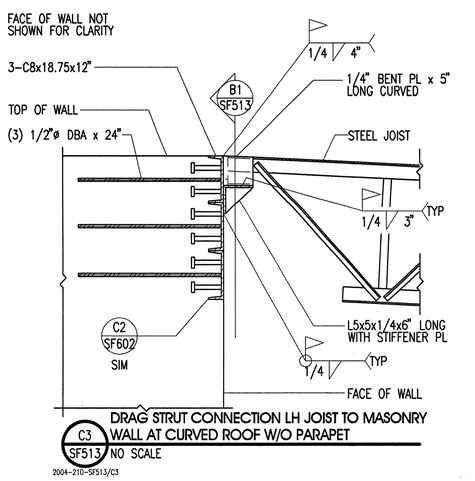
DRAG STRUT CONNECTION K-JOIST TO CONT MASONRY WALL
C1 SF513 NO SCALE

2004-210-SF513/C1



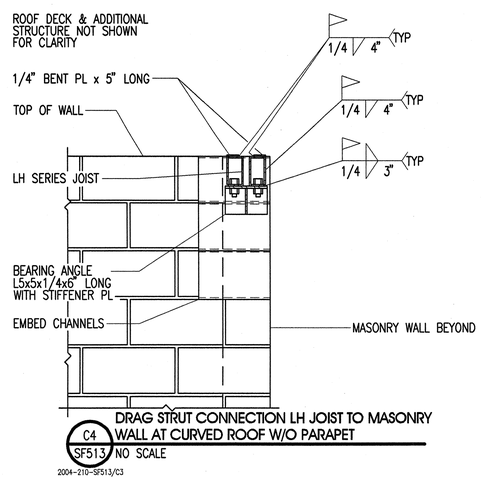
DRAG STRUT CONNECTION K-JOIST TO CONT MASONRY WALL
C2 SF513 NO SCALE

2004-210-SF513/C2



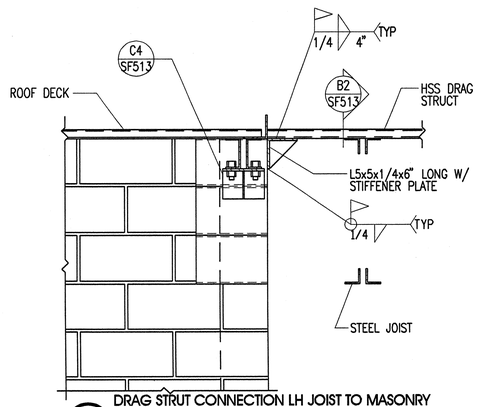
DRAG STRUT CONNECTION LH JOIST TO MASONRY WALL AT CURVED ROOF W/O PARAPET
C3 SF513 NO SCALE

2004-210-SF513/C3



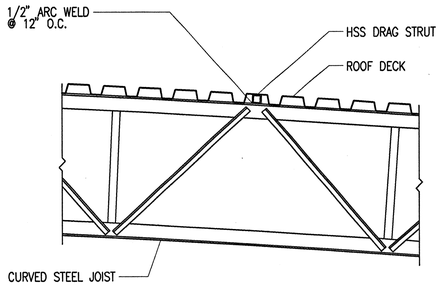
DRAG STRUT CONNECTION LH JOIST TO MASONRY WALL AT CURVED ROOF W/O PARAPET
C4 SF513 NO SCALE

2004-210-SF513/C3



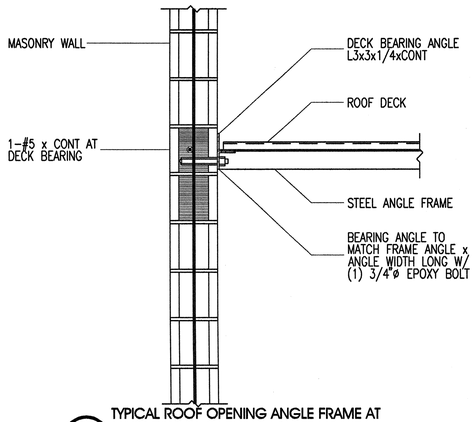
DRAG STRUT CONNECTION LH JOIST TO MASONRY WALL AT CURVED ROOF W/O PARAPET
B1 SF513 NO SCALE

2004-210-SF513/B1



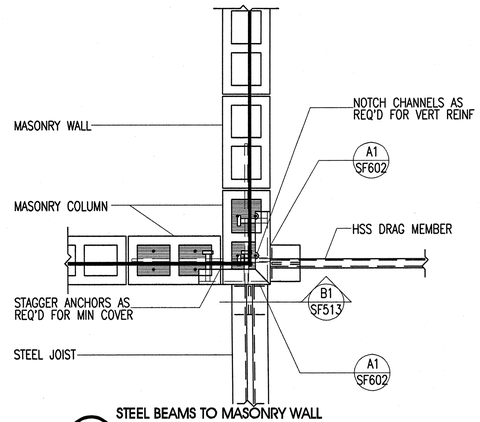
HSS DRAG STRUT PERPENDICULAR TO ROOF JOISTS
B2 SF513 NO SCALE

2004-210-SF513/B2



TYPICAL ROOF OPENING ANGLE FRAME AT MASONRY WALL
B3 SF513 NO SCALE

2004-210-SF513/B3



STEEL BEAMS TO MASONRY WALL CONNECTIONS
B4 SF513 NO SCALE

2004-210-SF513/B1

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MASONRY LINTEL SCHEDULE

ML-1

MARK	LINTEL DEPTH	LINTEL WIDTH	MASONRY TYPE	LINTEL		MAXIMUM SPAN	REMARKS
				HORIZONTAL	STIRRUPS		
ML-1	8"	8"	CMU	1- #4 CONT.	-----	3'-4"	
ML-2	16"	8"	CMU	1- #4 CONT. TOP & BOT.	#3 @ 6" O.C.	5'-4"	
ML-3	24"	8"	CMU	1- #6 CONT. TOP & BOT.	#3 @ 8" O.C.	8'-0"	
ML-4	32"	8"	CMU	1- #7 CONT. TOP & BOT.	#3 @ 8" O.C.	10'-0"	
ML-5	48"	8"	CMU	2- #6 CONT. TOP & BOT.	#3 @ 8" O.C.	---	

NOTES:

1. MASONRY LINTELS ML-1 THROUGH ML-4 SHALL BE USED OVER OPENINGS IN MASONRY WALLS WHEN A SPECIFIC MASONRY LINTEL IS NOT OTHERWISE SPECIFIED. THE MASONRY LINTEL TO BE USED SHALL BE DETERMINED BY THE MAXIMUM SPAN AS SPECIFIED IN THIS SCHEDULE. WHEN A SPECIFIC MASONRY LINTEL IS CALLED OUT ON THE PLAN, THE MAXIMUM SPAN AS NOTED IN THIS SCHEDULE SHALL NOT APPLY

2. MASONRY LINTELS ML-1 THROUGH ML-4 SHALL NOT BE LOCATED BELOW ANY FLOOR, OR ROOF BEAM, OR GIRDER, OR ANY OTHER CONCENTRATED LOAD UNLESS SHOWN SPECIFICALLY ON THE PLAN SHEET. JOISTS SHALL NOT BEAR ON ANY LINTEL LESS THAN 16" DEEP.

3. FOR MASONRY LINTELS NOT SHOWN ON THE DRAWINGS THAT CARRY ANY FLOOR, OR ROOF BEAM, OR GIRDER, OR ANY OTHER CONCENTRATED LOAD, OR THAT SPAN GREATER THAN 10'-0" CONSULT THE STRUCTURAL ENGINEER.

4. EXTEND ALL HORIZONTAL REINFORCING 48 BAR DIAMETERS BEYOND THE EDGE OF THE OPENING. IF HORIZONTAL REINFORCING CANNOT BE EXTENDED 48 BAR DIAMETERS BEYOND THE EDGE OF THE OPENING, PROVIDE 90 DEGREE STANDARD HOOK.

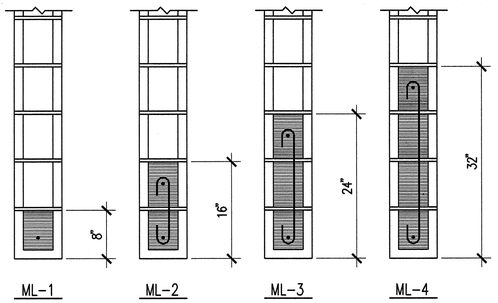
5. GROUT MASONRY LINTELS MONOLITHICALLY WITH THE SUPPORT WALL OR COLUMN AT EACH END.

6. SPLICE TOP BARS AT MIDSPAN OF LINTEL ONLY.

7. SPLICE BOTTOM BARS OVER SUPPORTS ONLY.

8. FOR WALL ABOVE LINTEL, DOWEL VERTICAL REINFORCING INTO FULL DEPTH OF THE LINTEL OR 48 BAR DIAMETERS, WHICHEVER IS LESS.

9. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING WOULD OCCUR IN THE SAME COURSE, THE LARGER BARS ARE TO REPLACE THE SMALLER BARS.



A1 SF601 ML-DET TYPICAL MASONRY LINTEL DETAILS
NO SCALE

MASONRY WALL SCHEDULE

MW-1

MARK	THICK	MATERIALS	REINFORCING			NOTES
			VERTICAL	HORIZONTAL	JOINTS	
MW-1	8"	CMU	#5 @ 32" O.C.	#5 @ 48" O.C.	--	
MW-2	8"	CMU	#5 @ 32" O.C.	#6 @ 48" O.C.	--	
MW-3	8"	CMU	#5 @ 32" O.C.	2-#5 @ 48" O.C.	--	

NOTES:

1. PROVIDE SCHEDULED VERTICAL REINFORCING BARS AT ALL CORNERS, ENDS OF WALLS, AND SPACED AS SCHEDULED, UNLESS NOTED OTHERWISE.

2. HORIZONTAL REINFORCING BARS SHALL BE CONTINUOUS AT ALL CORNERS AND AT INTERSECTING WALLS. PROVIDE CORNER BARS WITH THE REQUIRED LAP SPLICE LENGTH.

3. TERMINATE ALL HORIZONTAL REINFORCING BARS AT ENDS OF WALLS AND EDGES OF OPENINGS WITH A STANDARD HOOK AROUND VERTICAL REINFORCING BARS.

4. SEE PLANS, DETAILS AND GENERAL STRUCTURAL NOTES FOR ADDITIONAL REINFORCING REQUIREMENTS.

5. GROUT SOLID ALL CELLS BELOW GRADE, CELLS CONTAINING EMBEDS (HSA'S, DBA'S, ANCHOR BOLTS, ETC.), AND CELLS CONTAINING REINFORCING. CONSOLIDATE GROUT AS PER THE GENERAL STRUCTURAL NOTES.

6. HORIZONTAL WALL REINFORCING SHALL BE PLACED INSIDE THE VERTICALS OF MASONRY COLUMNS.

7. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, THE LARGER BARS ARE TO REPLACE THE SMALLER BARS.

MASONRY COLUMN SCHEDULE

MC-1

MARK	SIZE	REINFORCING		REMARKS
		VERTICAL	TIES	
MC-1	8"x16"	4-#5	NONE REQ'D	
MC-2	8"x24"	6-#5	NONE REQ'D	
MC-3	8"x32"	8-#5	NONE REQ'D	
MC-4	8"x40"	10-#5	NONE REQ'D	
MC-5	8"x48"	12-#5	NONE REQ'D	
MC-6	8"x54"	14-#5	NONE REQ'D	
MC-7	16"x8"x24"	8-#5	NONE REQ'D	2 BARS PER CELL SEE NOTE 4

NOTES:

1. THE CENTERLINE OF VERTICAL BARS SHALL BE LOCATED 2.1/2" FROM THE FACE OF THE MASONRY. HORIZONTAL BARS SHALL BE LOCATED TO THE INSIDE OF THE VERTICAL BARS.
2. UNLESS NOTED OTHERWISE, VERTICAL REINFORCING AND TIES SHALL EXTEND FULL HEIGHT OF THE WALL.
3. MASONRY COLUMN VERTICAL BARS OR DOWELS IN CONCRETE FOUNDATION WALLS SHALL BE TIED WITH #3 TIES @ 8" O.C.
4. SEE PLAN FOR ORIENTATION OF LONG LEG OF MC-7

MASONRY REINFORCING BAR LAP SPLICE SCHEDULE

MRBLS-1500

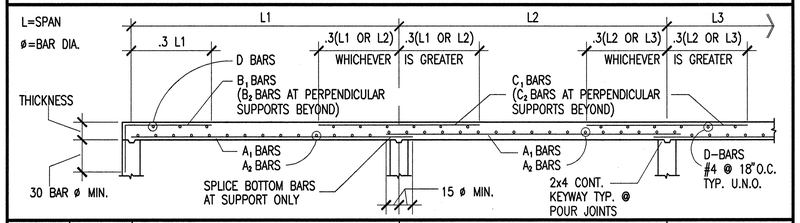
BAR SIZE	f'm = 1500 psi							f'm = 2500 psi		
	6" CMU	8" CMU		10" CMU		12" CMU		6" ATLAS	8" ATLAS	
	CLASS	CLASS		CLASS		CLASS		CLASS	CLASS	
	A	A	B	A	B	A	B	A	A	B
#3	19"	19"	19"	19"	19"	19"	19"	15"	15"	15"
#4	25"	25"	30"	25"	28"	25"	28"	20"	20"	24"
#5	39"	31"	49"	31"	45"	31"	45"	31"	24"	40"
#6	81"	57"	**	53"	92"	53"	92"	64"	45"	87"
#7	-	79"	**	61"	**	61"	**	-	63"	**
#8	-	**	**	87"	**	75"	**	-	89"	**
#9	-	-	-	**	**	90"	**	-	-	-

NOTES:

1. CLASS A SPLICES MAY BE USED WHEN ONLY ONE BAR IS CONTINUOUS IN THE MASONRY CELL OR COURSE.
2. CLASS B SPLICES SHALL BE USED WHEN TWO BARS ARE CONTINUOUS IN THE MASONRY CELL OR COURSE.
3. ** INDICATES THAT A LAP SPLICE IS NOT ALLOWED AND MECHANICAL BAR COUPLERS ARE REQUIRED FOR THE BAR SPLICES.
4. WHERE VERTICAL BARS HAVE A REQUIRED LAP SPLICE GREATER THAN THE HEIGHT OF THE GROUT POUR, THE BAR SPLICE SHALL BE MADE WITH A MECHANICAL BAR COUPLER. WHERE THE HEIGHT OF THE GROUT POUR EXCEEDS 60 INCHES, HIGH LIFT GROUTING PROCEDURES SHALL BE FOLLOWED.
5. WHERE MECHANICAL BAR COUPLERS ARE USED, THE CONNECTOR SHALL DEVELOP 125% OF THE SPECIFIED YIELD STRENGTH OF THE BAR IN TENSION AND COMPRESSION.

CONCRETE SUSPENDED SLAB SCHEDULE

CSS-1



MARK	THICK	PRIMARY REINFORCING			SECONDARY REINFORCING			D BARS
		A_1 BARS	B_1 BARS	C_1 BARS	A_2 BARS	B_2 BARS	C_2 BARS	
CSS-1	6"	#4 @ 6" O.C.	#4 @ 6" O.C.		#4 @ 6" O.C.	#4 @ 6" O.C.		

NOTES.

1. PRIMARY REINFORCING SHALL RUN PARALLEL TO THE DIRECTION OF THE ARROW (←——→).
2. PRIMARY BOTTOM REINFORCING SHALL BE PLACED BELOW SECONDARY BOTTOM REINFORCING. PRIMARY TOP REINFORCING SHALL BE PLACED ABOVE SECONDARY TOP REINFORCING.

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CAMP WG WILLIAMS
RIVERTON, UTAH

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MARK	DATE	DESCRIPTION

ISSUE DATA

ISSUE DATE:	DEC. 2005
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SF601

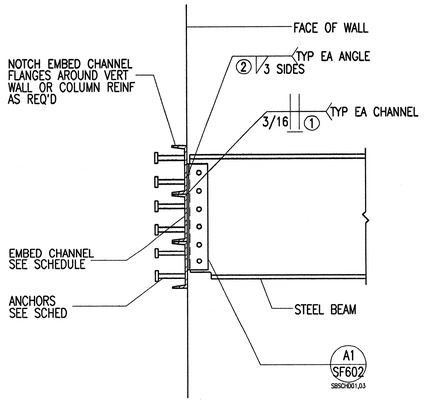
STEEL COLUMN SCHEDULE

SC-1

MARK	SIZE	STEEL BASE PLATE	STEEL CAP PLATE	REMARKS
SC-1	HSS6x6x1/4	7/8" SBP-1	7/8" SCP-1	
SC-2	HSS4x4x1/4	3/4" SBP-1	3/4" SCP-1	
SC-3	HSS6x4x5/16	3/4" SBP-3	SEE DETAIL C3/SF504	
SC-4	HSS4x4x1/4	3/4" SBP-3	3/4" SCP-1	
SC-5	HSS4x4x1/4	3/4" SBP-1	3/4" SCP-2	
SC-6	HSS4x4x5/16	SEE DETAIL C1/SF504	SEE DETAIL C1/SF504	
SC-7	HSS4x4x1/4	3/4" SBP-2	SEE DETAIL C2/SF504	
SC-8	HSS4x4x1/4	3/4" SBP-2	SEE DETAIL C3/SF504	
SC-9	HSS4x4x1/4	3/4" SBP-1	3/4" SCP-2	

NOTES:

1. UNLESS NOTED OTHERWISE, ALL COLUMNS SHALL BE INSTALLED W/4-3/4" DIAMETER A.B. W/ 3" (MIN) HOOKS. PROJECT ANCHOR BOLTS 3" (MIN) ABOVE THE TOP OF THE BASE PLATE. EMBEDMENT SHALL BE 9" (MIN). ALL BOLTS SHALL BE INSTALLED W/ HARDENED WASHERS BENEATH THE NUT. ANY BOLT HOLES LARGER THAN THE BOLT DIAMETER PLUS 5/16" SHALL HAVE 5/16" PLATE WASHERS INSTALLED BENEATH THE HARDENED WASHERS.
2. ALL BOLTS IN CAP PLATES SHALL BE 3/4" DIA, A325N BOLTS TYPICAL U.N.O.
3. COLUMN SPLICES SHALL BE LOCATED PER SPLICE DETAILS, TYPICAL, U.N.O.
4. WELDS SHALL NOT BE MADE TO ANCHOR BOLTS.

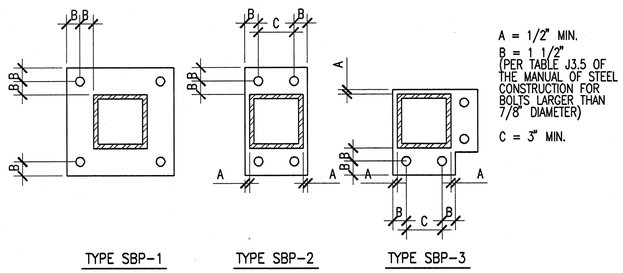


C2 / SF602 TYPICAL EMBED CHANNEL CONNECTION SCHEDULE
NO SCALE
SBSCHD13

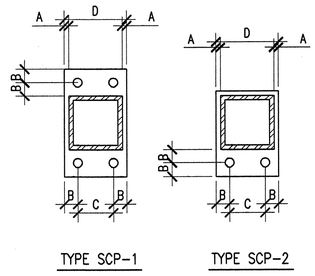
CONNECTION SCHEDULE

BEAM DEPTH	EMBED CHANNEL	ANCHORS
TO 10"	1-C8x18.75x12"	4-3/4"ø x 5" STUDS
TO 16"	2-C8x18.75x12"	8-3/4"ø x 5" STUDS
TO 24"	3-C8x18.75x12"	12-3/4"ø x 5" STUDS

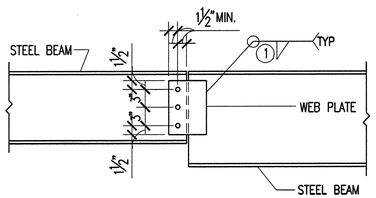
① WELD MULTIPLE EMBEDS TOGETHER WITH BUTT WELD EACH SIDE, FULL LENGTH.
② FILLET WELDS SHALL BE ANGLE THICKNESS MINUS 1/16" (1/4" MIN.)
③ FOR ALL DRAG STRUT CONNECTIONS, USE CONNECTIONS FOR BEAMS UP TO 24" DEEP.



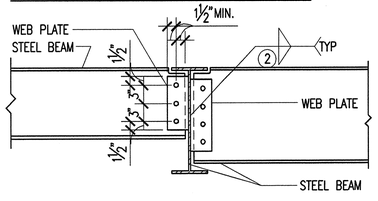
B1 / SF602 TYPICAL STEEL BASE PLATES - PLAN VIEW
NO SCALE
SBP-1



B2 / SF602 TYPICAL STEEL CAP PLATES - REFLECTED PLAN VIEW
NO SCALE
SCP-1



BEAM TO BEAM WEB PLATE CONNECTION



BEAM TO BEAM WEB PLATE CONNECTION

A325 BOLT SCHEDULE

MAXIMUM BEAM SIZE IN EACH BEAM DEPTH GROUP	WEB PLATE THICKNESS	A325N BOLTS	
		NUMBER	SIZE
W8	5/16"	2	3/4"ø
W10	5/16"	2	3/4"ø
W12	5/16"	3	3/4"ø
W14	5/16	3	3/4"ø
W16	5/16	4	3/4"ø
W18	5/16	5	3/4"ø
W21	3/8"	6	3/4"ø
W24	7/16"	7	3/4"ø
C12	5/16"	2	3/4"ø

NOTES:
① FILLET WELDS ONE SIDE SHALL EQUAL THE PLATE THICKNESS MINUS 1/16" (1/4" MIN.)
② FILLET WELDS TWO SIDES SHALL BE 3/4 THE PLATE THICKNESS (1/4" MIN.) EACH SIDE
③ BOLT EDGE DISTANCE SHALL BE 1.1/2" MIN. AT ALL EDGES. BOLT SPACING SHALL BE 3" MIN.
④ THICKNESS EQUALS BEAM FLANGE THICKNESS OF BEAM FRAMING INTO COLUMN WEB. (3/8" MIN.)

A1 / SF602 TYPICAL BOLTED WEB PLATE CONNECTIONS W/ BOLT SCHEDULE (SINGLE SHEAR)
NO SCALE
SBSCHD06

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REVISIONS

MARK	DATE	DESCRIPTION

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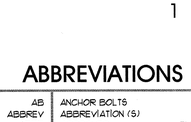
ABBREVIATIONS

AB	ANCHOR BOLTS	EA	EACH	MAINT	MAINTENANCE	SECT	SECTION
ABBREV	ABBREVIATION (S)	ELEC	ELECTRICAL	MAT'L	MATERIAL	SHT	SHEET
APPR	APPROVED	ELEV or EL	ELEVATION	MAX	MAXIMUM	SIM	SIMILAR
ACOUS	ACOUSTICAL	ELEV	ELEVATOR	MB	MACHINE BOLT	SIMUL	SIMULATED
AC	ASPHALT CONCRETE	EQ	EQUAL	MECH	MECHANICAL	SPECS	SPECIFICATIONS
A/C	AIR CONDITIONING	EQUIP	EQUIPMENT	MET	METAL	SS	STAINLESS STEEL
AD	ACCESS DOOR	EXIST	EXISTING	MEZZ	MEZZANINE	STA	STATION
AFF.	ABOVE FINISHED FLOOR	EJ	EXPANSION JOINT	MFR	MANUFACTURER		
ALUM	ALUMINUM	EXT	EXTERIOR	MISC	MISCELLANEOUS	STD	STANDARD
		EWC	ELEC WATER COOLER	MO	MASONRY OPENING	STL	STEEL
BD	BOARD	FF	FINISH FLOOR	MIN	MINIMUM	SQ	SQUARE
BLDG	BUILDING	FHC	FIRE HOSE CABINET	MULL	MULLION	STRUCT	STRUCTURAL
BLK	BLOCK	FIN	FINISH	(N)	NEW	SUSP	SUSPENDED
BM	BEAM	FLR	FLOOR	N.I.C.	NOT IN CONTRACT	SYM	SYMMETRICAL
B.O.B.	BOTTOM OF BEAM	FO	FINISHED OPENING	NO.	NUMBER		
BOT	BOTTOM	FOC	FACE OF CONCRETE	N.T.S.	NOT TO SCALE	T	TREAD
				NR	NON-RATED	TBC	TOP BACK OF CURB
C/C	CENTER TO CENTER	FOS	FACE OF STUD			TDC	TRAFFIC DECK COVERING
CER	CERAMIC	FOW	FACE OF WALL	OC	ON CENTER	T & G	TONGUE & GROOVE
CH	CEILING HEIGHT	FDN	FOUNDATION	OCC.	OPERATOR CONTROL CENTER	TEMP	TEMPERED
CEM	CEMENT	FRP	FIBER REINFORCED PLASTIC	OD	OUTSIDE DIAMETER	TEL	TELEPHONE
CLG	CEILING	FD	FLOOR DRAIN	OF	OVERFLOW	THK	THICK
CLOS.	CLOSET	FS	FLOOR SINK	OFF	OFFICE	T.O.C.	TOP OF CONCRETE or CURB
CLR	CLEAR	FS	FULL SIZE	OFI	OWNER FURNISH ITEM	T.O.F.	TOP OF FOOTING
CMU	CONCRETE MASONRY UNIT	FSD	FULL SIZE DETAIL	OFOI	OWNER FURNISHED	T.O.P.	TOP OF PARAPET
COL	COLUMN	FT.	FEET		OWNER INSTALLED	T.O.S.	TOP OF STEEL
COMPO	COMPOSITION	FH	FULL HEIGHT	OH	OPPOSITE HAND	TYP	TYPICAL
CONC	CONCRETE			OPNG	OPENING	T.O.W.	TOP OF WALL
CONN	CONNECTION	GA	GAUGE	OPP	OPPOSITE	UNO	UNLESS NOTED OTHERWISE
CONT	CONTINUOUS	GC	GENERAL CONTRACTOR				
CORR	CORRIDOR	GL	GLASS	PC	PORTLAND CEMENT	VER	VERTICAL
CSK	COUNTERSINK	GALV or GV	GALVANIZED	PL	PLATE		
		GR	GRADE	PLAS	PLASTER	W	WIDE
DEL	DELETE	GYP BD	GYPSUM BOARD	PLYWD	PLYWOOD	W	WITH
DET	DETAIL	GSD	GRANITE SCHOOL DISTRICT	PR	PAIR	WC	WATER CLOSET
DF	DOUGLAS FIR			PREP	PREPARATION	WD	WOOD
DF	DRINKING FOUNTAIN	H	HIGH	PROJ	PROJECTION	WI	WROUGHT IRON
DIA	DIAMETER	HDWD	HARDWOOD	PT	PRESSURE TREATED	WL	WATER LEVEL
DIAG	DIAGONAL	HORIZ	HORIZONTAL	PTDF	PRESSURE TREATED DOUGLAS FIR	WP	WORKING POINT
DIM	DIMENSION	HM	HOLLOW METAL			WP	WATERPROOFING
DIR	DIRECTION	HP	HIGH POINT				
DBL	DOUBLE			QT	QUARRY TILE		
DN	DOWN	ID	INSIDE DIAMETER				
DO	DOOR OPENING	INSUL	INSULATION	R	RISER		
DR	DOOR	INT	INTERIOR	R or RAD	RADIUS		
DS	DOWNSPOUT			RD	ROOF DRAIN		
DWG	DRAWING	JAN	JANITOR	REF	REFERENCE		
				REFL	REFLECTED		
		LAV	LAVATORY	REINF	REINFORCE		
		LEV	LEVEL	REQ'D	REQUIRED		
		LTG	LIGHTING	RES	RESILIENT		
		LTWT	LIGHT WEIGHT	RM	ROOM		
				RO.	ROUGH OPENING		
				REDWD	REDWOOD		

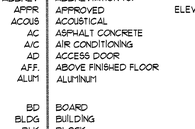
MATERIALS LEGEND (TYPICAL PLAN VIEWS)

NEW CONSTRUCTION

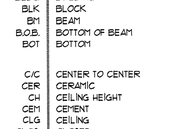
ALL NEW CONSTRUCTION IS SHOWN HATCHED AS ILLUSTRATED BELOW
ALL NEW CONSTRUCTION IS FULLY NOTED AS SHOWN BELOW
ALL WORK IS NEW UNLESS LABELED "EXISTING"



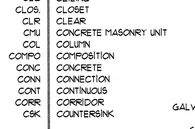
METAL OR WOOD STUD FRAMED WALL
- ACTUAL WIDTH OF WALL DIMENSIONED
- WIDTH OF WALL DIMENSIONED TO FACE OF STUD
- DOORS DIMENSIONED TO NOMINAL DOOR SIZE EXCLUDING FRAME
- WINDOWS DIMENSIONED TO NOMINAL FRAME SIZE - NOT ROUGH OPENING



METAL OR WOOD STUD FRAMED WALL W/ MASONRY OR STONE VENEER
- WIDTH OF WALL DIMENSIONED FROM FACE OF STUD TO OVERALL NOMINAL WIDTH.
- WIDTH OF WALL DIMENSIONED TO FACE OF STUD AT INTERIOR.
- DOORS DIMENSIONED TO NOMINAL DOOR SIZE EXCLUDING FRAME
- WINDOWS DIMENSIONED TO NOMINAL FRAME SIZE - NOT ROUGH OPENING



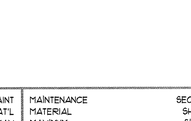
MASONRY WALL
- WIDTH OF WALL DIMENSIONED TO NOMINAL SIZE.
- DOORS DIMENSIONED TO NOMINAL DOOR SIZE EXCLUDING FRAME
- WINDOWS DIMENSIONED TO NOMINAL FRAME SIZE - NOT ROUGH OPENING



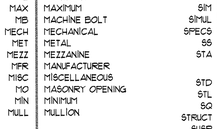
CONCRETE WALL
- ACTUAL WIDTH OF WALL DIMENSIONED
- DOORS DIMENSIONED TO NOMINAL DOOR SIZE EXCLUDING FRAME
- WINDOWS DIMENSIONED TO NOMINAL FRAME SIZE - NOT ROUGH OPENING

EXISTING CONDITIONS

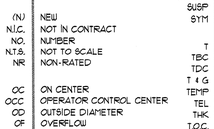
ALL EXISTING CONSTRUCTION IS SHOWN WITHOUT HATCH AS ILLUSTRATED BELOW
ALL EXISTING CONSTRUCTION IS LABELED "EXISTING" BY KEYNOTE OR OTHERWISE
ALL DIMENSIONS TO EXISTING ARE APPROXIMATE ONLY AND ARE TO BE FIELD VERIFIED BY THE CONTRACTOR



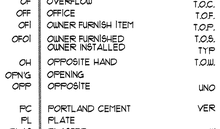
EXISTING
METAL OR WOOD STUD FRAMED WALL
- WIDTH OF WALL DIMENSIONED TO FINISHED FACE.
- DOORS DIMENSIONED TO NOMINAL DOOR SIZE EXCLUDING FRAME
- WINDOWS DIMENSIONED TO NOMINAL FRAME SIZE



EXISTING
METAL OR WOOD STUD FRAMED WALL W/ MASONRY OR STONE VENEER
- WIDTH OF WALL DIMENSIONED FROM FINISH FACE TO OVERALL NOMINAL WIDTH.
- DOORS DIMENSIONED TO NOMINAL DOOR SIZE EXCLUDING FRAME
- WINDOWS DIMENSIONED TO NOMINAL FRAME SIZE



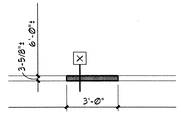
EXISTING
MASONRY WALL (TO REMAIN)
- WIDTH OF WALL DIMENSIONED TO NOMINAL SIZE.
- DOORS DIMENSIONED TO NOMINAL DOOR SIZE EXCLUDING FRAME
- WINDOWS DIMENSIONED TO NOMINAL FRAME SIZE



EXISTING
CONCRETE WALL (TO REMAIN)
- NOMINAL WIDTH OF WALL DIMENSIONED
- DOORS DIMENSIONED TO NOMINAL DOOR SIZE EXCLUDING FRAME
- WINDOWS DIMENSIONED TO NOMINAL FRAME SIZE

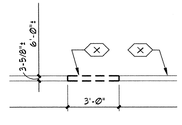
NEW CONSTRUCTION AT EXISTING

NEW CONSTRUCTION SHOWN HATCHED W/ THICKER LINE
ALL EXISTING COMPONENTS SHOWN THIN LINE SOLID, NO HATCH.



DEMOLITION AT EXISTING CONDITIONS

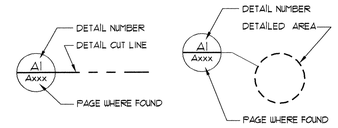
ALL EXISTING COMPONENTS SHOWN THIN LINE SOLID, NO HATCH.
ALL COMPONENTS TO BE DEMOLISHED ARE SHOWN DASHED, THIN LINE



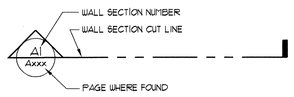
SYMBOLS

ANGLE
CHANNEL
PLATE
CENTER LINE
DIAMETER or ROUND
SQUARE
WORK POINT OR ELEV BENCH MARK
NUMBER
AND
AT
DELTA
PENNY

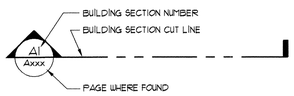
DETAIL TAGS



DETAIL TAGS



WALL SECTION TAG



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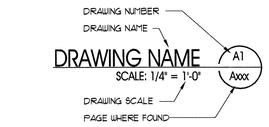

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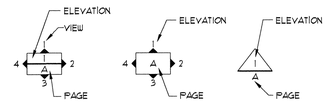
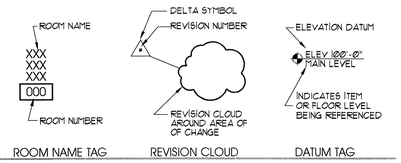


INTERIOR ELEVATION TAGS

WALL TYPE DESIGNATION.
KEYNOTE.
WALL, FLOOR, BASE, CEILING FINISH KEYNOTE.
EXISTING/DEMO KEYNOTE.
DOOR DESIGNATION. SEE DOOR SCHEDULE.
WINDOW DESIGNATION. SEE WINDOW SCHEDULE.

MISCELLANEOUS KEYED NOTE SYMBOLS



ROOM NAME TAG | REVISION CLOUD | DATUM TAG

GENERAL NOTES:

0.1 THE GENERAL CONTRACTOR SHALL HEREAFTER BE REFERRED TO AS "GENERAL CONTRACTOR" OR "GC". THE OWNER MAY HEREAFTER BE REFERRED TO AS "OWNER"

0.2 PLEASE NOTE:
THE GENERAL CONTRACTOR IS RESPONSIBLE FOR ALL WORK REGARDLESS OF THE LOCATION OF THE INFORMATION IN THE DOCUMENTS. THE GENERAL CONTRACTOR SHALL UTILIZE THE CONSTRUCTION DRAWINGS AND WRITTEN SPECIFICATIONS FOR ALL REQUIRED INFORMATION TO PROVIDE COMPLETE CONSTRUCTION OF THIS PROJECT. ITEMS LISTED IN DRAWINGS MAY NOT BE INCLUDED IN SPECIFICATIONS. ITEMS LISTED IN SPECIFICATIONS MAY NOT BE INCLUDED IN DRAWINGS.

0.3 UNLESS OTHERWISE INDICATED IN THE CONSTRUCTION DOCUMENTS AS BEING NOT IN CONTRACT (N.I.C.) OR EXISTING, ALL ITEMS, MATERIALS AND INSTALLATION OF SAME ARE PART OF THE CONTRACT AS DEFINED BY THE CONSTRUCTION DOCUMENTS. THE GC SHALL PROVIDE AND INSTALL ALL ACCESSORIES, COMPONENTS AND ASSEMBLIES REQUIRED FOR THE WORK DEPICTED OR SPECIFIED.

0.4 THE GENERAL CONTRACTOR SHALL FIELD VERIFY ALL CONDITIONS AND DIMENSIONS PRIOR TO BEGINNING ANY WORK AND SHALL BE RESPONSIBLE FOR ALL WORK AND MATERIALS INCLUDING THOSE FURNISHED BY SUBCONTRACTORS. THE GC SHALL ACCEPT PREMISES AS FOUND. OWNER WILL MAINTAIN THE EXISTING CONDITION OF THE SITE AND EXISTING STRUCTURES AT THE TIME OF BIDDING.

0.5 DISCREPANCIES BETWEEN PORTIONS OF THE CONTRACT DOCUMENTS ARE NOT INTENDED. THE GENERAL CONTRACTOR IS TO CLARIFY WITH THE ARCHITECT ANY SUCH DISCREPANCIES PRIOR TO COMMENCING WORK.

0.6 DIMENSIONS TAKE PRECEDENCE OVER DRAWINGS; DO NOT SCALE DRAWINGS TO DETERMINE ANY LOCATIONS. THE ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCY PRIOR TO CONTINUING WITH WORK.

0.7 ALL PLAN DIMENSIONS ARE FROM GRIDLINE OR FACE OF STUD OR FACE OF BLOCK UNLESS OTHERWISE INDICATED. SEE SECTION ON "DIMENSIONING" THIS SHEET.

0.8 THE CONTRACTOR SHALL REPORT TO THE ARCHITECT ALL CONDITIONS REQUIRING COORDINATION/ CHANGES WITH THE CONTRACT DOCUMENTS. COORDINATION / APPROVAL SHALL TAKE PLACE BEFORE THE WORK BEGINS. ALL CHANGES TO THE CONTRACT COST SHALL BE APPROVED THROUGH A CHANGE ORDER

0.9 DETAILED DRAWINGS AND LARGER SCALE DRAWINGS TAKE PRECEDENCE OVER SMALL SCALE DRAWINGS.

0.10 THE ARCHITECT WILL REVIEW SHOP DRAWINGS AND SAMPLES FOR CONFORMANCE WITH DESIGN CONCEPT OF THE PROJECT. THE ARCHITECT'S REVIEW OF A SEPARATE ITEM SHALL NOT INDICATE APPROVAL OF AN ASSEMBLY IN WHICH THE ITEM FUNCTIONS. THE ARCHITECT WILL NOT REVIEW SHOP DRAWINGS UNTIL THE GC HAS REVIEWED AND STAMPED THE SHOP DRAWING/SUBMITTAL. THE GC IS RESPONSIBLE FOR FIELD VERIFYING ALL DIMENSIONS SHOWN ON THE SHOP DRAWINGS. THE ARCHITECT'S REVIEW OF THE SHOP DRAWINGS SHALL NOT OVERRIDE THE CONDITIONS DESCRIBED IN THE CONTRACT DOCUMENTS UNLESS SPECIFICALLY NOTED OTHERWISE BY THE ARCHITECT

0.11 FOR CONSTRUCTION DETAILS NOT SHOWN, USE THE MANUFACTURER'S STANDARD DETAILS OR APPROVED SHOP DRAWINGS / DATA SHEETS IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

0.12 THE GENERAL CONTRACTOR SHALL REFER AND CONFORM TO ALL RECOMMENDATIONS AND FINDINGS AS SET FORTH IN THE GEOTECHNICAL REPORT. THE ARCHITECT ACCEPTS NO RESPONSIBILITY FOR THE ACCURACY OF THE FINDINGS OR FOR THE FINAL RECOMMENDATIONS.

0.13 FOR GRADING, TRENCHING ETC., CONTACT THE ARCHITECT FOR INSTRUCTIONS PRIOR TO THE CONTINUATION OF WORK SHOULD ANY UNUSUAL CONDITIONS BECOME APPARENT DURING GRADING OR FOUNDATION CONSTRUCTION. EXISTING ELEVATIONS AND LOCATIONS TO BE JOINED SHALL BE VERIFIED BY THE GENERAL CONTRACTOR BEFORE CONSTRUCTION.

1.1 ALL WORK, MATERIALS AND METHODS SHALL BE IN CONFORMANCE WITH THE CODES, ORDINANCES AND REGULATIONS OF ALL GOVERNMENTAL AGENCIES HAVING JURISDICTION AT THE PROJECT LOCATION. THE GENERAL CONTRACTOR MUST COMPLY WITH THE CONTRACTOR REGISTRATION REQUIREMENTS OF ALL GOVERNING AUTHORITIES.

1.2 ALL PROJECT CONSTRUCTION SHALL CONFORM WITH ANSI A-117.1-1998, AND THE AMERICANS WITH DISABILITIES ACT (ADA).

1.3 THE GENERAL CONTRACTOR SHALL NOTIFY ALL APPLICABLE LOCAL GOVERNING AUTHORITIES AND UTILITY COMPANIES PRIOR TO COVERING UP ANY WORK REQUIRING INSPECTION.

1.4 THE GENERAL CONTRACTOR SHALL MAINTAIN ALL REQUIRED EXITS AND FIRE LANES IN WORKING ORDER.

1.5 THE GENERAL CONTRACTOR SHALL PROVIDE AND INSTALL FIRE EXTINGUISHERS. GOVERNING CODES AND THE FIRE MARSHALL HAVING JURISDICTION SHALL DICTATE SIZE, TYPE, QUANTITY AND LOCATIONS OF BOTH TEMPORARY AND PERMANENT FIRE EXTINGUISHERS

1.6 MINIMUM FLAME SPREAD CLASSIFICATION OF INTERIOR FINISH SHALL CONFORM TO THE BUILDING CODE AND LOCAL GOVERNING BUILDING CODES/ORDINANCES.

2.1 THE GENERAL CONTRACTOR SHALL PROVIDE AND IS SOLELY RESPONSIBLE AND LIABLE FOR PUBLIC AND EMPLOYEE PROTECTION AS NECESSARY AND AS REQUIRED BY THE CODES, INCLUDING EXTERIOR AND INTERIOR PEDESTRIAN TRAFFIC BARRIERS. ALL WORK SHALL CONFORM TO THE ORDINANCES AND REGULATIONS OF GOVERNMENTAL AGENCIES HAVING JURISDICTION AT THE PROJECT.

2.2 THE GENERAL CONTRACTOR SHALL PROVIDE TEMPORARY BARRICADES FOR DUST AND NOISE CONTROL WHERE WORK JOINS EXISTING CONDITIONS.

2.3 ALL DEBRIS SHALL BE REMOVED FROM PREMISES AND ALL AREAS SHALL BE LEFT IN A CLEAN (BROOM) CONDITION AT ALL TIMES.

2.4 IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO LOCATE ALL EXISTING UTILITIES, WHETHER SHOWN HEREIN OR NOT, AND TO PROTECT THEM FROM DAMAGE. THE GENERAL CONTRACTOR SHALL BEAR ALL EXPENSE OF REPAIR OR REPLACEMENT OF UTILITIES OR OTHER PROPERTY DAMAGED BY OPERATIONS IN CONJUNCTION WITH THE PROSECUTION OF THE WORK.

2.5 APPROVED PLANS SHALL BE KEPT IN A PLAN BOX AND SHALL NOT BE USED BY WORKMEN. ALL CONSTRUCTION SETS SHALL REFLECT THE SAME INFORMATION. THE GENERAL CONTRACTOR SHALL ALSO MAINTAIN IN GOOD CONDITION, ONE COMPLETE SET OF PLANS WITH ALL REVISIONS, ADDENDA AND CHANGE ORDERS, ON THE PREMISES AT ALL TIMES. THESE ARE TO BE KEPT UNDER THE CARE OF THE JOB SUPERINTENDENT.

2.6 THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR AND SHALL REPLACE OR REMEDY ANY FAULTY, IMPROPER OR INFERIOR MATERIALS OR WORKMANSHIP WHICH SHALL APPEAR WITHIN ONE (1) YEAR (OR AS OTHERWISE SPECIFIED) FOR A SPECIFIC COMPONENT AFTER THE DATE OF SUBSTANTIAL COMPLETION EXCEPTION: THE ROOFING SUBCONTRACTOR SHALL FURNISH AND MAINTAIN AN AGREEMENT CO-SIGNED BY THE GENERAL CONTRACTOR TO MAINTAIN THE ROOFING IN A WATERTIGHT CONDITION FOR A PERIOD AS LISTED IN THE SPECIFICATIONS, STARTING AFTER DATE OF SUBSTANTIAL COMPLETION.

2.7 THE GENERAL CONTRACTOR SHALL PROVIDE A 6' HIGH (MIN) CHAIN LINK (MIN) JOB FENCE FOR SECURITY AT THE CONSTRUCTION SITE DURING ALL PHASES OF CONSTRUCTION. THE CONSTRUCTION FENCE SHALL BE GATED AND LOCKABLE. THE FENCE SHALL BE DURABLE ENOUGH TO WITHSTAND THE SPECIFIC JOB SITE CONDITIONS.

2.8 CONTRACTOR SHALL PROVIDE TEMPORARY WATER, POWER, TOILET FACILITIES AND ON SITE PHONE OR CELLULAR.

2.9 THE CONTRACTOR IS TO PROVIDE BLOCKING AS REQUIRED FOR MOUNTING OF WALL MOUNTED SHELVES, CABINETS, HC GRAB BARS AND PARTITION BRACES. BLOCKING SHALL BE FIRE TREATED WHERE REQUIRED BY THE BUILDING CODE.

2.10 THE GENERAL CONTRACTOR IS RESPONSIBLE FOR RECEIVING, UNLOADING, UNCRATING, INSTALLATION AND HOOK-UP OF ALL OWNER FURNISHED ITEMS UNLESS NOTED OTHERWISE ON THE DRAWINGS.

2.11 WHERE GAS SERVICE IS BEING PROVIDED, THE GC SHALL PROVIDE AN OUTSIDE GAS SHUT-OFF VALVE CONSPICUOUSLY MARKED.

2.12 ALL EXPOSED ELECTRICAL EQUIPMENT SHALL BE PAINTED TO MATCH ADJACENT SURFACES.

2.13 THE GENERAL CONTRACTOR IS TO ASSURE THAT NO REBAR OR REINFORCEMENT IS PRESENT PRIOR TO CORE DRILLING OR PLACING BOLTS OR ANY OTHER ITEM WHICH COULD DISTURB THE STRUCTURAL SLAB OR FOUNDATION WALLS.

2.14 PROVIDE GALVANIC PROTECTION BETWEEN DISSIMILAR MATERIALS WHERE REQUIRED.

2.15 PROVIDE METAL TRIM OF CASING AT ALL EDGES OF PLASTER AND DRYWALL SURFACES WHERE IT TERMINATES OR MEETS ANY OTHER MATERIAL, UNLESS NOTED OTHERWISE.

2.16 PROVIDE METAL CORNER TRIM AT ALL CORNERS (INSIDE AND OUT) OF PLASTER AND DRYWALL SURFACES.

2.17 ALL PENETRATIONS THROUGH ANY SURFACE SHALL BE THOROUGHLY SEALED WITH APPROPRIATE SEALANT MATERIAL.

2.18 UNLESS OTHERWISE NOTED, ALL EXTERIOR AND INTERIOR METAL, TRIM, TREILLAGE, RAILINGS, MOLDINGS, FRAMES, CASTING ETC., SHALL BE PAINTED.

2.19 FOR PLUMBING, FIRE SPRINKLER AND ELECTRICAL SYSTEMS, PROVIDE APPROVED ASSEMBLIES WITH SELF CLOSING DEVICES FOR ANY PENETRATIONS IN RATED CONSTRUCTION.

2.20 THE GC SHALL VERIFY LOCATIONS OF ALL CEILING ACCESS PANELS WITH MECHANICAL, FIRE SPRINKLER AND PLUMBING PLANS. ACCESS PANELS SHALL BE FURNISHED AND INSTALLED WITH A FIRE RATING EQUAL TO THE WALL OR CEILING ASSEMBLY INTO WHICH THEY ARE TO BE INSTALLED. FINISH AND LOCATION SHALL BE APPROVED BY THE ARCHITECT.

2.21 THE GC SHALL VERIFY DIMENSIONS OF ALL EQUIPMENT PADS & BASES WITH EQUIPMENT MANUFACTURERS & SHALL VERIFY ALL SIZES AND LOCATIONS OF DUCT OPENINGS ON ROOF AND INTERIOR SHAFTS.

MATERIALS LEGEND (TYPICAL SECTION VIEWS)

METAL
EARTH
CONCRETE
CONCRETE MASONRY UNIT
FINISHED WOOD MEMBER
PLYWOOD
BATT INSULATION
GYP BOARD
CONTINUOUS WOOD MEMBER
WOOD BLOCKING
PLASTER OR STUCCO
RIGID INSULATION

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STATE OF UTAH · KURT R. KRIEG · 175095 · 12/16/05 · LICENSED ARCHITECT

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

GENERAL NOTES
LEGENDS
SYMBOLS &
ABBREVIATIONS

REVISIONS

MARK	DATE	DESCRIPTION

ISSUE DATA

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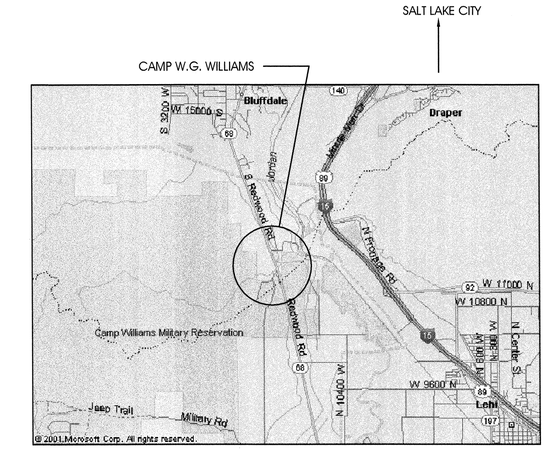
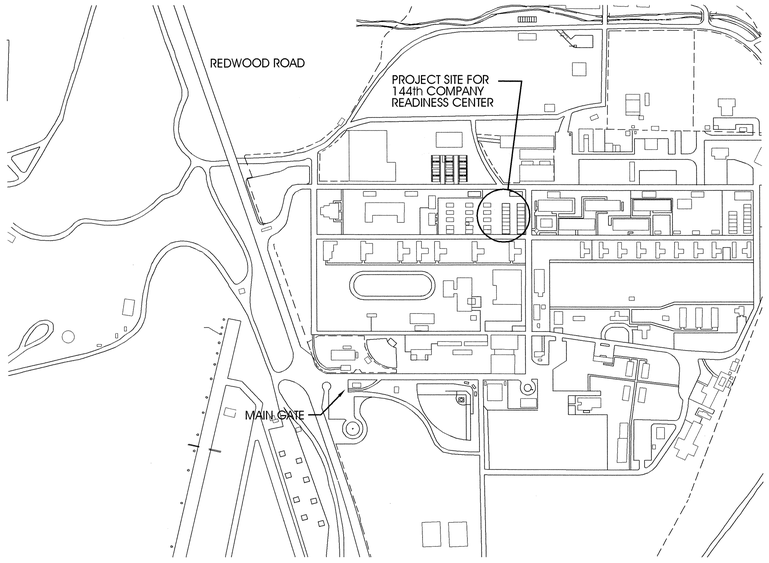
CAD FILE NAME:	0470GI002
DFCM PROJECT #	04042480

SHEET NUMBER:

GI002



LOCATION MAP
SCALE: NO SCALE
C5
GI003



SITE MAP
SCALE: NO SCALE
A5
GI003

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CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

LOCATION MAP
SITE MAP

REVISIONS

MARK	DATE	DESCRIPTION

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DFCM PROJECT #	04042480

SHEET NUMBER:

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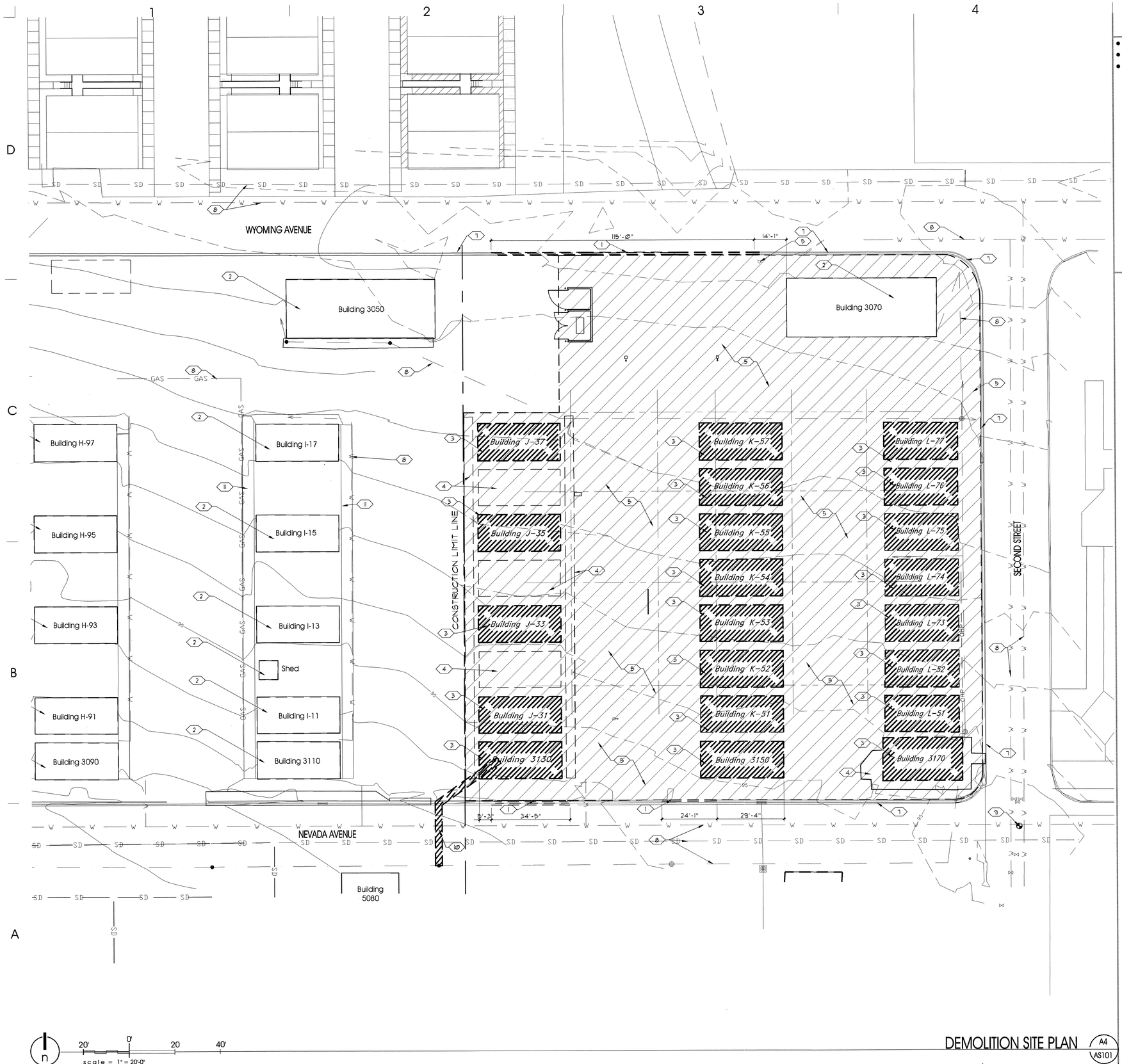

DEMOLITION SITE PLAN A4 AS101

GENERAL NOTES AND LEGEND:
FOR SHEET AS101 ONLY.

- SEE SHEET G002 FOR GENERAL NOTES.
- SEE COVER SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.

KEYED NOTE LEGEND:
FOR SHEET AS101 ONLY.

1. EXISTING CURB TO BE DEMOLISHED BY NATIONAL GUARD PERSONNEL PRIOR TO GC COMMENCING WORK.
2. EXISTING BUILDING TO REMAIN - PROTECT.
3. EXISTING BUILDING TO BE DEMOLISHED, BY NATIONAL GUARD PERSONNEL PRIOR TO COMMENCEMENT OF WORK BY GENERAL CONTRACTOR.
4. EXISTING CONCRETE PAVING TO BE REMOVED.
5. EXISTING ASPHALT SHOWN HATCHED TO BE REMOVED.
6. EXISTING FIRE HYDRANT TO REMAIN - PROTECT.
7. EXISTING CURB AND GUTTER TO REMAIN PROTECT.
8. EXISTING UTILITY LINES - SEE CIVIL DRAWINGS.
9. SURVEY MARKER - SEE CIVIL DRAWINGS.
10. SAW CUT & REMOVE EXISTING ASPHALT PAVING AS REQUIRED FOR NEW UNDERGROUND POWER CONDUITS TO NEW BUILDING. SEE ELECTRICAL SITE PLAN, SHEET E201
11. EXISTING CONCRETE PAVING TO REMAIN.

NOTE: ALL EXISTING BUILDINGS ON SITE INDICATED TO BE DEMOLISHED WILL BE DONE SO BY UTNG PERSONNEL PRIOR TO COMMENCEMENT OF WORK BY GENERAL CONTRACTOR. ALL HAZARDOUS MATERIALS ON SITE WILL BE REMOVED BY UTNG PERSONNEL PRIOR TO COMMENCEMENT OF WORK BY GENERAL CONTRACTOR. IF ANY HAZARDOUS MATERIALS ARE FOUND, GENERAL CONTRACTOR TO CONTACT OWNER IMMEDIATELY. REFER TO KEYED NOTE LEGEND ABOVE.

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RIVERTON, UTAH

SHEET NAME:

DEMOLITION SITE PLAN

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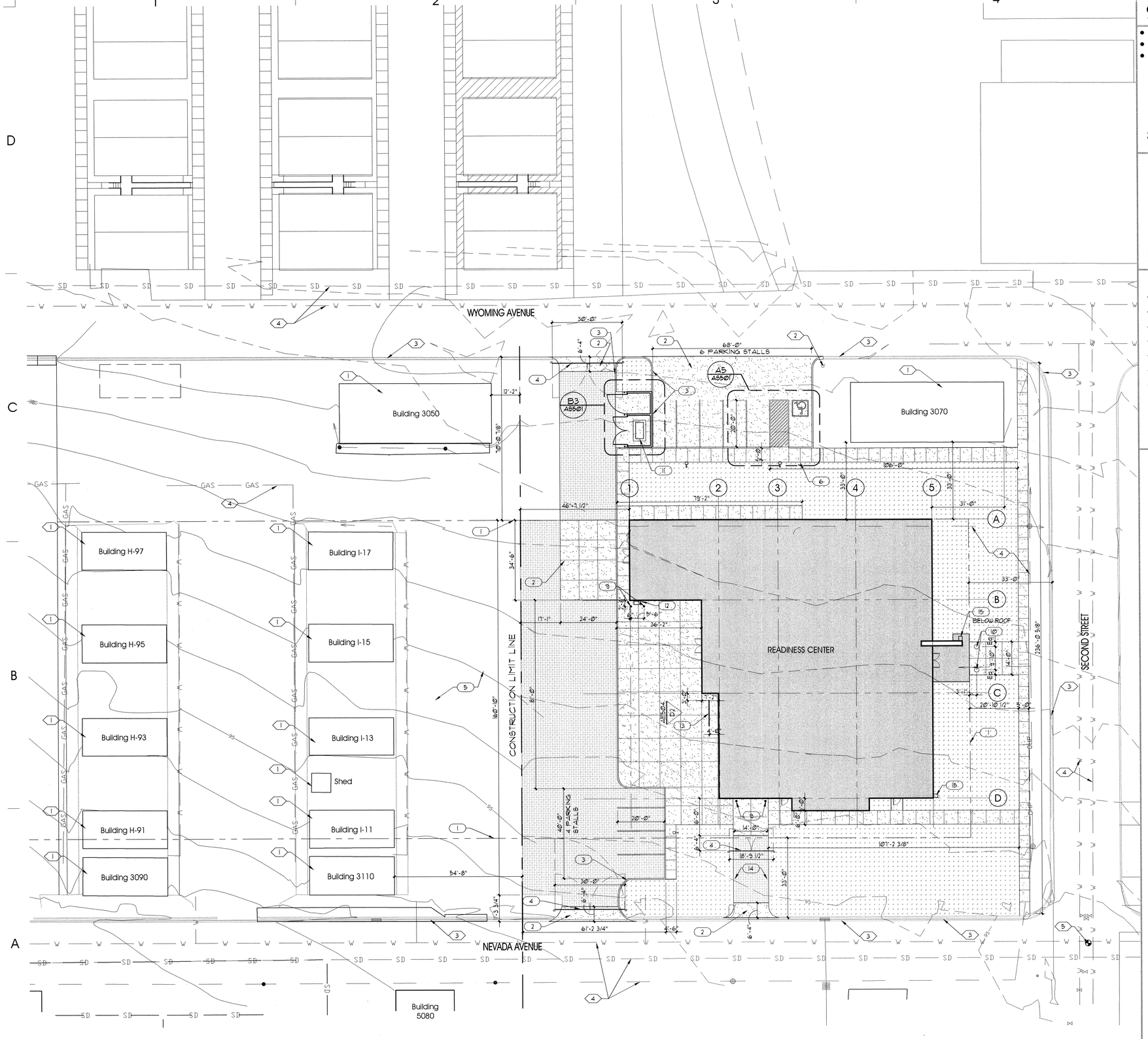

CONSTRUCTION SITE PLAN A4 AS102

GENERAL NOTES AND LEGEND:

*FOR SHEET AS1Ø2 ONLY.

- SEE SHEET G1Ø2 FOR GENERAL NOTES.
- SEE COVER SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.

SITE AREA LEGEND:

*FOR SHEET AS1Ø2 ONLY.

- IMPERVIOUS SURFACE AREA (CONCRETE)
- IMPERVIOUS SURFACE AREA (ASPHALT)
- LANDSCAPE AREA
- NEW CURB/GUTTER

EXISTING KEYED NOTE LEGEND:

*FOR SHEET AS1Ø2 ONLY.

1. EXISTING BUILDING TO REMAIN - PROTECT.
2. EXISTING FIRE HYDRANT TO REMAIN - PROTECT.
3. EXISTING CURB AND GUTTER TO REMAIN PROTECT.
4. EXISTING UTILITY LINES - SEE CIVIL DRAWINGS.
5. SURVEY MARKER - SEE CIVIL DRAWINGS.
6. EXISTING ASPHALT PAVING.
7. SAWCUT & REMOVE EXISTING ASPHALT PAVING

NEW CONSTRUCTION KEYED NOTE LEGEND:

*FOR SHEET AS1Ø2 ONLY.

1. REQUIRED SETBACK PER D.O.D.
2. CONCRETE APRON, SEE D5/AE55Ø1 FOR JOINT REQUIREMENTS.
3. NEW CURB AND GUTTER.
4. CONTROLLED ACCESS GATE.
5. CONSTRUCTION STAGING AREA.
6. ACCESSIBLE PARKING SIGN.
7. NEW GAS METER.
8. NEW SITE LIGHTING. SEE ELECTRICAL DRAWINGS.
9. BOLLARD SEE DETAIL C1/AE5Ø5.
10. CONCRETE BARRIER - SEE DETAIL B2/A55Ø1.
11. EMERGENCY GENERATOR ON 4" CONC. HOUSEKEEPING PAD - SEE ELECTRICAL DRAWINGS AND SPECIFICATIONS.
12. LOCATION OF GAS METER - SEE MECHANICAL DRAWINGS AND SPECIFICATIONS.
13. BIKE RACK - SEE ELEVATION INDICATED. COORDINATE EXACT LOCATION WITH UTNG PERSONNEL.
14. MOW STRIP - SEE CIVIL DRAWINGS.
15. PRECAST CONCRETE SPLASHBLOCK 18x24.

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SHEET NAME:

CONSTRUCTION SITE PLAN

REVISIONS

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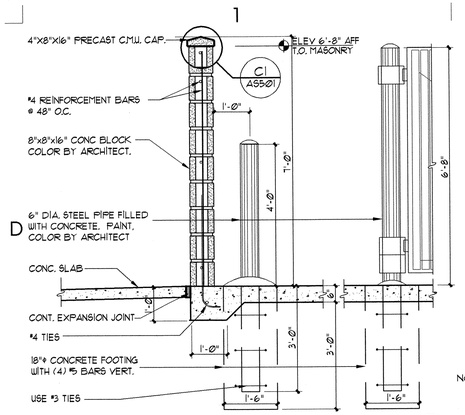
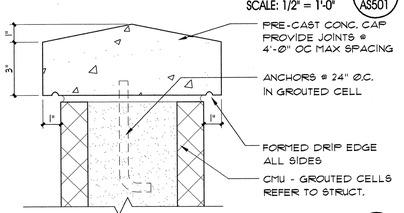
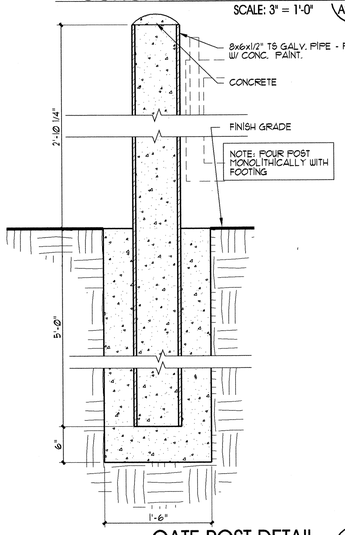
AS102



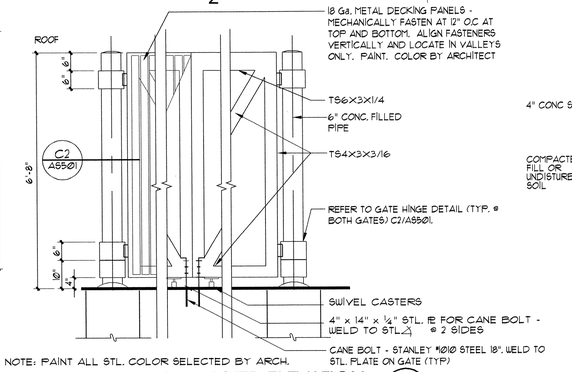
DUMPSTER DETAIL D1 / AS501
SCALE: 1/2" = 1'-0"



CONC. WALL CAP DETAIL C1 / AS501
SCALE: 3" = 1'-0"

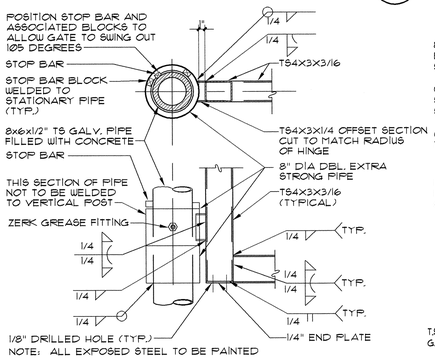


GATE POST DETAIL B1 / AS501
SCALE: 1" = 1'-0"



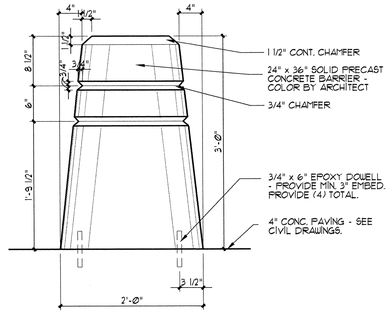
NOTE: PAINT ALL STL. COLOR SELECTED BY ARCH.

DUMPSTER ELEVATION D2 / AS501
SCALE: 1/2" = 1'-0"



NOTE: ALL EXPOSED STEEL TO BE PAINTED

GALV. STL. DUMPSTER GATE DETAIL C2 / AS501
SCALE: 1" = 1'-0"



CONCRETE BARRIER DETAIL B2 / AS501
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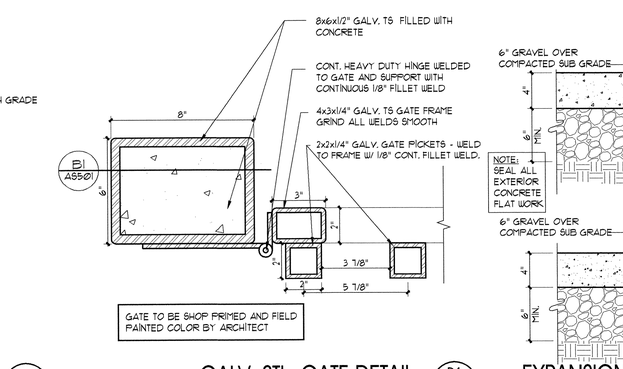
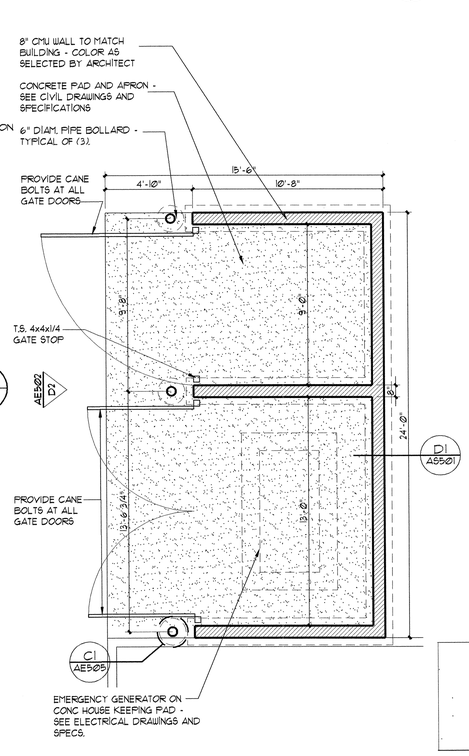
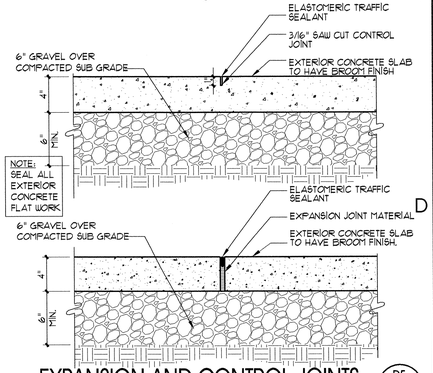


LANDSCAPE DETAIL D3 / AS501
SCALE: 1" = 1'-0"

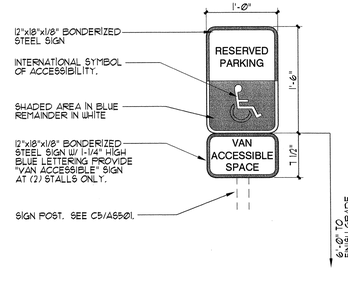


PLAN AT DUMP./ GEN. ENCLOSURE B3 / AS501
SCALE: 1/4" = 1'-0"

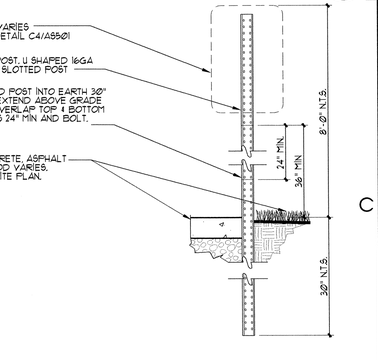
GALV. STL. GATE DETAIL D4 / AS501
SCALE: 3" = 1'-0"



EXPANSION AND CONTROL JOINTS D5 / AS501
SCALE: 1 1/2" = 1'-0"



ACCESSIBLE PARKING DETAIL C4 / AS501
SCALE: 1" = 1'-0"



ACCESSIBLE PARKING DETAIL C5 / AS501
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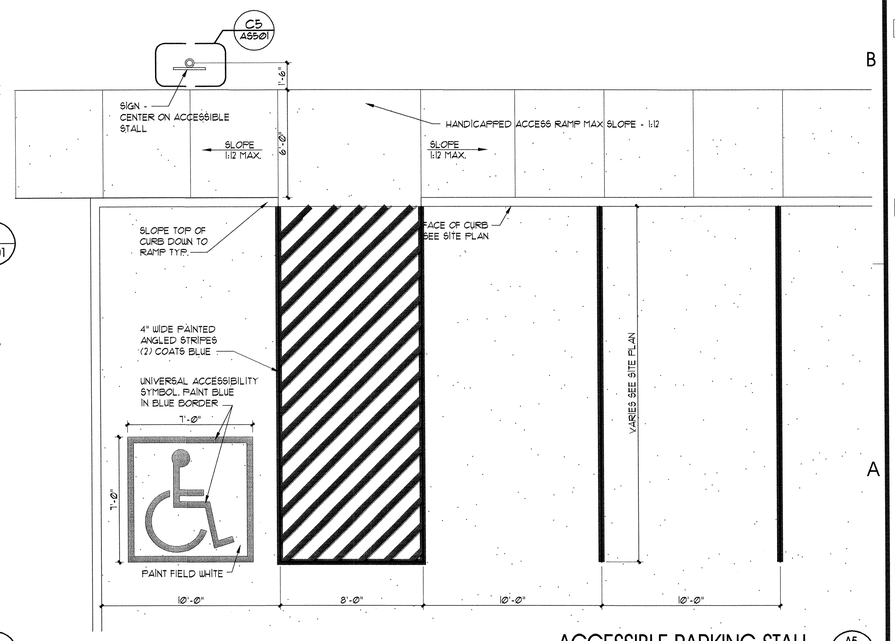
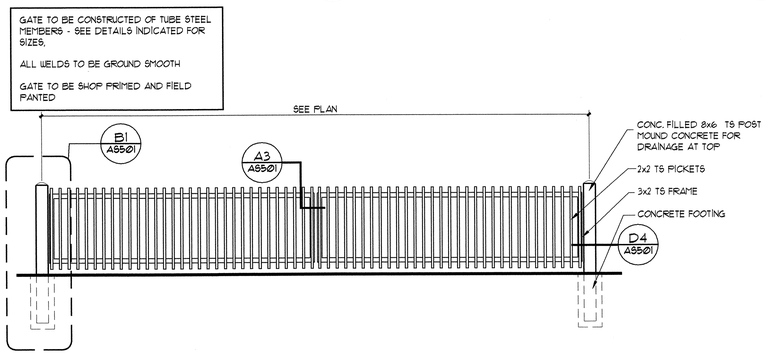
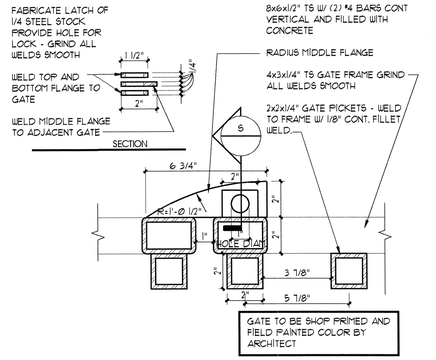


ACCESSIBLE PARKING STALL A5 / AS501
SCALE: 1/4" = 1'-0"



GATE ELEVATION A2 / AS501
SCALE: 1/4" = 1'-0"



GATE LATCH DETAIL A3 / AS501
SCALE: 3" = 1'-0"

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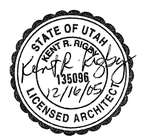

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

SITE DETAILS

REVISIONS

MARK	DATE	DESCRIPTION

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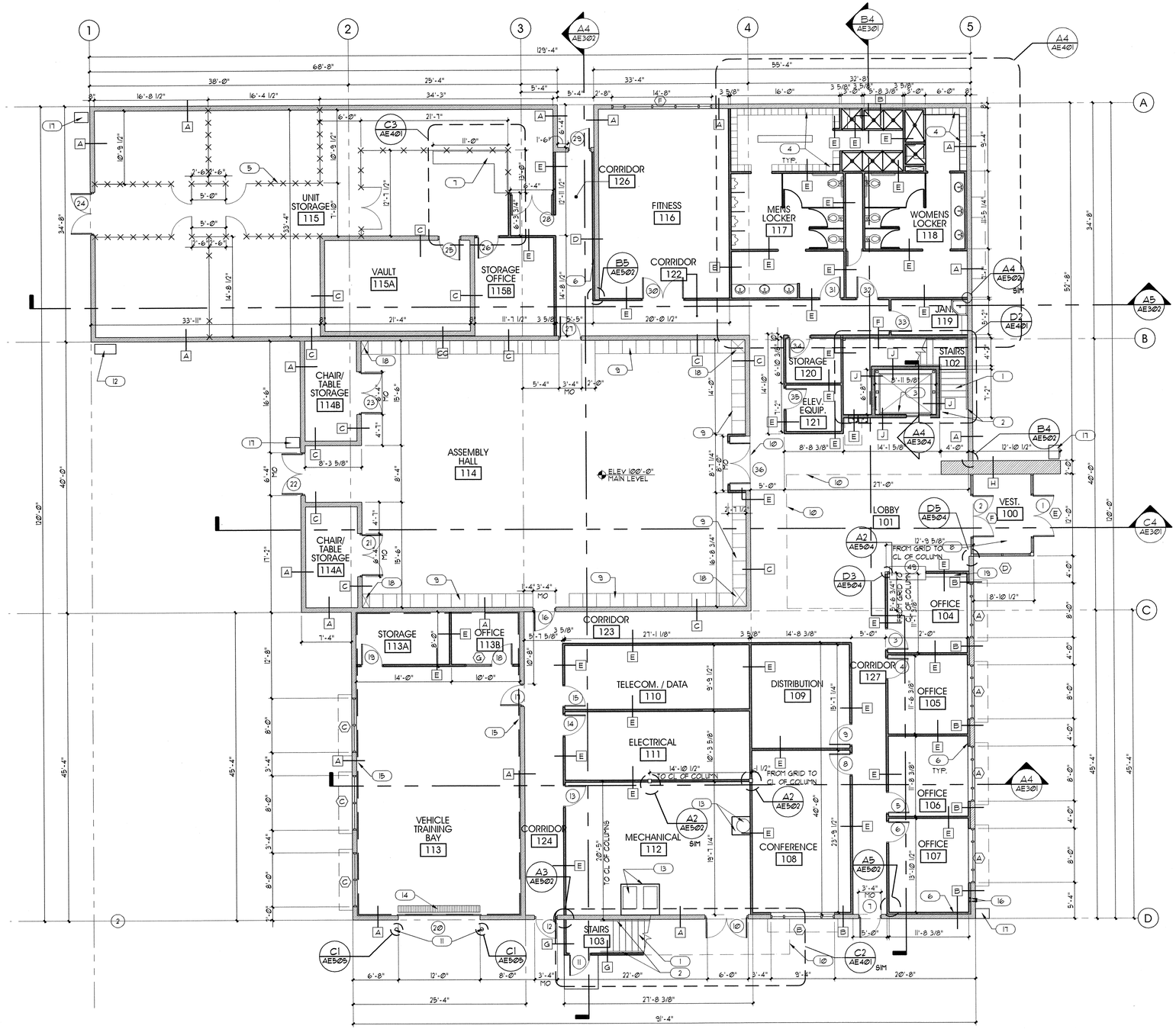

MAIN LEVEL FLOOR PLAN
SCALE: 1/8" = 1'-0"

GENERAL NOTES AND LEGEND:

*FOR SHEET AE101 ONLY.

- SEE SHEET G1002 FOR GENERAL NOTES.
- SEE TITTLE SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.
- MASONRY OPENING DIMENSIONS SHOWN ARE NOMINAL DIMENSIONS - NOT ACTUAL.

WINDOW TYPES - SEE SHEET AE603

WALL TYPES - SEE SHEET AE501

DOOR TYPES - SEE SHEET AE602

KEYED NOTE LEGEND:

*FOR SHEET AE101 ONLY.

1. CONCRETE / METAL PAN STAIRS.
2. STEEL PIPE HANDRAIL.
3. HYDRAULIC ELEVATOR.
4. STEEL LOCKER - 12"x12"x6Ø" WITH DUST SHELF.
5. HEAVY GAUGE WIRE MESH PARTITION AND GATES, SEE SHEET AE506 FOR DETAILS AND SPECIFICATION DIVISION 10.
6. STEEL STUD FURRING AT MASONRY WALLS - SEE WALL TYPES.
7. MILLWORK SEE DETAILS AND SECTIONS INDICATED.
8. WALK OFF MAT - SEE SPECIFICATIONS AND FINISH SCHEDULE SHEET AE601.
9. 24"x24"x48" DOUBLE STACK LOCKERS - COLOR BY ARCHITECT.
10. LINE OF SOFFIT ABOVE - SEE SHEET AE104.
11. 6" DIAM. CONCRETE FILLED PIPE BOLLARDS -PAINT COLOR SAFETY YELLOW.
12. GAS METER - SEE MECHANICAL DRAWINGS.
13. MECHANICAL EQUIPMENT AND HOUSEKEEPING PADS - SEE MECHANICAL DRAWINGS.
14. TRENCH DRAIN - SEE PLUMBING DRAWINGS AND SPECIFICATIONS.
15. DASHED LINE INDICATES 1 HOUR SEPARATION FOR INCIDENTAL USE AREAS.
16. ROOF LINE DRAIN DROPS, SEE MECHANICAL DRAWINGS.
17. PRECAST CONCRETE SPLASH BLOCK - 18x24.
18. METAL LOCKER INFILL PANEL.
19. PROVIDE 10" THICK WALL BELOW COUNTER HEIGHT FOR MECHANICAL UNIT - COORDINATE WITH MECHANICAL DRAWINGS.

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

MAIN LEVEL
FLOOR PLAN

REVISIONS

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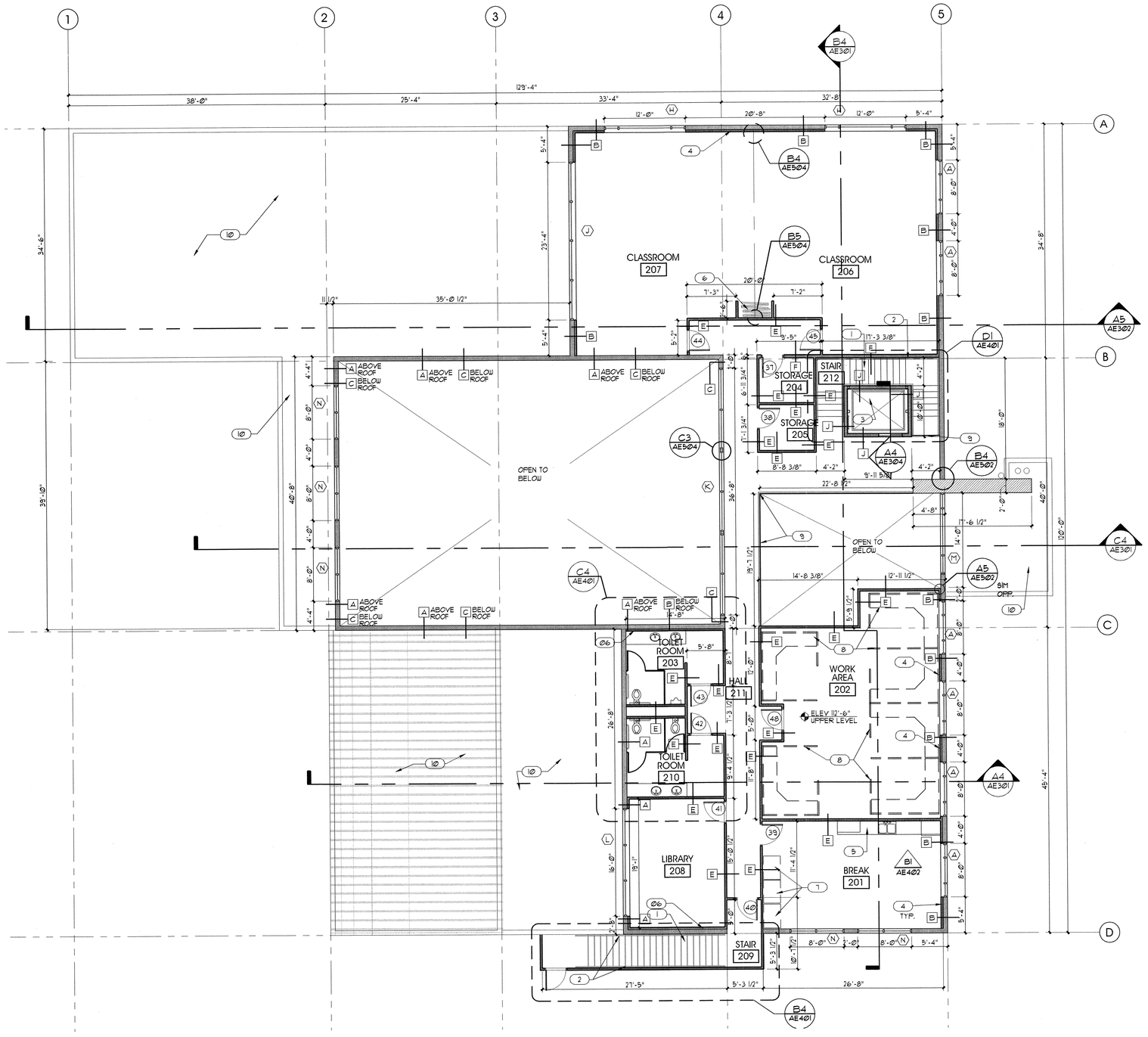

UPPER LEVEL FLOOR PLAN
SCALE: 1/8" = 1'-0"

GENERAL NOTES AND LEGEND:

*FOR SHEET AE1Ø1 ONLY.

- SEE SHEET G1Ø02 FOR GENERAL NOTES.
- SEE TITTLE SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.
- SEE SHEET AE5Ø1 FOR WALL TYPE DESIGNATIONS.
- MASONRY OPENING DIMENSIONS SHOWN ARE NOMINAL DIMENSIONS, NOT ACTUAL.

(*) WINDOW TYPES - SEE SHEET AE6Ø3

(*) WALL TYPES - SEE SHEET AE5Ø1

(*) DOOR TYPES - SEE SHEET AE6Ø2

KEYED NOTE LEGEND:

*FOR SHEET AE1Ø2 ONLY.

1. CONCRETE / METAL PAN STAIRS.
2. STEEL PIPE HANDRAIL - PAINT COLOR AS SELECTED BY ARCHITECT.
3. HYDRAULIC ELEVATOR.
4. STEEL STUD FURRING AT MASONRY WALLS - SEE WALL TYPES.
5. MILLWORK SEE DETAILS AND SECTIONS INDICATED.
6. MOVEABLE PARTITION ROOM DIVIDER WITH SOUND INSULATION.
7. VENDING MACHINES BY OWNER.
8. SYSTEMS FURNITURE BY OWNER.
9. GS AND WOOD GUARDRAIL.
10. ROOF BELOW - SEE ROOF PLAN SHEET AE1Ø3 FOR SLOPE AND DRAIN LOCATIONS.

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

UPPER LEVEL
FLOOR PLAN

REVISIONS

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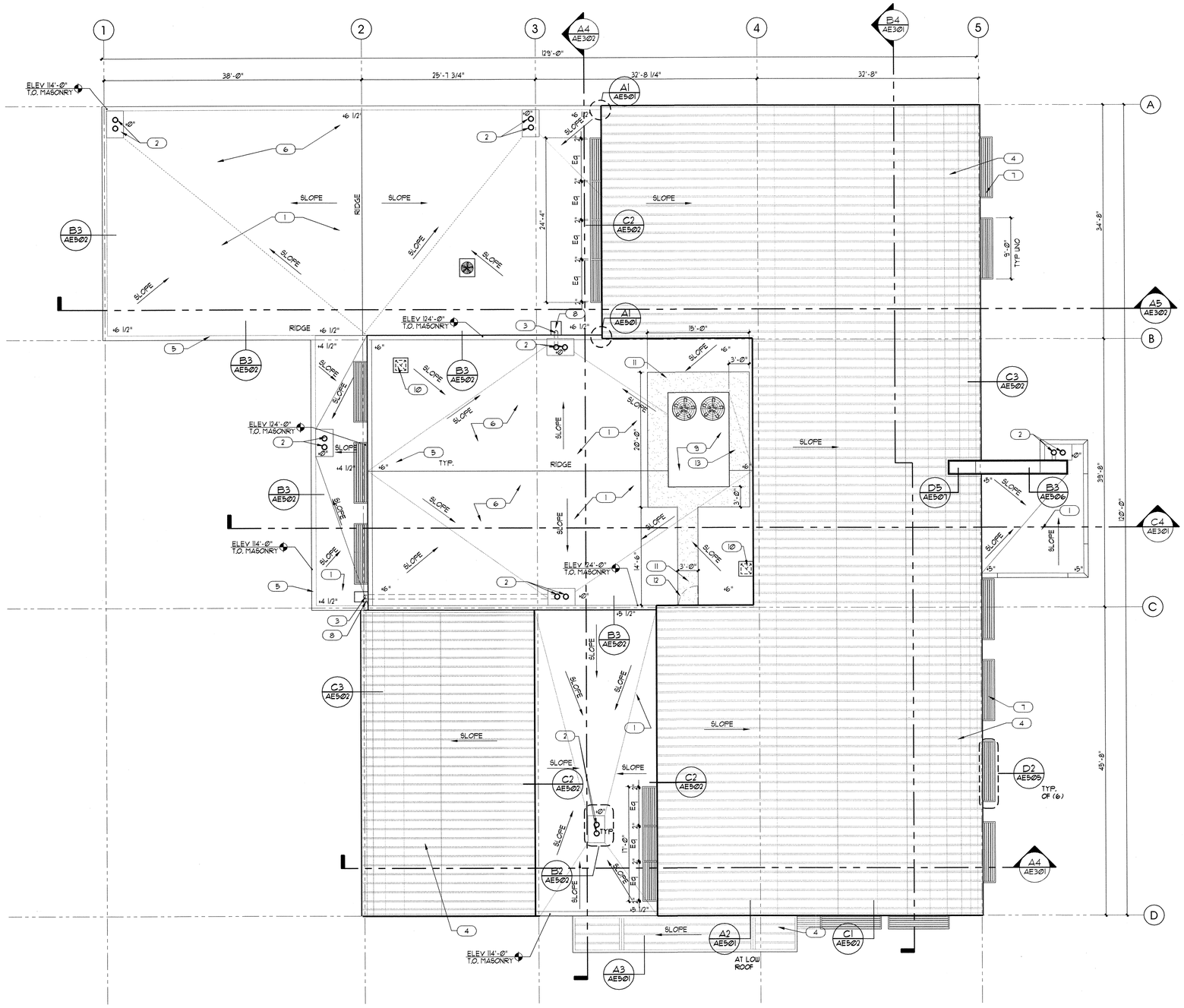
AE102



ROOF PLAN
SCALE: 1/8" = 1'-0"

A4 / AE103

GENERAL NOTES AND LEGEND:

*FOR SHEET AE103 ONLY.

- SEE SHEET G102 FOR GENERAL NOTES.
- SEE TITLE SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.

KEYED NOTE LEGEND:

*FOR SHEET AE103 ONLY.

1. FULLY ADHERED SINGLE PLY TPO ENERGY STAR ROOFING MEMBRANE
2. PRIMARY AND SECONDARY ROOF DRAINS. SEE MECHANICAL DRAWINGS.
3. DRAIN TO LOWER ROOF
4. STANDING SEAM ENERGY STAR ROOF
5. NUMBERS INDICATE THICKNESS OF RIGID INSULATION (TYP.)
6. STRUCTURE TO BE SLOPED THIS AREA
7. LOUVERED SHADING DEVICE
8. PRECAST CONCRETE SPLASH BLOCK
9. ROOF MOUNTED MECHANICAL UNIT - SEE MECHANICAL DRAWINGS.
10. ROOF MOUNTED MECHANICAL VENT / FAN - SEE MECHANICAL DRAWINGS.
11. PROVIDE ROOF MANUFACTURERS WALK PAD CONTINUOUS FROM ACCESS DOOR AND AROUND MECHANICAL UNIT.
12. LINE OF ACCESS DOOR - SEE BUILDING SECTIONS.
13. CRICKET CURB AS REQUIRED.

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

ROOF
PLAN

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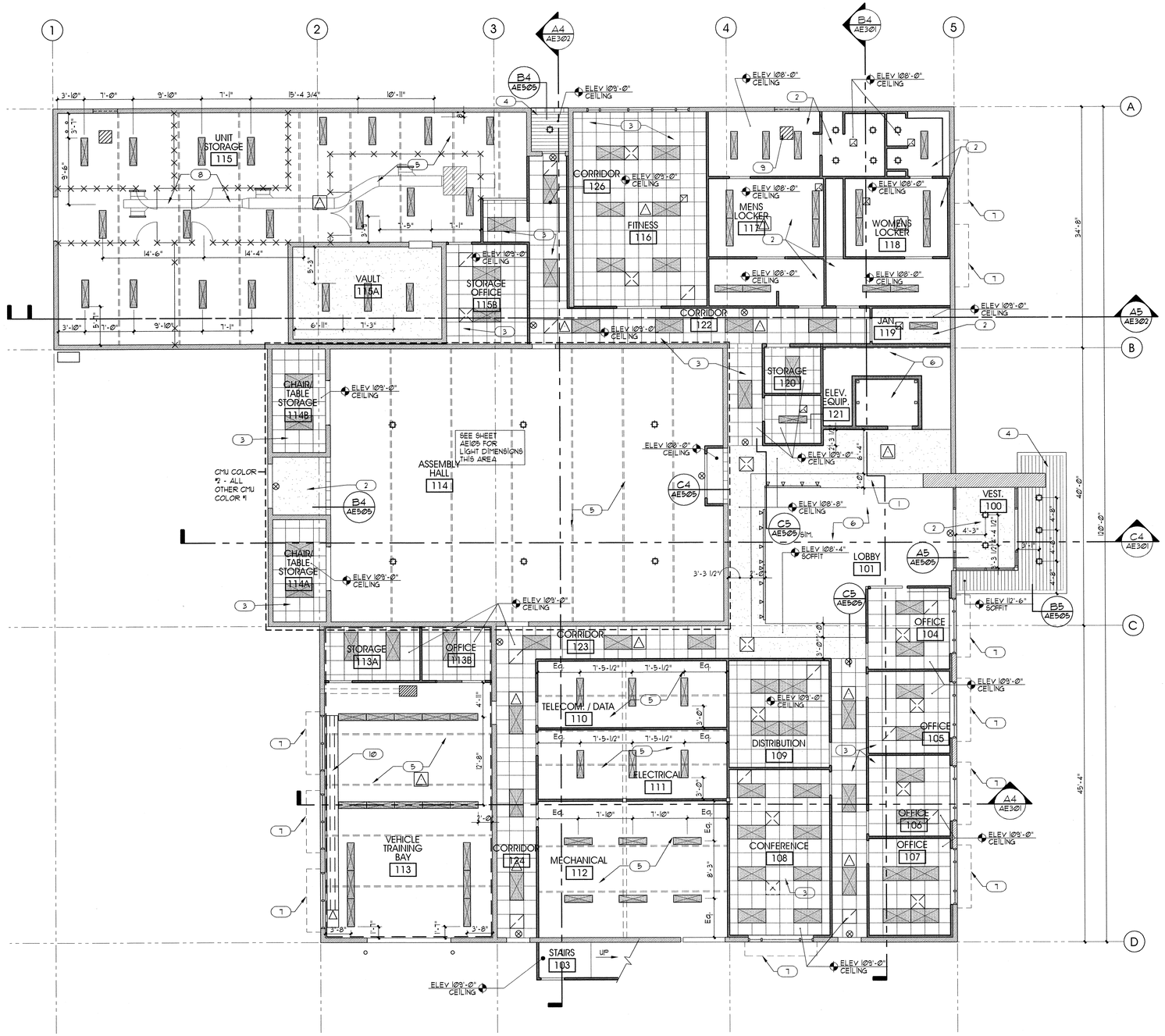

MAIN LEVEL REFLECTED CEILING PLAN

SCALE: 1/8" = 1'-0"

A4 AE104

GENERAL NOTES AND LEGEND:

*FOR SHEET AE104 ONLY.

- SEE SHEET G1002 FOR GENERAL NOTES.
- SEE TITTLE SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.
- DO NOT SUSPEND MECHANICAL EQUIPMENT, DUCTING PIPING OR ELECTRICAL EQUIPMENT FROM METAL DECKING.

2x4 FLUORESCENT FIXTURE

1x4 FLUORESCENT FIXTURE

EXIT SIGN

RECESSED CAN LIGHT

DIRECTIONAL TRACK LIGHTING

2x2 ACOUSTIC PANEL LAY IN CEILING

5/8" SUSPENDED GYP. BD. CEILING.

FIRE-FINISHED METAL SOFFIT

SPEAKER

RETURN AIR GRILLE

SUPPLY AIR DIFFUSER

KEYED NOTE LEGEND:

*FOR SHEET AE104 ONLY.

1. GYP. BD. SOFFIT - SEE SECTIONS AND DETAILS.
2. 5/8" TYP. "X" GYP. BD. CEILING - INSTALL WR GYP. IN ALL WET AREAS
3. 2X2 ACOUSTICAL PANEL CEILING IN PREFINISHED METAL T-GRID - SEE FINISH SCHEDULE SHEET AE601 - DO NOT SUSPEND FROM METAL DECKING.
4. FIRE-FINISHED METAL PAN SOFFIT - SEE SECTIONS AND DETAILS.
5. CEILING OPEN TO STRUCTURE ABOVE THIS AREA. ALL EXPOSED STRUCTURE TO BE PAINTED
6. SEE SHEET AE105 FOR CEILING HEIGHTS THIS AREA.
7. LINE OF LOUVERED SHADING DEVICE - SEE A4/AE103
8. EXPOSED MECHANICAL DUCTING - SEE MECHANICAL DRAWINGS. PAINT COLOR SELECTED BY ARCHITECT.
9. CEILING ACCESS DOOR - SEE MECHANICAL PLANS.
10. RADIENT HEATER SEE MECHANICAL PLANS.

NOTE: SEE DETAILS D1 THROUGH D4 ON SHEET AE502 FOR CEILING SUSPENSION AND BRACING.

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

MAIN LEVEL REFLECTED CEILING PLAN

REVISIONS

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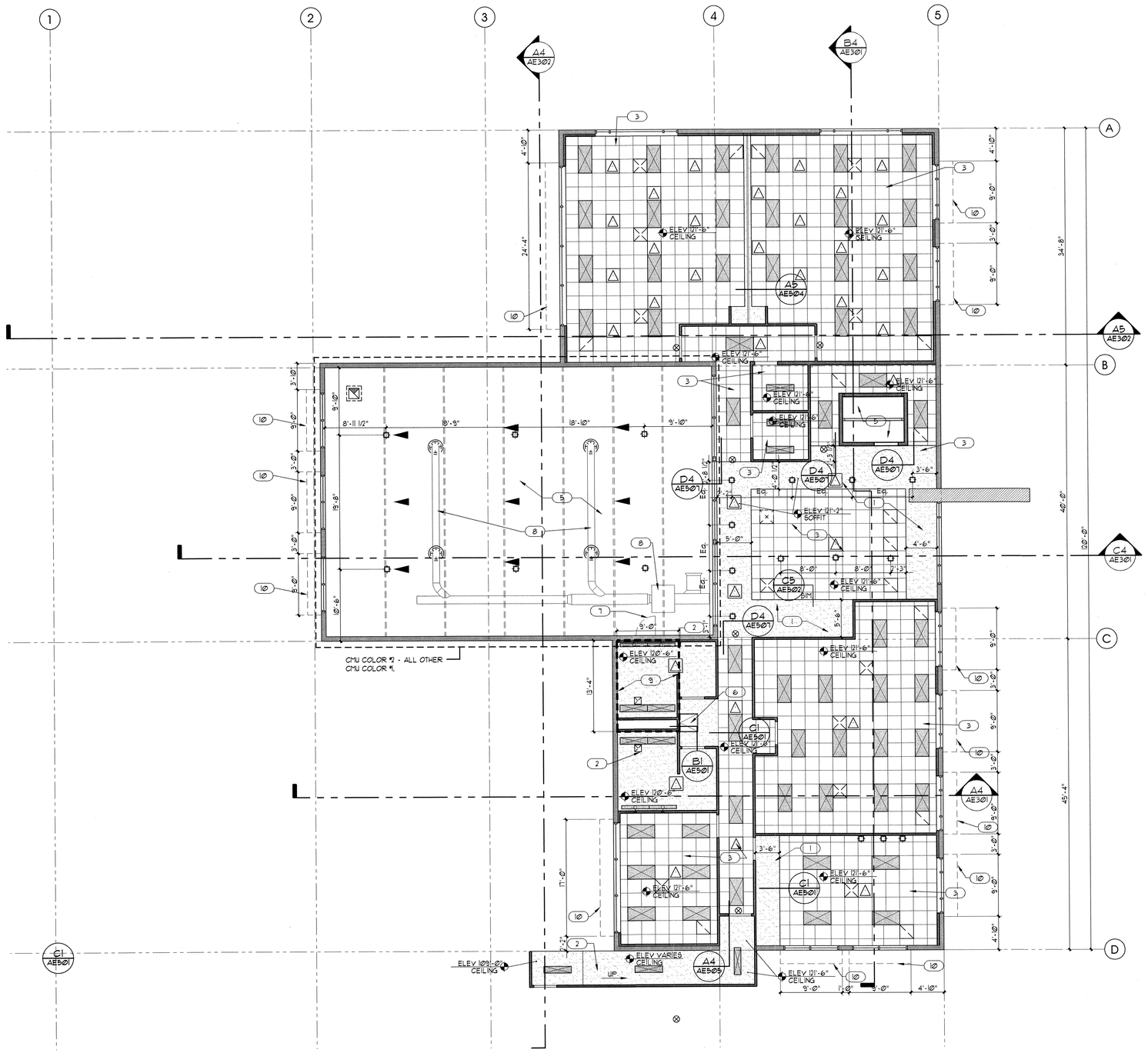

UPPER LEVEL REFLECTED CEILING PLAN

SCALE: 1/8" = 1'-0"

GENERAL NOTES AND LEGEND:

*FOR SHEET AE105 ONLY.

- SEE SHEET G1002 FOR GENERAL NOTES.
- SEE COVER SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.
- DO NOT SUSPEND MECHANICAL EQUIPMENT, DUCTING PIPING OR ELECTRICAL EQUIPMENT FROM METAL DECKING.

2x4 FLUORESCENT FIXTURE

1x4 FLUORESCENT FIXTURE

WALL MOUNTED FLUORESCENT FIXTURE

RECESSED CAN LIGHT

2x2 ACOUSTIC PANEL LAY IN CEILING

5/8" SUSPENDED GYP. BD. CEILING.

RETURN AIR GRILLE

SUPPLY AIR DIFFUSER

SPEAKERS

EXIT SIGN

KEYED NOTE LEGEND:

*FOR SHEET AE105 ONLY.

1. GYP. BD. SOFFIT - SEE SECTIONS AND DETAILS.
2. 5/8" TYP. "X" GYP. BD. CEILING - INSTALL EXTERIOR GYP. SOFFIT BD. IN ALL WET AREAS.
3. 2X2 ACOUSTICAL PANEL CEILING IN PREFINISHED METAL T-GRID - SEE FINISH SCHEDULE SHEET AE600 - DO NOT SUSPEND FROM METAL DECKING.
4. EIFS SOFFIT - SEE SECTIONS AND DETAILS.
5. CEILING OPEN TO STRUCTURE ABOVE THIS AREA. PAINT ALL EXPOSED STRUCTURE
6. 2'-8" x 2'-8" CEILING ACCESS DOOR AND FRAME TO PLATFORM ABOVE.
7. LINE OF ACCESS HATCH ABOVE (ON ROOF) - SEE DETAIL B3/AE505.
8. EXPOSED MECHANICAL DUCTING - SEE MECHANICAL DRAWINGS. PAINT COLOR SELECTED BY ARCHITECT.
9. DASHED LINE INDICATES EXTENT OF PLATFORM ABOVE.
10. LINE OF LOUVERED SHADING DEVICE ABOVE - SEE A4/AE103.

NOTE: SEE DETAILS D1 THROUGH D4 ON SHEET AE502 FOR CEILING SUSPENSION AND BRACING.

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

UPPER LEVEL REFLECTED CEILING PLAN

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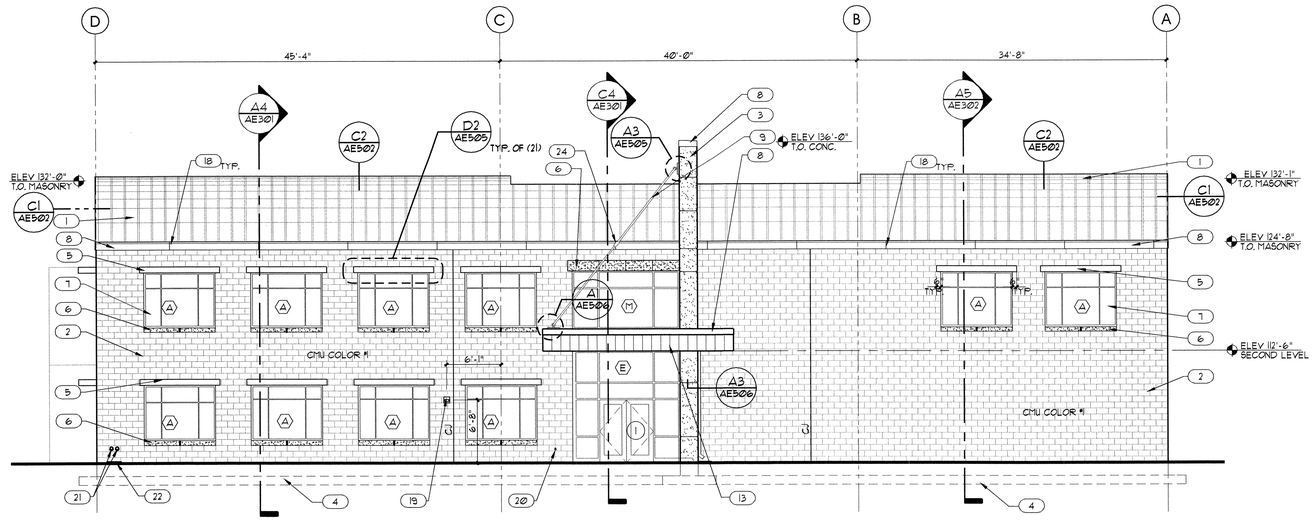
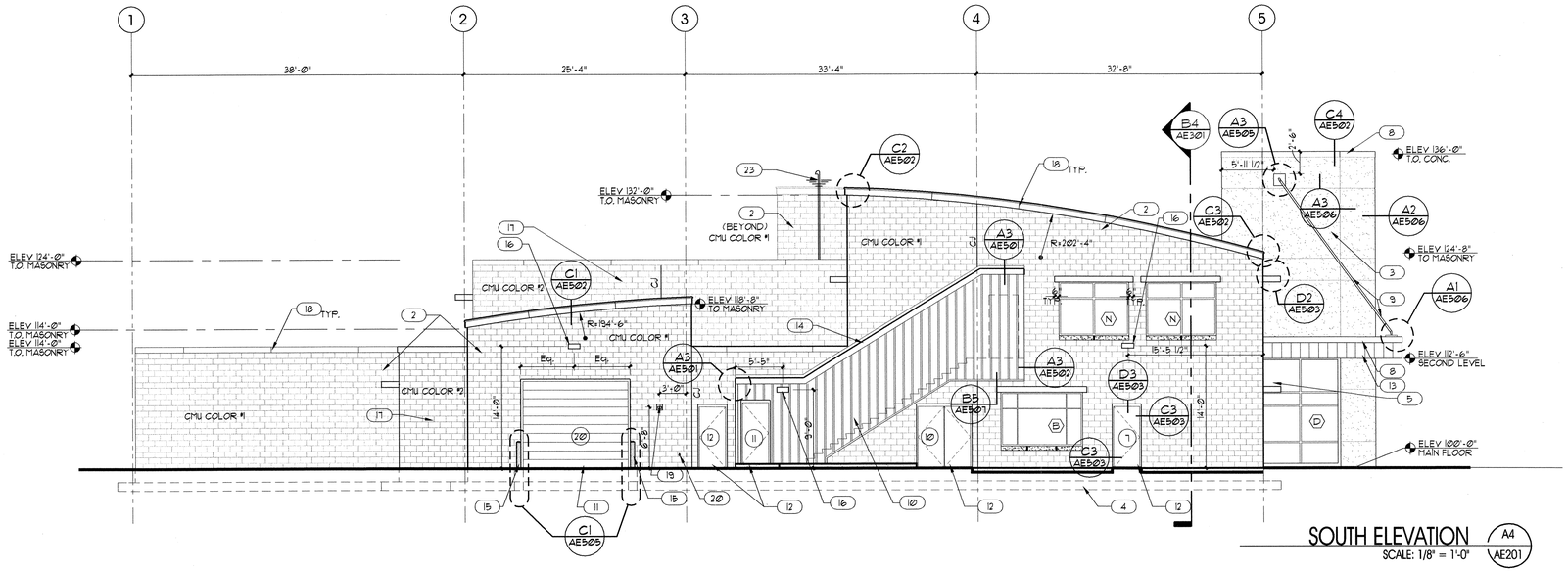


EAST ELEVATION (C4 / AE201)
SCALE: 1/8" = 1'-0"



SOUTH ELEVATION (A4 / AE201)
SCALE: 1/8" = 1'-0"

GENERAL NOTES AND LEGEND:

*FOR SHEET AE201 ONLY.

- SEE SHEET G1002 FOR GENERAL NOTES.
- SEE TITLE SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.

*FOR SHEET AE201 ONLY.

1. PRE-FINISHED STANDING SEAM METAL ROOFING.
2. SMOOTH FACE INTEGRALLY COLORED CMU WITH WATER REPELLENT - COLOR #1 SELECTED BY ARCHITECT.
3. ARCHITECTURAL GRADE CAST IN PLACE CONCRETE W/ WATER REPELLENT. SEE ELEVATION A2/AE505 FOR REVEALS.
4. LINE OF FOOTING AND FOUNDATION. SEE STRUCTURAL DRAWINGS.
5. LOUVERED SHADE DEVICE. PAINTED.
6. PRECAST COLORED CONCRETE SILL / LINTEL - COLOR SELECTED BY ARCHITECT.
7. ANODIZED ALUMINUM STOREFRONT SYSTEM WITH INSULATED GLAZING - COLOR SELECTED BY ARCHITECT. SEE SHEET AE602.
8. PRE-FINISHED METAL CAP FLASHING - COLOR SELECTED BY ARCHITECT.
9. STEEL TIE ROD & CLEVIS, PAINTED
10. PRE-FINISHED METAL STANDING SEAM WALL PANELS - COLOR AS SELECTED BY ARCHITECT.
11. OVERHEAD COILING DOOR. EE SHEET AE602
12. HOLLOW METAL DOOR & FRAME, PAINTED. SEE SHEET AE602.
13. PRE-FINISHED METAL FASCIA PANELS - COLOR AS SELECTED BY ARCHITECT.
14. CONTINUOUS REGLET AND COUNTER FLASHING.
15. 6" DIAM. CONCRETE FILLED STEEL BOLLARDS. PAINT SAFETY YELLOW.
16. EXTERIOR LIGHTING. SEE ELECTRICAL DRAWINGS AND SPECS.
17. SMOOTH FACE INTEGRALLY COLORED CMU WITH WATER REPELLENT. COLOR #2 SELECTED BY ARCHITECT.
18. LAP SEAM JOINTS AT 10'-0" OC MAX. SPACING. SPACE SYMMETRICALLY ABOUT ROOF EDGE.
19. HORN STROBE - SEE ELECTRICAL DRAWINGS.
20. HOSE BIB - SEE PLUMBING DRAWINGS.
21. ROOF DRAIN LINES - SEE PLUMBING DRAWINGS.
22. PRECAST CONCRETE SPLASH BLOCK - 18x24.
23. ANTENNA - FOR BIDDING PURPOSES ASSUME AN ANTENNA MOUNTING HEIGHT OF 8 FEET ABOVE THE ROOFLINE. REFER TO SPECIFICATION 16850 FOR ADDITIONAL INFORMATION AND REQUIREMENTS.
24. THREADED ROD COUPLER @ MIDSPAN

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

EXTERIOR ELEVATIONS

REVISIONS

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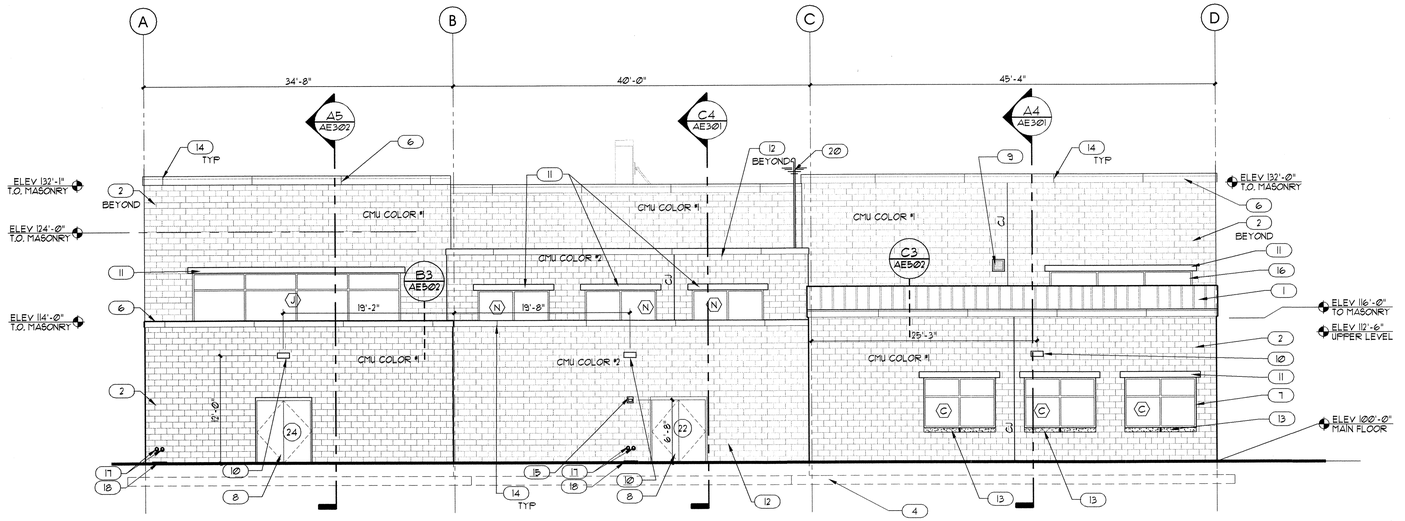
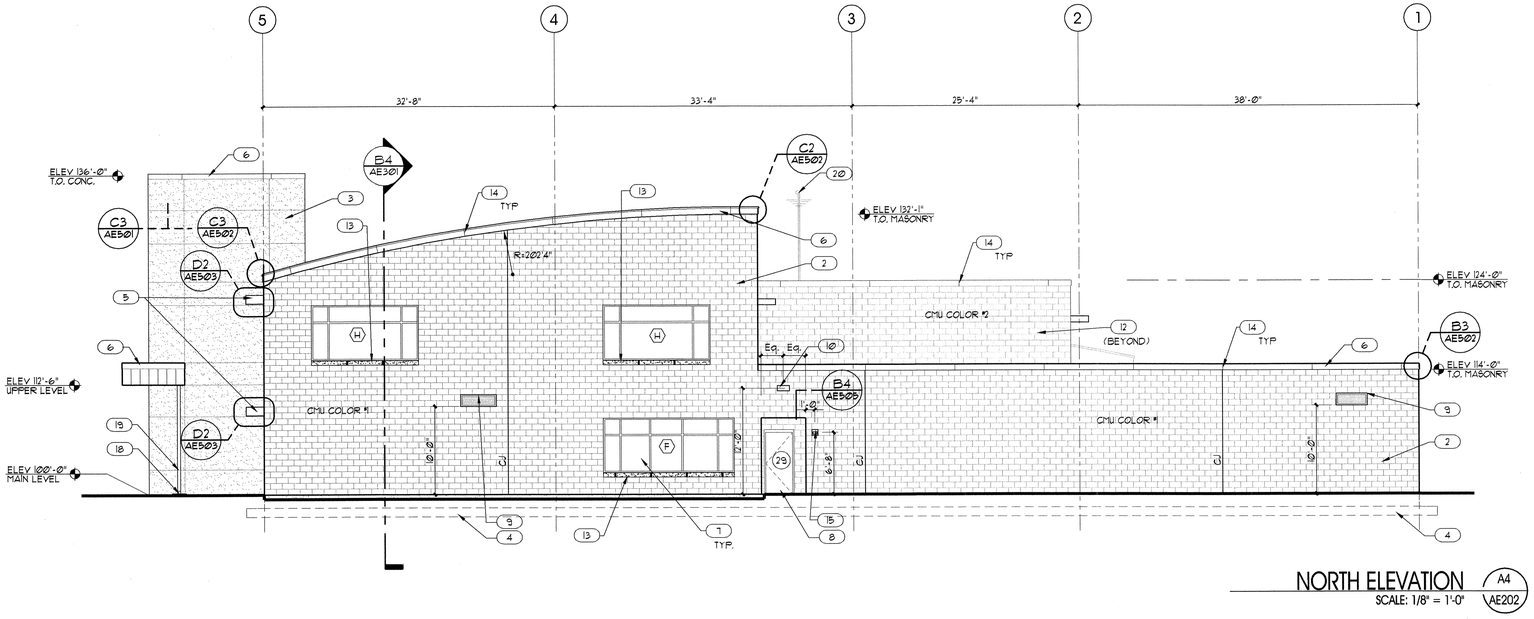


WEST ELEVATION (C4 / AE202)
SCALE: 1/8" = 1'-0"



NORTH ELEVATION (A4 / AE202)
SCALE: 1/8" = 1'-0"

GENERAL NOTES AND LEGEND:

*FOR SHEET AE202 ONLY.

- SEE SHEET G1002 FOR GENERAL NOTES.
- SEE TITLE SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.

KEYED NOTE LEGEND:

*FOR SHEET AE202 ONLY.

1. PRE-FINISHED STANDING SEAM METAL ROOFING.
2. SMOOTH FACE INTEGRALLY COLORED CMU WITH WATER REPELLENT - COLOR #1 SELECTED BY ARCHITECT.
3. ARCHITECTURAL GRADE CAST IN PLACE CONCRETE W/ WATER REPELLENT - SEE ELEVATION A2/AE505 FOR REVEAL.
4. LINE OF FOOTING AND FOUNDATION
5. LOUVERED SHADE DEVICE
6. PRE-FINISHED METAL CAP FLASHING - COLOR SELECTED BY ARCHITECT.
7. ANODIZED ALUMINUM STOREFRONT SYSTEM WITH INSULATED GLAZING.
8. HOLLOW METAL DOOR & FRAME, PAINTED. SEE AE602
9. MECHANICAL LOUVER/VENT - SEE MECHANICAL DRAWINGS AND SPECIFICATIONS.
10. EXTERIOR LIGHT FIXTURE - SEE ELECTRICAL DRAWINGS AND SPECIFICATIONS.
11. LOUVERED SHADE DEVICE - PAINTED.
12. SMOOTH FACE INTEGRALLY COLORED CMU WITH WATER REPELLENT - COLOR #2 TO BE SELECTED BY ARCHITECT.
13. PRECAST COLORED CONCRETE SILL W/ WATER REPELLENT - COLOR SELECTED BY ARCHITECT.
14. LAP SEAM JOINTS AT 10'-0" OC MAX. SPACING. SPACE SYMMETRICALLY ABOUT ROOF EDGE.
15. HORN STROBE - SEE ELECTRICAL DRAWINGS.
16. THIS WINDOW HAS A PRECAST CONCRETE SILL.
17. PRIMARY AND SECONDARY ROOF DRAIN LINES - SEE PLUMBING DRAWINGS.
18. PRECAST CONCRETE SPLASH BLOCK - 18x24.
19. PROVIDE PRE-FINISHED METAL DOWNSPOUT AT PRIMARY ROOF DRAIN ONLY.
20. ANTENNA - FOR BIDDING PURPOSES ASSUME AN ANTENNA MOUNTING HEIGHT OF 8 FEET ABOVE THE ROOFLINE. REFER TO SPECIFICATION 16850 FOR ADDITIONAL INFORMATION AND REQUIREMENTS.

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STATE OF UTAH · KENT R. ROGERS · 135096 · 12/10/05 · LICENSED ARCHITECT

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

EXTERIOR ELEVATIONS

REVISIONS

MARK	DATE	DESCRIPTION

ISSUE DATA

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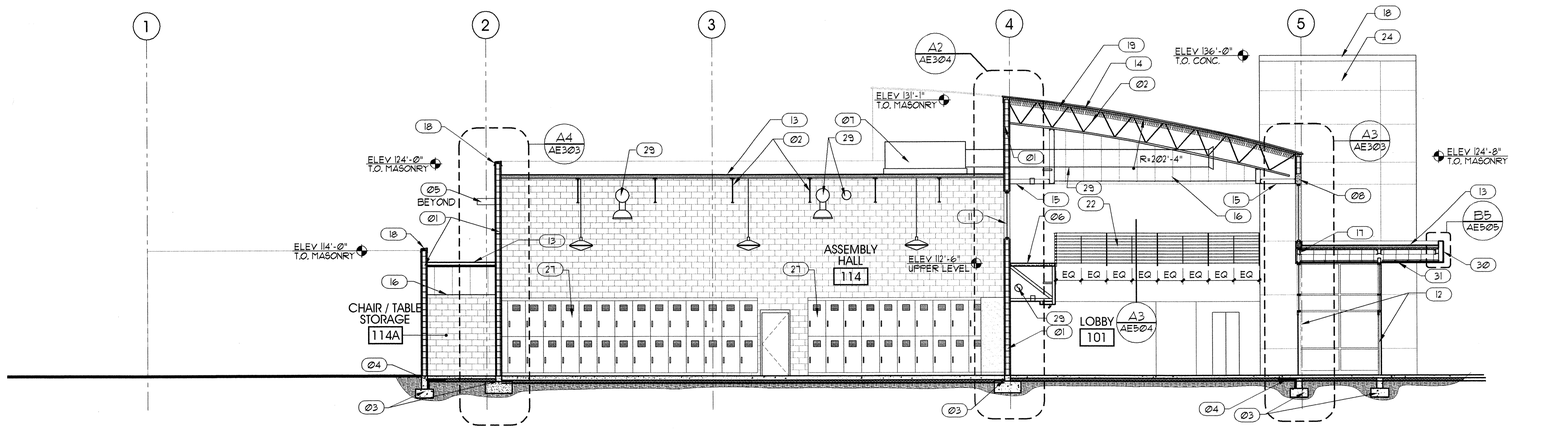
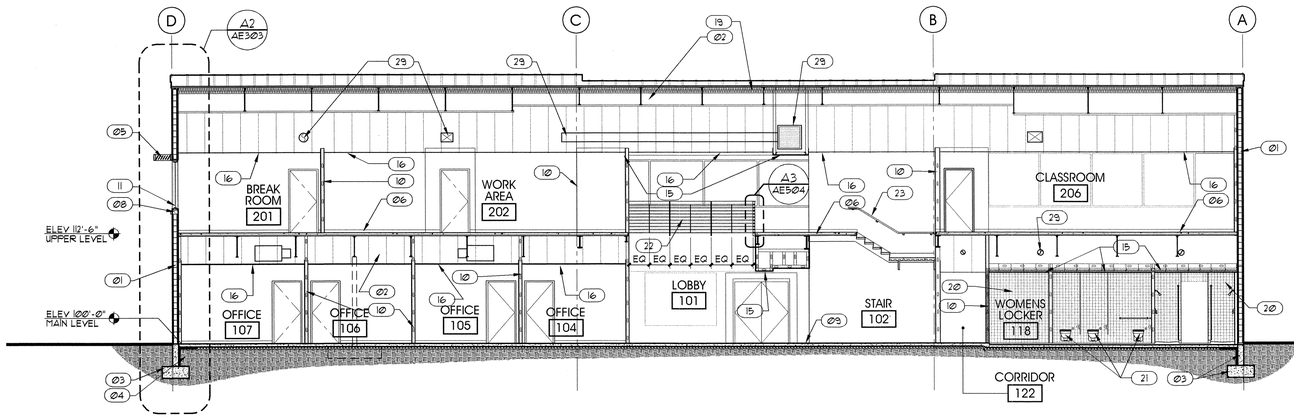
DFCM PROJECT #	04042480

SHEET NUMBER:

AE202



BUILDING SECTION (C4 / AE301)
SCALE: 1/8" = 1'-0"



BUILDING SECTION (B4 / AE301)
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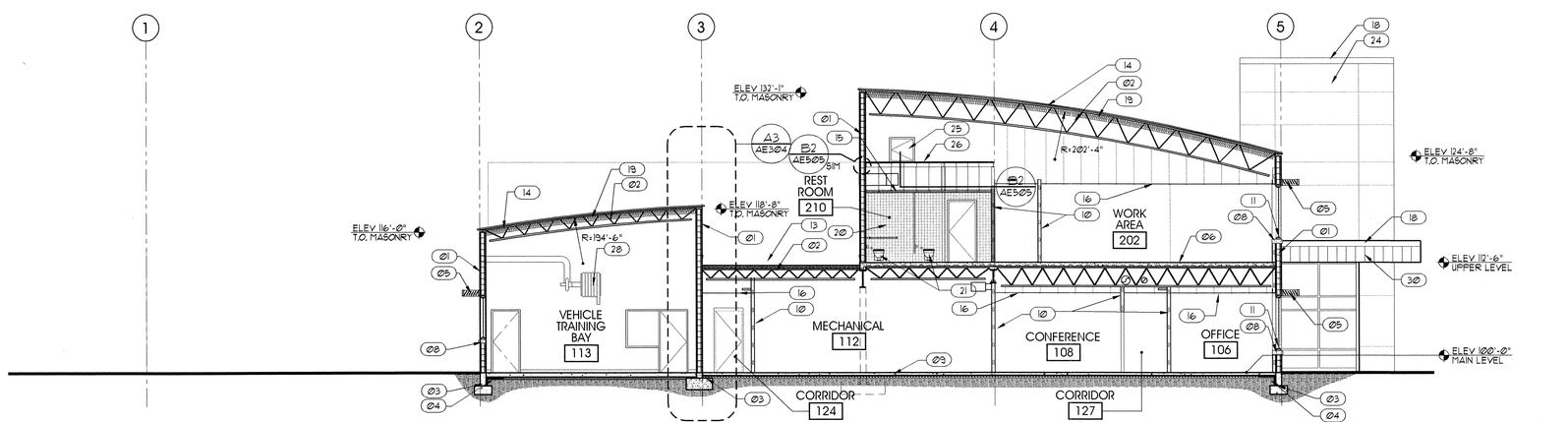

BUILDING SECTION (A4 / AE301)
SCALE: 1/8" = 1'-0"

GENERAL NOTES AND LEGEND:

*FOR SHEET AE301 ONLY.

- SEE SHEET G1002 FOR GENERAL NOTES.
- SEE TITLE SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.
- ROOF RADI MEASURED TO TOP CORD OF TRUSS.

KEYED NOTE LEGEND:

*FOR SHEET AE301 ONLY.

01 8" CMU WALL. SEE STRUCTURAL DRAWINGS, SEE STRUCTURAL DRAWINGS, EXTERIOR ELEVATIONS FOR COLOR DESIGNATIONS, SEE WALL TYPES SHEET AE501.
02 STRUCTURAL STEEL FRAMING, PAINT EXPOSED STRUCTURAL MEMBERS. SEE STRUCTURAL DRAWINGS.
03 CONCRETE FOOTING AND FOUNDATION. SEE STRUCTURAL DRAWINGS.
04 1-1/2" RIGID PERIMETER INSULATION CONTINUOUS. SEE A3/AE303.
05 LOUVERED SHADE DEVICE PAINTED COLOR AS SELECTED BY ARCHITECT.
06 CONCRETE SLAB OVER METAL DECKING. SEE STRUCTURAL DRAWINGS. SEE PLANS FOR FLOOR FINISH.
07 ROOF MOUNTED MECHANICAL UNIT - SEE MECHANICAL DRAWINGS.
08 PRECAST CONCRETE SILL / LINTEL W/ WATER REPELLENT- COLOR SELECTED BY ARCHITECT.
09 CONCRETE FLOOR SLAB. SEE STRUCTURAL DRAWINGS.
10 METAL STUD FRAMING W/ 5/8" TYPE "X" GYP. BD. EACH SIDE.
11 ANODIZED ALUMINUM WINDOW SYSTEM WITH 1" INSULATED GLAZING.
12 ANODIZED ALUMINUM ENTRANCE DOOR.
13 FULLY ADHERED ENERGY STAR TPO ROOF MEMBRANE OVER MIN. R-30 RIGID INSULATION AND METAL ROOF DECK.
14 PRE-FINISHED STANDING SEAM METAL ENERGY STAR ROOFING - COLOR SELECTED BY ARCHITECT
15 5/8" TYP. "X" GYP. BD CEILING / SOFFIT ASSEMBLY.
16 SUSPENDED ACOUSTICAL PANEL CEILING IN PRE-FINISHED METAL GRID SYSTEM.
17 T.S. FRAMING AT ENTRANCE CANOPY - SEE STRUCTURAL DRAWINGS.
18 PRE-FINISHED METAL CAP FLASHING WITH CONTINUOUS HOLD DOWN CLIPS - COLOR SELECTED BY ARCHITECT
19 MIN. R-38 BATT INSULATION W/ VAPOR BARRIER.
20 CERAMIC WALL TILE OVER WR GYP. BD. USE CEMENT BD. AT SHOWERS.
21 PLUMBING FIXTURE - SEE PLUMBING DRAWINGS.
22 SS AND WOOD GUARDRAIL.
23 STEEL PIPE HANDRAIL - PAINT. COLOR SELECTED BY ARCHITECT.
24 ARCHITECTURAL GRADE CAST IN PLACE CONCRETE W/ WATER REPELLENT.
25 ROOF ACCESS DOOR AND FRAME - PAINT COLOR AS SPECIFIED BY ARCHITECT.
26 PLYWOOD PLATFORM FOR ROOF ACCESS STAGING SEE DETAILS INDICATED. SEE A4/AE506 FOR SIZE.
27 DOUBLE TIER EQUIPMENT LOCKERS W/ METAL BASE AND SLOPING TOPS.
28 VEHICLE EXHAUST SYSTEM SEE MECHANICAL DRAWINGS.
29 HVAC DUCTWORK. PAINT WHERE EXPOSED COLOR AS SELECTED BY ARCHITECT.
30 PRE-FINISHED METAL WALL PANELS COLOR AS SELECTED BY ARCHITECT.
31 PRE-FINISHED METAL PANEL SOFFIT - SEE DETAIL INDICATED.

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OWNER INFORMATION

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

BUILDING SECTIONS

REVISIONS

MARK	DATE	DESCRIPTION

ISSUE DATA

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DRAWN BY:	BJA
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CAD FILE NAME:	0470AE301
DFCM PROJECT #	04042480

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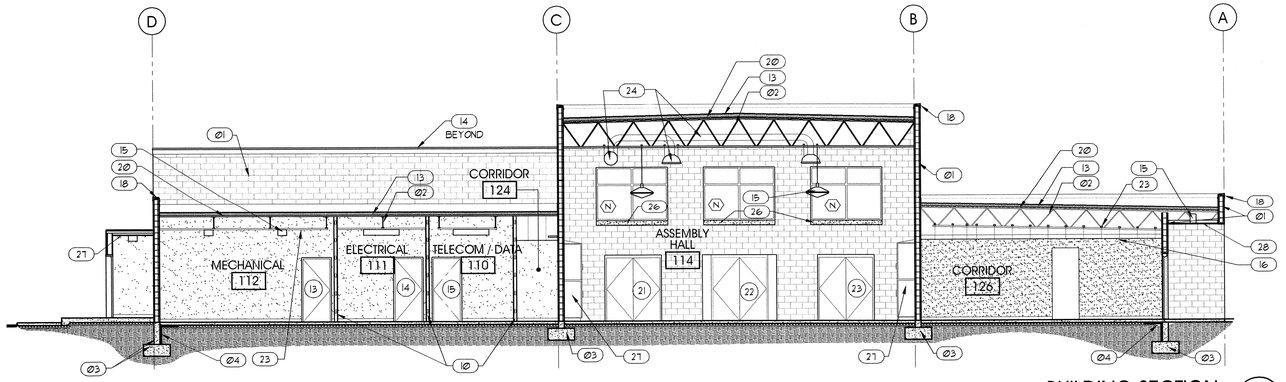
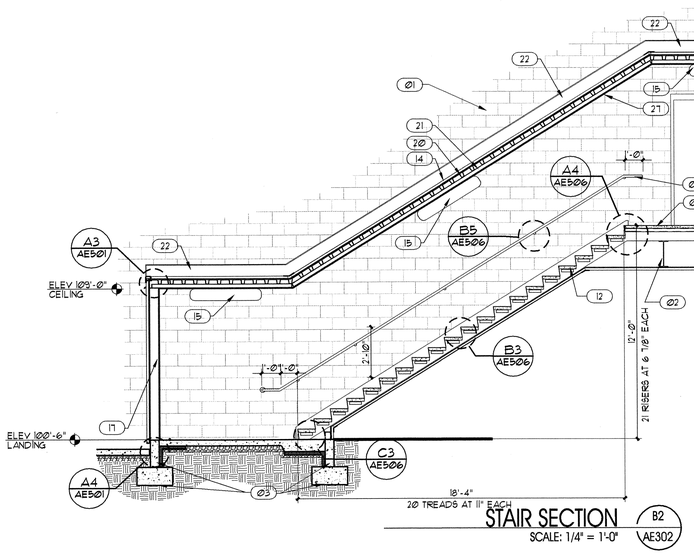
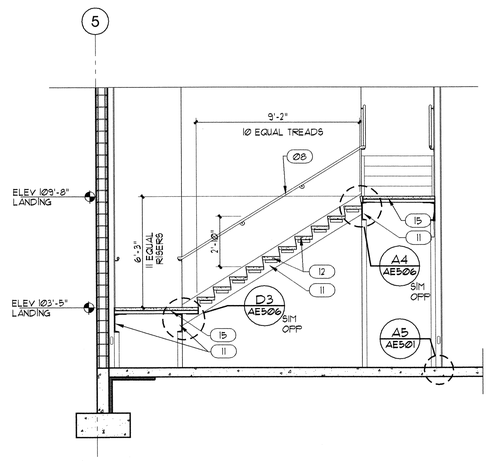
AE301



BUILDING SECTION (A4 / AE302)
SCALE: 1/8" = 1'-0"



STAIR SECTION (B2 / AE302)
SCALE: 1/4" = 1'-0"



STAIR SECTION (B3 / AE302)
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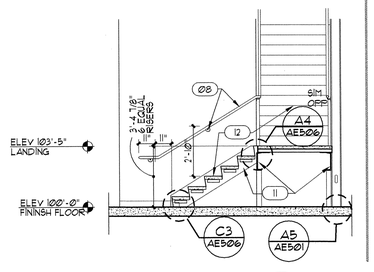

STAIR SECTION (B4 / AE302)
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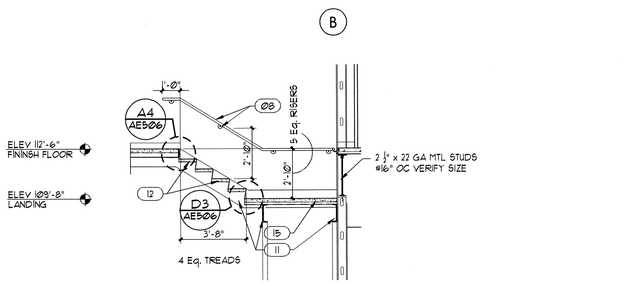
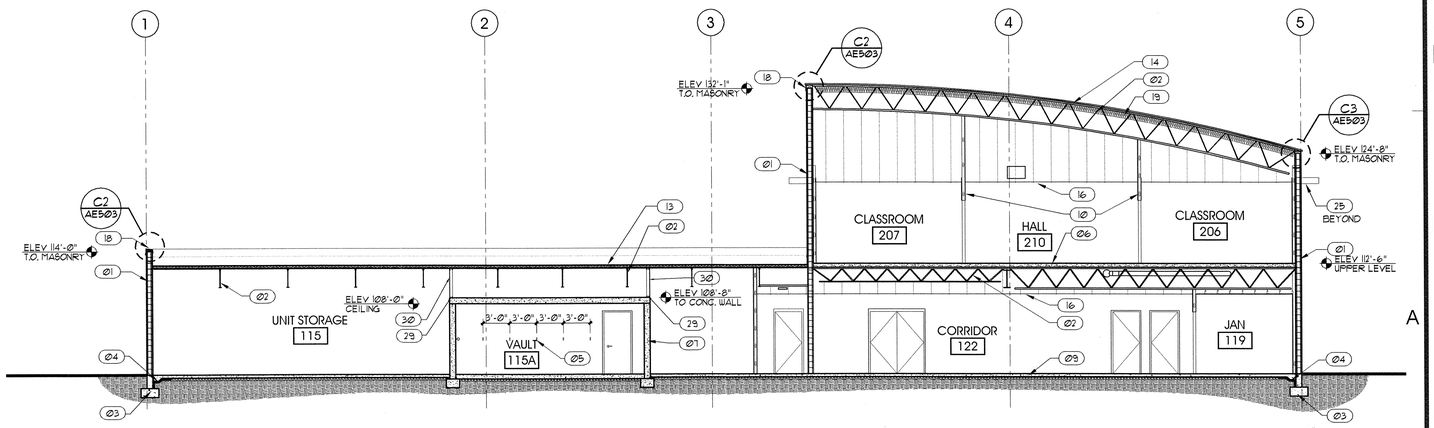


STAIR SECTION (A1 / AE302)
SCALE: 1/4" = 1'-0"



BUILDING SECTION (A5 / AE302)
SCALE: 1/8" = 1'-0"

GENERAL NOTES AND LEGEND:

FOR SHEET AE302 ONLY.

- SEE SHEET G1002 FOR GENERAL NOTES.
- SEE TITLE SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.

KEYED NOTE LEGEND:

FOR SHEET AE302 ONLY.

01 8" CMU WALL. SEE STRUCTURAL DRAWINGS, SEE STRUCTURAL DRAWINGS SEE EXTERIOR ELEVATIONS FOR COLOR DESIGNATION. SEE WALL TYPES SHEET AE501.
02 STRUCTURAL STEEL FRAMING. SEE STRUCTURAL DRAWINGS.
03 CONCRETE FOOTING AND FOUNDATION. SEE STRUCTURAL DRAWINGS.
04 1-1/2" RIGID PERIMETER INSULATION CONTINUOUS, SEE A3/AE303.
05 ANCHOR RINGS - INSTALL AT 48" AFF, AND AT 3'-0" O.C. CONTINUOUS AT VAULT. RINGS TO HAVE 4" OUTSIDE DIAMETER CONSTRUCTED OF 1/2" DIAM. STEEL.
06 CONCRETE SLAB OVER METAL DECKING. SEE STRUCTURAL DRAWINGS. SEE PLANS FOR FLOOR FINISH.
07 ARMORY VAULT - SEE STRUCTURAL DRAWINGS FOR REINFORCEMENT.
08 STEEL PIPE HANDRAILING WITH BRACKETS AT 6'-0" MAX. O.C. PAINT COLOR AS SELECTED BY ARCHITECT.
09 CONCRETE FLOOR SLAB. SEE STRUCTURAL DRAWINGS.
10 METAL STUD FRAMING W/ 5/8" TYPE "X" GYP. BD. EACH SIDE.
11 12 X 30 STEEL CHANNEL STRINGER - SEE STRUCTURAL DRAWINGS, PAINT COLOR AS SELECTED BY ARCHITECT.
12 CONCRETE AND STEEL PAN STAIR WITH CAST IN ALUMINUM ABRASIVE / DETECTABLE WARNING STRIP - SEE SPECIFICATIONS.
13 FULLY ADHERED ENERGY STAR TPO ROOF MEMBRANE OVER MIN. R-30 RIGID INSULATION.
14 PRE-FINISHED STANDING SEAM METAL ENERGY STAR ROOFING - COLOR SELECTED BY ARCHITECT
15 LIGHT FIXTURE - SEE ELECTRICAL DRAWINGS.
16 SUSPENDED ACOUSTICAL PANEL CEILING IN PRE-FINISHED METAL GRID SYSTEM.
17 6" 20 Ga. STEEL STUD FRAMING CONTINUOUS AT EXTERIOR OF STAIR.
18 PRE-FINISHED METAL CAP FLASHING WITH CONTINUOUS HOLD DOUN CLIPS - COLOR SELECTED BY ARCHITECT
19 MIN. R-38 BATT INSULATION W/ VAPOR BARRIER.
20 STRUCTURAL STEEL DECKING - SEE STRUCTURAL DRAWINGS.
21 2 1/2" 20 Ga. STEEL STUD - SECURE TO DECKING.
22 PRE-FINISHED METAL FLASHING CONTINUOUS - SEE STRUCTURAL DRAWINGS.
23 PROVIDE STEEL SUPPORT SYSTEM FOR ALL SUSPENDED ITEMS, DO NOT SUSPEND FROM METAL DECKING.
24 MECHANICAL DUCTING AND GRILLES - PAINT WHERE EXPOSED, SEE MECHANICAL DRAWINGS.
25 LOUVERED SHADE DEVICE PAINTED COLOR AS SELECTED BY ARCHITECT.
26 PRECAST CONCRETE SILL / LINTEL - COLOR SELECTED BY ARCHITECT.
27 5/ 8" TYP. "X" GYP. BD. CEILING / SOFFIT ASSEMBLY.
28 EIFS SOFFIT - COLOR AS SELECTED BY ARCHITECT.
29 3 5/ 8" 20 STEEL STUD FRAMING AT 16" O.C. MAX. SPACING. SECURE TO VAULT W/ POWDER ACTUATED FASTENERS AT 16" O.C. MAX. SPACING. PROVIDE SLIP JOINT AT TOP OF WALL - SEE B5/AE502 FOR TYPICAL SLIP JOINT FRAMING.
30 5 /8" TYP. "X" GYP. BD. OVER 3 5/8" x 22 GA METAL STUD FRAMING CONTINUOUS AT INFILL AT TOP OF VAULT. PROVIDE CONTINUOUS METAL EDGE TRIM TOP AND BOTTOM. PAINT COLOR AS SELECTED BY ARCHITECT.

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STATE OF UTAH · KENT R. RIGBY · 135096 · 12/16/05 · LICENSED ARCHITECT

OWNER INFORMATION

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Department of Administrative Services

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Salt Lake City, Utah 84114
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Fax: (801) 538 - 3267

Internet: http://www.dfcm.utah.gov

PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

BUILDING SECTIONS AND STAIR SECTIONS

REVISIONS

MARK	DATE	DESCRIPTION

ISSUE DATA

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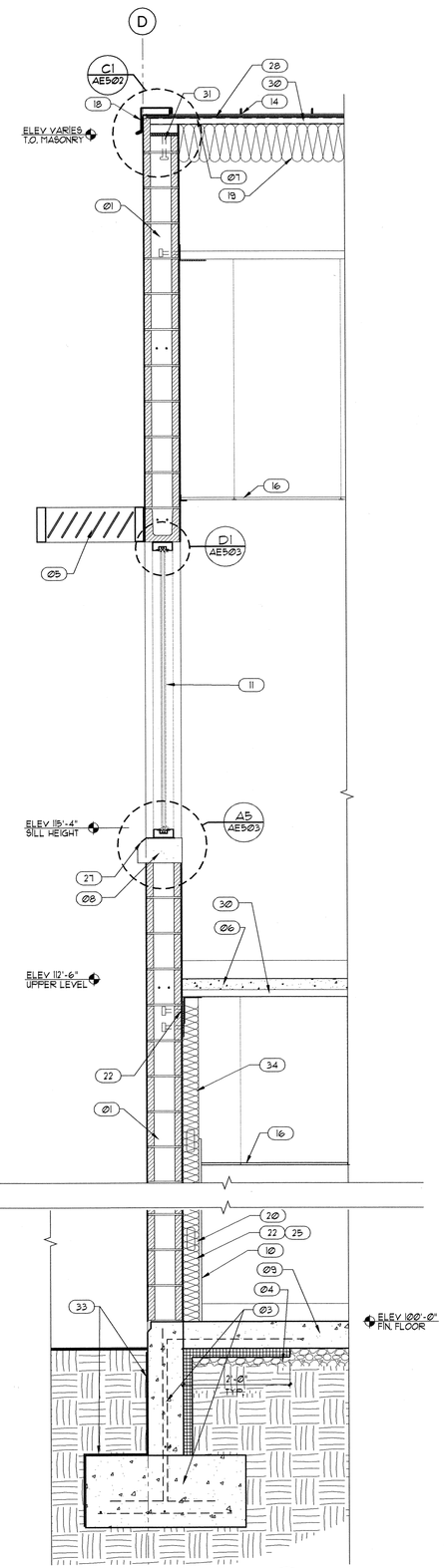
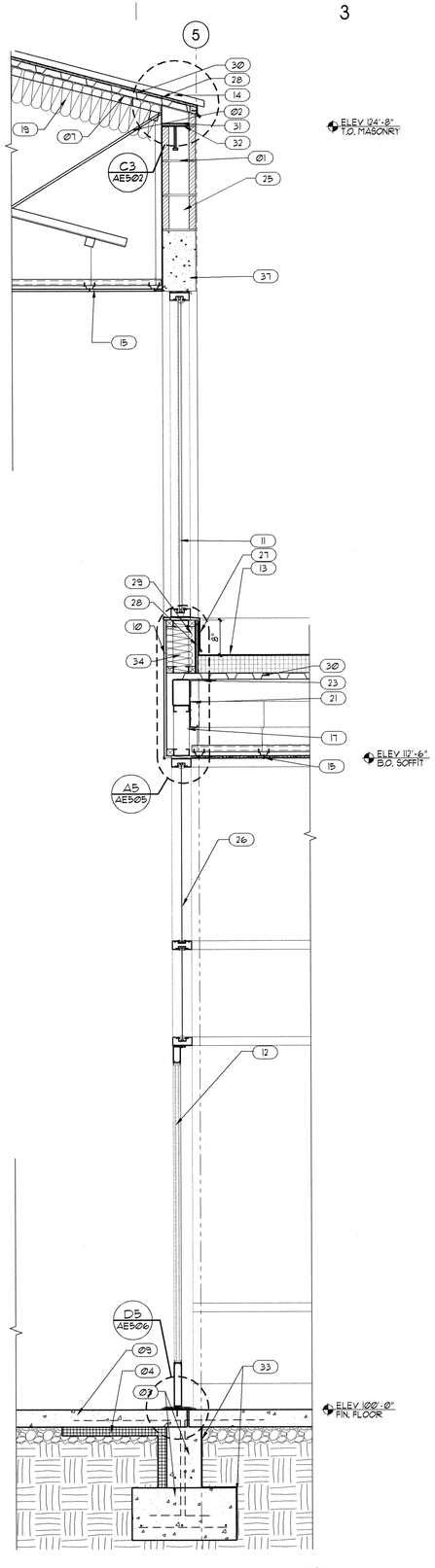
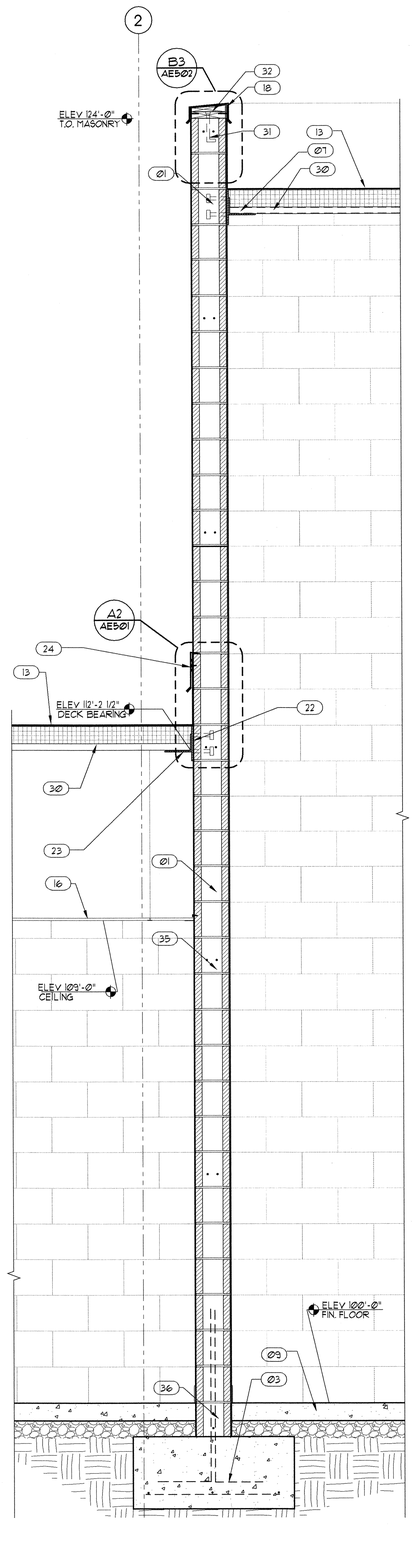


WALL SECTION A2 / AE303
SCALE: 3/4" = 1'-0"



WALL SECTION A3 / AE303
SCALE: 3/4" = 1'-0"



WALL SECTION A4 / AE303
SCALE: 3/4" = 1'-0"

GENERAL NOTES AND LEGEND:

*FOR SHEET AE303 ONLY.

- SEE SHEET G1002 FOR GENERAL NOTES.
- SEE TITLE SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.

KEYED NOTE LEGEND:

*FOR SHEET AE303 ONLY.

01 8" CMU WALL. SEE STRUCTURAL DRAWINGS, SEE WALL TYPES SHEET AE501.
02 STRUCTURAL STEEL FRAMING. SEE STRUCTURAL DRAWINGS.
03 REINFORCED CONCRETE FOOTING AND FOUNDATION. SEE STRUCTURAL DRAWINGS.
04 1-1/2" RIGID PERIMETER INSULATION.
05 LOUVERED SHADE DEVICE - GALVANIZED T.S. FRAME WITH GALVANIZED STEEL LOUVERS, GRIND ALL WELDS SMOOTH AND PAINT COLOR SELECTED BY ARCHITECT.
06 NEW CONCRETE SLAB OVER METAL DECKING. SEE STRUCTURAL DRAWINGS. SEE PLANS FOR FLOOR FINISH.
07 ROOF STRUCTURE. SEE STRUCTURAL DRAWINGS. SEE ROOF PLANS.
08 PRECAST CONCRETE SILL - COLOR SELECTED BY ARCHITECT.
09 CONCRETE FLOOR SLAB. SEE STRUCTURAL DRAWINGS.
10 5/8" TYPE "X" GYP. BD. TAPED, SANDED AND PAINTED (COLOR BY ARCHITECT.
11 ANODIZED ALUMINUM WINDOW SYSTEM WITH 1" INSULATED GLAZING.
12 ANODIZED ALUMINUM ENTRANCE DOOR WITH 1" INSULATED GLAZING.
13 FULLY ADHERED ENERGY STAR TPO ROOF MEMBRANE OVER MIN. R-30 RIGID INSULATION.
14 PRE-FINISHED STANDING SEAM ENERGY STAR ROOF - COLOR SELECTED BY ARCHITECT
15 5/8" TYPE "X" GYP. BD CEILING / SOFFIT ASSEMBLY.
16 SUSPENDED ACOUSTICAL PANEL CEILING IN PREFINISHED METAL GRID SYSTEM.
17 T.S. FRAMING AT ENTRANCE CANOPY - SEE STRUCTURAL DRAWINGS.
18 PREFINISHED CAP FLASHING WITH CONTINUOUS HOLD DOWN CLIPS - COLOR SELECTED BY ARCHITECT
19 MIN. R-38 BATT INSULATION W/ VAPOR BARRIER.
20 3 5/8" x 20 GAUGE STEEL STUD FRAMING AT 16" O.C. MAX. SPACING.
21 6" x 18 GAUGE STEEL STUD FRAMING AT 16" O.C. MAX. SPACING - SECURE TO T.S. FRAMING W/ TEK SCREWS AT 16" O.C.
22 MIN. R-19 BATT INSULATION.
23 DECK BEARING ANGLE - SEE STRUCTURAL DRAWINGS.
24 CONTINUOUS METAL REGLET FLASHING AND COUNTER FLASHING.
25 VAPOR BARRIER
26 ANODIZED ALUMINUM STOREFRONT SYSTEM WITH 1/4" GLAZING.
27 PREFINISHED METAL FLASHING - COLOR SELECTED BY ARCHITECT.
28 PTFT WOOD SHEATHING - SEE STRUCTURAL DRAWINGS.
29 PTFT WOOD BLOCKING AS REQUIRED.
30 STRUCTURAL METAL DECKING - SEE STRUCTURAL DRAWINGS.
31 PROVIDE 5/8" DIAM. J-BOLT FOR TOP PLATE HOLD DOWN AT 48" O.C. SEE STRUCTURAL.
32 PTDFFT TOP PLATE - SEE STRUCTURAL DRAWINGS.
33 DAMPPROOFING - CONTINUOUS AT ALL EXTERIOR FOOTINGS AND FOUNDATION WALLS
34 MIN. R-19 BATT INSULATION WITH VAPOR BARRIER - R-13 AT 3 5/8" STUD WALLS.
35 CMU H-BLOCK BOND BEAM TYP.
36 CMU STARTER COURSE, TYP.
37 COLORED PRECAST CONCRETE LINTEL - COLOR AS SELECTED BY ARCHITECT.

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STATE OF UTAH
KENT R. RIGBY
135096
12/16/05
LICENSED ARCHITECT

OWNER INFORMATION

State of Utah
Department of Administrative Services

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

WALL
SECTIONS

REVISIONS

MARK	DATE	DESCRIPTION

ISSUE DATA

ISSUE DATE:	DEC. 2005
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DFCM PROJECT #	04042480

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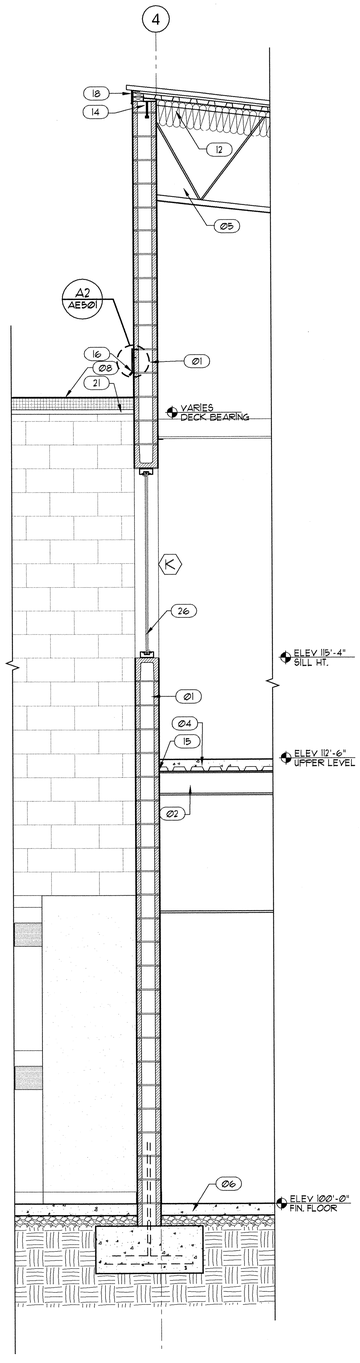
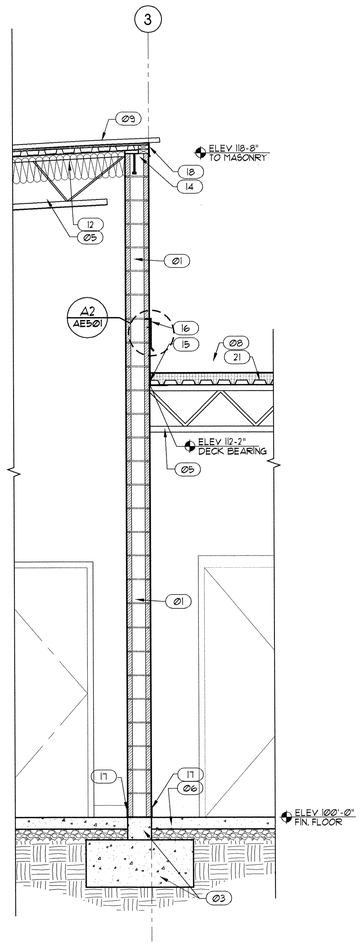


WALL SECTION A2 / AE304
SCALE: 1/2" = 1'-0"



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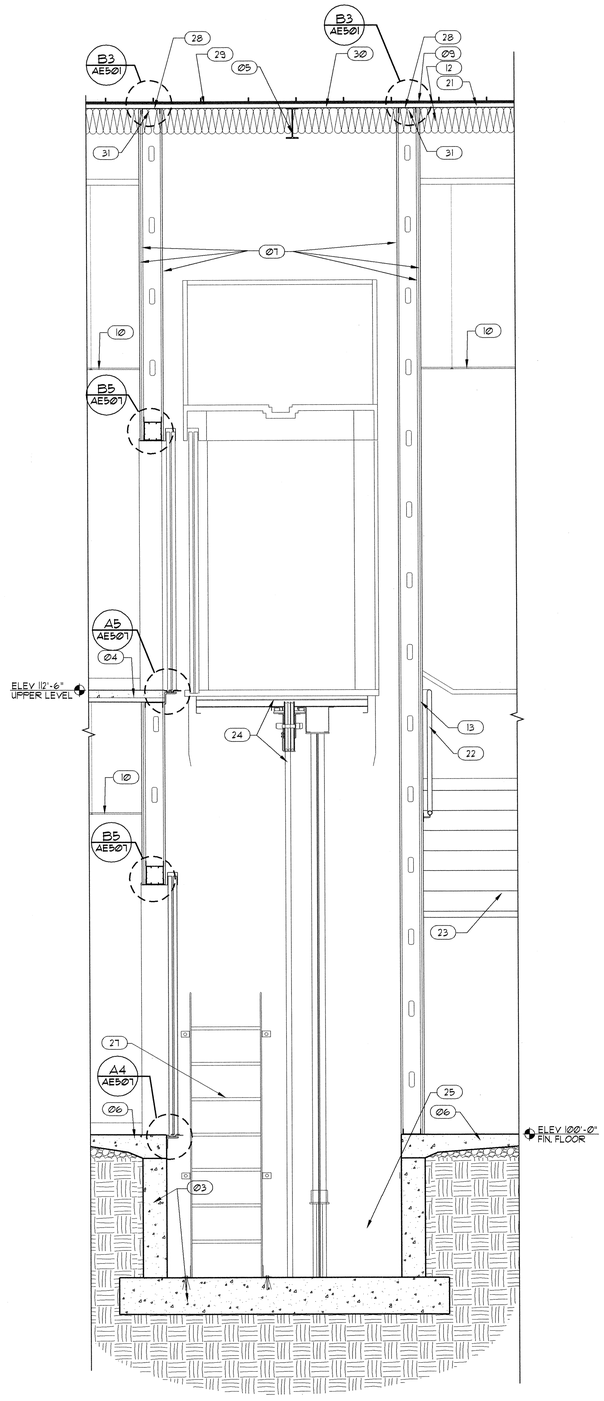

ELEVATOR SECTION A4 / AE304
SCALE: 1/2" = 1'-0"

GENERAL NOTES AND LEGEND:

*FOR SHEET AE304 ONLY.

- SEE SHEET G1002 FOR GENERAL NOTES.
- SEE COVER SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.

KEYED NOTE LEGEND:

*FOR SHEET AE304 ONLY.

Ø1 8" CMU WALL. SEE STRUCTURAL DRAWINGS.
Ø2 STRUCTURAL STEEL FRAMING. SEE STRUCTURAL DRAWINGS.
Ø3 REINFORCED CONCRETE FOOTING AND FOUNDATION. SEE STRUCTURAL DRAWINGS.
Ø4 NEW CONCRETE SLAB OVER METAL DECKING. SEE STRUCTURAL DRAWINGS. SEE PLANS FOR FLOOR FINISH.
Ø5 ROOF STRUCTURE. SEE STRUCTURAL DRAWINGS. SEE ROOF PLANS.
Ø6 CONCRETE FLOOR SLAB. SEE STRUCTURAL DRAWINGS.
Ø7 5/8" TYPE "X" GYP. BD. TAPED, SANDED AND PAINTED (COLOR BY ARCHITECT.
Ø8 FULLY ADHERED ENERGY STAR TPO ROOF MEMBRANE OVER MIN. R-30 RIGID INSULATION.
Ø9 PRE-FINISHED STANDING SEAM ENERGY STAR ROOF - COLOR SELECTED BY ARCHITECT
10 SUSPENDED ACOUSTIC PANEL CEILING IN PREFINISHED METAL GRID.
11 PREFINISHED CAP FLASHING WITH CONTINUOUS HOLD DOWN CLIPS - COLOR SELECTED BY ARCHITECT
12 MIN. R-38 BATT INSULATION W/ VAPOR BARRIER.
13 SHAFT WALL ENCLOSURE - SEE PLANS AND WALL TYPES.
14 BEARING PLATE WITH HEADED STUD ANCHORS - SEE STRUCTURAL DRAWINGS.
15 DECK BEARING ANGLE - SEE STRUCTURAL DRAWINGS.
16 CONTINUOUS REGLET AND COUNTER FLASHING.
17 4" RUBBER BASE.
18 PREFINISHED METAL FLASHING AND COUNTER FLASHING- COLOR SELECTED BY ARCHITECT.
19 PTFT WOOD SHEATHING - SEE STRUCTURAL DRAWINGS.
20 PTFT WOOD BLOCKING AS REQUIRED.
21 STRUCTURAL METAL DECKING - SEE STRUCTURAL DRAWINGS.
22 1-1/2" ø PIPE HANDRAIL. PAINTED.
23 CONC. FILLED METAL PAN STAIR TREAD.
24 ELEVATOR. SEE SPECIFICATIONS.
25 ELEVATOR PIT. SEE STRUCTURAL DRAWINGS.
26 ANODIZED ALUMINUM STOREFRONT SYSTEM WITH 1" INSULATED GLAZING.
27 ELEVATOR PIT LADDER - SEE B5/AE5Ø3.
28 FILL ROOF DECK FLUTES AT WALL INTERSECTION FOR RATED SHAFT, UL. #1477 G.C. TO PROVIDE ELEVATOR SHAFT VENTING PER IBC AND UTAH ELEVATOR INSPECTOR REQUIREMENTS.
29 RATED ROOF/CEILING ASSEMBLY REQUIRED FOR RATED SHAFT.
30 MAINTAIN WALL RATING AT STUD/DECK CONNECTION.
31

ARCHITECT AJC PROJECT #0470

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STATE OF UTAH
KERRY R. RIGBY
135096
12/16/05
LICENSED ARCHITECT

OWNER INFORMATION

State of Utah
Department of Administrative Services

Division of Facilities
Construction & Management
4110 State Office Building
Salt Lake City, Utah 84114
Phone: (801) 538 - 3018
Fax: (801) 538 - 3267

Internet: http://www.dfcm.utah.gov

PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:
WALL
SECTIONS

REVISIONS

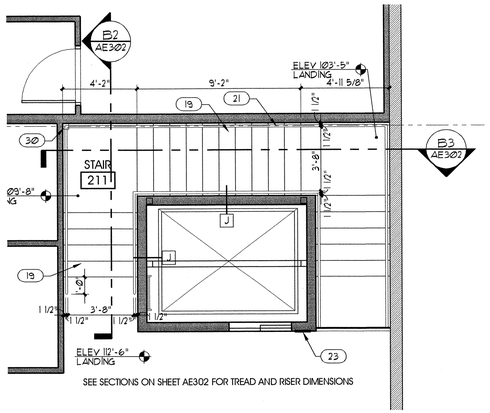
MARK	DATE	DESCRIPTION

ISSUE DATA

ISSUE DATE:	DEC. 2005
ISSUE TYPE:	CONDOCS
DRAWN BY:	BJA
CHECKED BY:	KRR
CAD FILE NAME:	0470AE304
DFCM PROJECT #	04042480

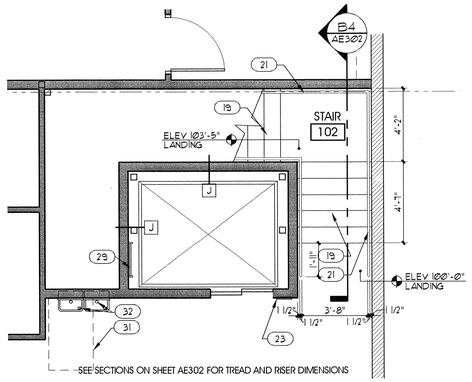
SHEET NUMBER:

AE304



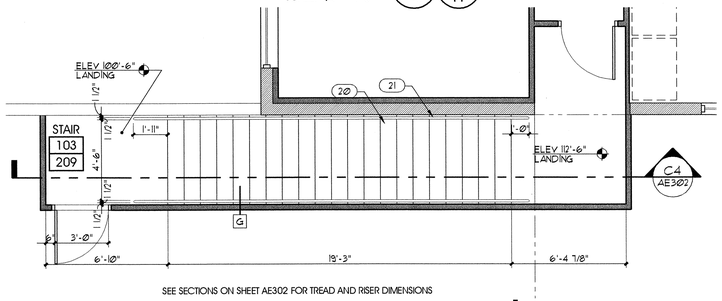
LARGE SCALE PLAN AT STAIR 211

SCALE: 1/4" = 1'-0" (D2 / AE401)



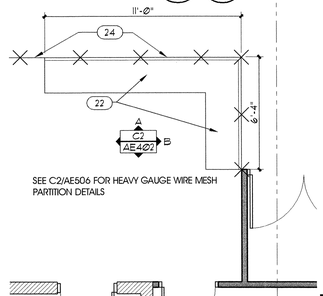
LARGE SCALE PLAN AT STAIR 102

SCALE: 1/4" = 1'-0" (D3 / AE401)



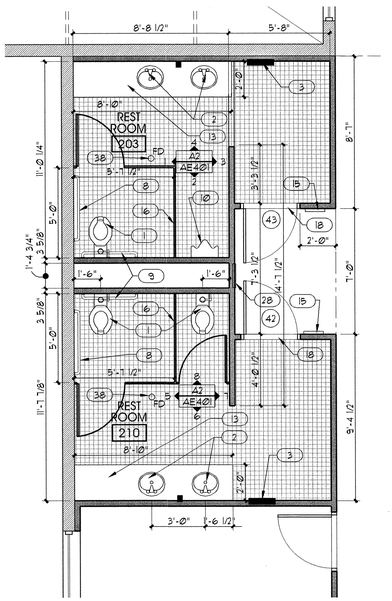
LARGE SCALE PLAN AT STAIR 103

SCALE: 1/4" = 1'-0" (C2 / AE401)



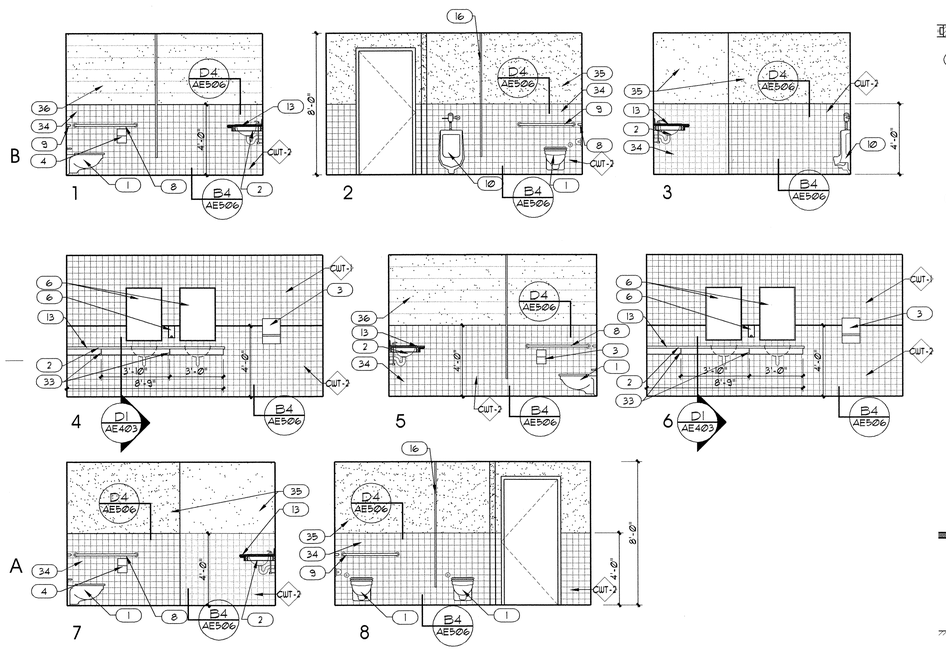
MILLWORK UNIT STORAGE 115

SCALE: 1/4" = 1'-0" (C3 / AE401)



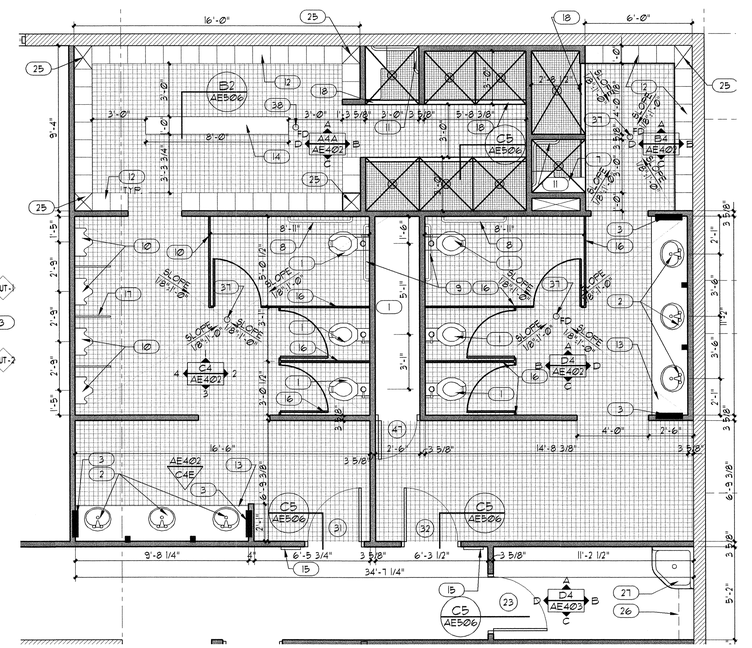
LARGE SCALE PLAN AT REST ROOMS

SCALE: 1/4" = 1'-0" (C4 / AE401)



LARGE SCALE PLAN AT RESTROOMS 203 AND 210

SCALE: 1/4" = 1'-0" (A2 / AE401)



LARGE SCALE PLAN AT LOCKER ROOMS

SCALE: 1/4" = 1'-0" (A4 / AE401)

GENERAL NOTES AND LEGEND:

*FOR SHEET AE401 ONLY.

SEE SHEET G1002 FOR GENERAL NOTES.

SEE TITLE SHEET FOR DRAWING INDEX.

DO NOT SCALE DRAWINGS.

CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.

◇ FINISH NOTE - SEE FINISH SCHEDULE SHEET AE602

KEYED NOTE LEGEND:

*FOR SHEET AE401 ONLY.

1. WALL MOUNTED WATER CLOSET - SEE PLUMBING DRAWINGS.
2. LAV - SEE PLUMBING DRAWINGS.
3. PAPER TOWEL DISPENSER - BOBRICK B-359 OR EQUAL.
4. TOILET PAPER DISPENSER - BOBRICK B-2888 OR EQUAL.
5. SOAP DISPENSER - BOBRICK B-211 OR EQUAL.
6. MIRROR - BOBRICK B165 2436 OR EQUAL.
7. TRANSFER TYPE ACCESSIBLE SHOWER STALL SEAT (FOLDING).
8. 42" GRAB BAR - BOBRICK B-6806x42" OR EQUAL.
9. 36" GRAB BAR - BOBRICK B-6806x36" OR EQUAL.
10. URINAL - SEE FLUMBING DRAWINGS.
11. SHOWER STALL GRAB BAR - BOBRICK B-6861 OR EQUAL.
12. 12"x12"x72" PRE-FINISHED STEEL LOCKER - COLOR SELECTED BY ARCHITECT, PROVIDE METAL BASE AND SLOPING METAL TOP.
13. SOLID SURFACE COUNTERTOP OVER PLYWOOD OR MEDEX BD.
14. PERMANENTLY ANCHORED CHANGING BENCH.
15. ROOM SIGNAGE
16. TOILET PARTITIONS.
17. URINAL SCREEN.
18. MARBLE THRESHOLD.
19. CONCRETE FILLED STEEL PAN STAIRS WITH RUBBER TREADS AND RISERS-SEE SECTIONS INDICATED
20. CONCRETE FILLED STEEL PAN STAIRS WITH CAST IN ALUMINUM DETECTABLE WARNING STRIP. EXPOSED CONCRETE TO BE SEALED. EXPOSED STEEL RISERS AND TS STRINGERS TO BE PAINTED. COLOR SELECTED BY ARCHITECT.
21. 1 1/2" OD STEEL PIPE RAILING PAINTED COLOR SELECTED BY ARCHITECT.
22. PLASTIC LAMINATE COUNTER SEE ELEVATIONS INDICATED.
23. ELEVATOR CALL STATION.
24. HEAVY GAUGE WIRE MESH PARTITION SEE C2/AE506 FOR DETAILS.
25. FILLER PANEL.
26. LINE OF MOP RACK - SEE D4/AE403.
27. MOP SINK - SEE PLUMBING DRAWINGS.
28. PROVIDE LOCKING 24"x16" ACCESS PANEL.
29. PIT LADDER FOR ELEVATOR - SEE A4/AE304.
30. LINE OF COLUMN BELOW.
31. DASHED LINE INDICATES REQUIRED CLEARANCE AT ACCESSIBLE WATER COOLER.
32. ELECTRIC WATER COOLER - SEE PLUMBING DRAWINGS.
33. ANGLE BRACE AT COUNTER - SEE SECTION INDICATED.
34. CERAMIC TILE WAINSCOT SEE FINISH SCHEDULE SHEET AE600. PROVIDE BULLNOSE EDGE - TYPICAL
35. EPOXY PAINT OVER 5/8" TYP. "X" WR GYP. BD. FULL HEIGHT IN WET AREAS. SEE FINISH SHEET AE601.
36. EPOXY PAINT OVER CMU. SEE FINISH SHEET AE601.
37. FLOOR DRAIN, SLOPE SLAB 1/8" PER 1'-0" AS SHOWN.
38. FLOOR DRAIN, NO SLOPE.

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STATE OF UTAH
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LICENSED ARCHITECT

OWNER INFORMATION

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

LARGE SCALE PLANS

REVISIONS

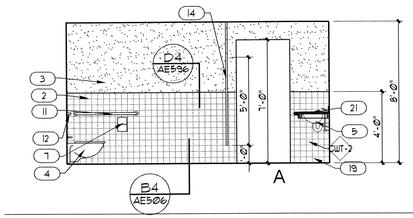
MARK	DATE	DESCRIPTION

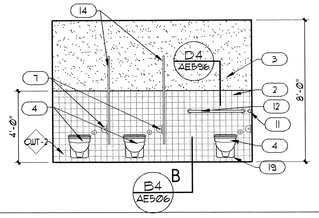
ISSUE DATA

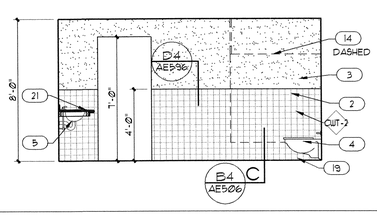
ISSUE DATE:	DEC. 2005
ISSUE TYPE:	CONDOCS
DRAWN BY:	BJA
CHECKED BY:	KRR
CAD FILE NAME:	0470AE401
DFCM PROJECT #	04042480

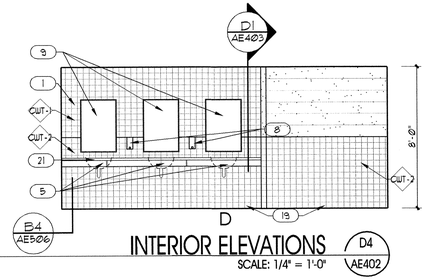
SHEET NUMBER:

AE401

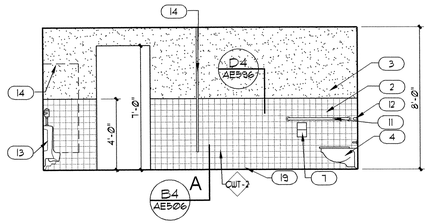


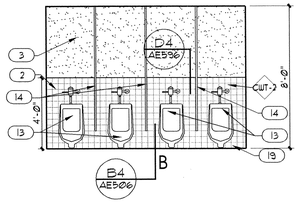


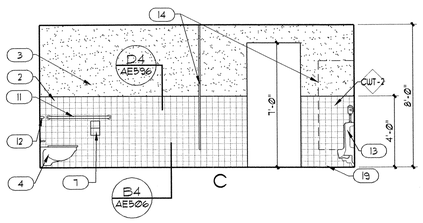


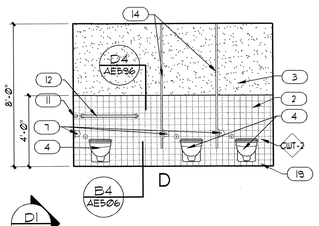


INTERIOR ELEVATIONS D4 AE402
SCALE: 1/4" = 1'-0"

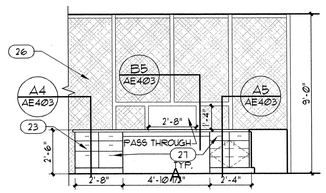


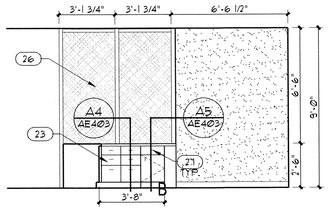






NOTE:
SEE C2/AE506 FOR WIRE MESH INSTALLATION DETAILS





INTERIOR ELEVATIONS C2 AE402
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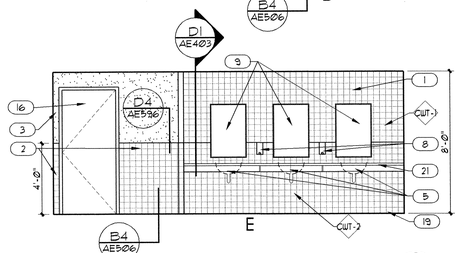

INTERIOR ELEVATIONS C4 AE402
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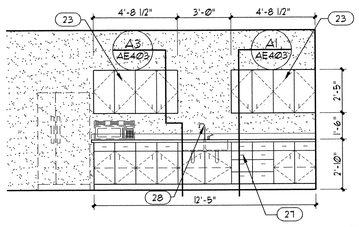
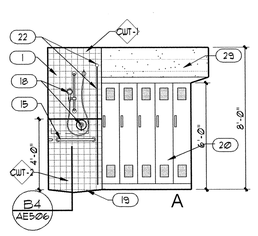
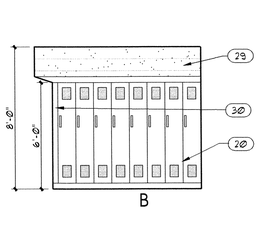
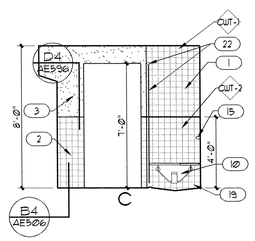
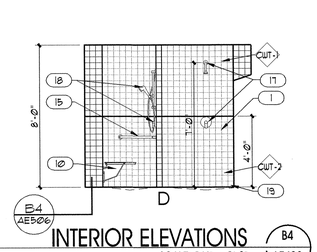


INTERIOR ELEVATIONS B1 AE402
SCALE: 1/4" = 1'-0"









INTERIOR ELEVATIONS B4 AE402
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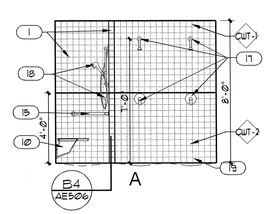
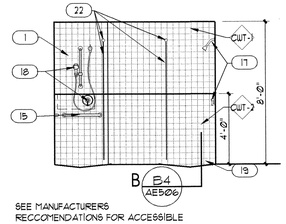
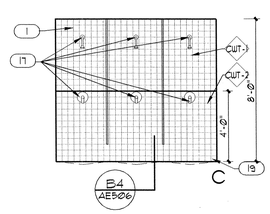
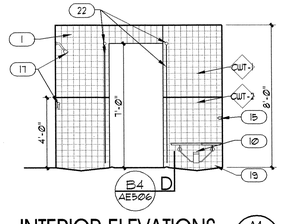




SEE MANUFACTURERS RECCOMENDATIONS FOR ACCESSIBLE SHOWER EQUIPMENT MOUNTING HEIGHTS





INTERIOR ELEVATIONS A4 AE402
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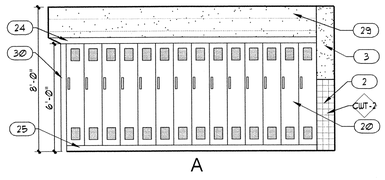

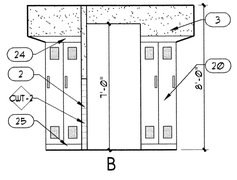

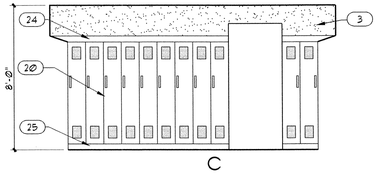

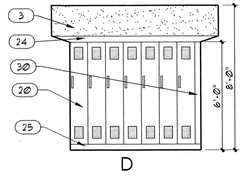

INTERIOR ELEVATIONS A4a AE402
SCALE: 1/4" = 1'-0"

GENERAL NOTES AND LEGEND:

*FOR SHEET AE402 ONLY.

- SEE SHEET G1001 FOR GENERAL NOTES.
- SEE COVER SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.

KEYED NOTE LEGEND:

*FOR SHEET AE402 ONLY.

1. CERAMIC WALL TILE FULL HEIGHT OVER 5/8" TYP. "X" WR GYP. BD. (CEMENT BD @ SHOWERS) SEE FINISH SHEET AE601.
2. CERAMIC TILE WAINSCOT SEE FINISH SCHEDULE SHEET AE601. PROVIDE BULLNOSE EDGE - TYPICAL
3. EPOXY PAINT OVER 5/8" TYP. "X" WR GYP. BD. FULL HEIGHT IN WET AREAS. SEE FINISH SHEET AE601.
4. WALL MOUNTED WATER CLOSET - SEE PLUMBING DRAWINGS.
5. LAV - SEE PLUMBING DRAWINGS. INSULATE PIPE PER CODE.
6. PAPER TOWEL DISPENSER - BOBRICK B-359 OR EQUAL.
7. TOILET PAPER DISPENSER - BOBRICK B-2888 OR EQUAL.
8. SOAP DISPENSER - BOBRICK B-211 OR EQUAL.
9. MIRROR - BOBRICK B-165 2436 OR EQUAL.
10. TRANSFER TYPE ACCESSIBLE SHOWER STALL SEAT (FOLDING). ANCHOR TO SOLID BLOCKING IN WALL.
11. 42" GRAB BAR - BOBRICK B-6806x42" OR EQUAL.
12. 36" GRAB BAR - BOBRICK B-6806x36" OR EQUAL.
13. URINAL - SEE PLUMBING DRAWINGS.
14. SOLID PHENOLIC CEILING HUNG TOILET PARTITION/URINAL SCREEN COLOR SELECTED BY ARCHITECT
15. SHOWER STALL GRAB BAR - BOBRICK B-6861 OR EQUAL.
16. DOOR / WINDOW - SEE PLANS AND DOOR / WINDOW SCHEDULE SHEET AE602
17. SHOWER NOZZLE AND CONTROLLER - SEE PLUMBING DRAWINGS.
18. ACCESSIBLE SHOWER NOZZLE AND CONTROLLER - SEE PLUMBING PLANS.
19. SANITARY TILE COVE BASE - COLOR SELECTED BY ARCHITECT.
20. 12"x12"x12" PRE-FINISHED STEEL LOCKER W/ METAL BASE & SLOPING TOPS - COLOR SELECTED BY ARCHITECT.
21. SOLID SURFACE COUNTERTOP OVER PLYWOOD OR MEDEX BD.
22. SHOWER CURTAIN AND CURTAIN ROD.
23. PLASTIC LAMINATE MILLWORK.
24. PROVIDE SLOPING TOP AT ALL LOCKERS.
25. 4" MTL. LOCKER BASE TO MATCH LOCKERS.
26. HEAVY GAUGE WIRE PARTITIONS.
27. WIRE PULLS.
28. SINK - SEE PLUMBING DRAWINGS.
29. EPOXY PAINT OVER CMU. SEE FINISH SHEET AE601.
30. METAL LOCKER FILLER PANEL.

NOTE: SEE B4/AE403 FOR TYPICAL ACCESSIBLE MOUNTING HEIGHTS.

PROVIDE PT BLOCKING FOR ALL ACCESSORIES.

ARCHITECT AJC PROJECT #0470

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STATE OF UTAH
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LICENSED ARCHITECT

OWNER INFORMATION

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:
INTERIOR ELEVATIONS

REVISIONS

MARK	DATE	DESCRIPTION

ISSUE DATA

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DFCM PROJECT #	04042480

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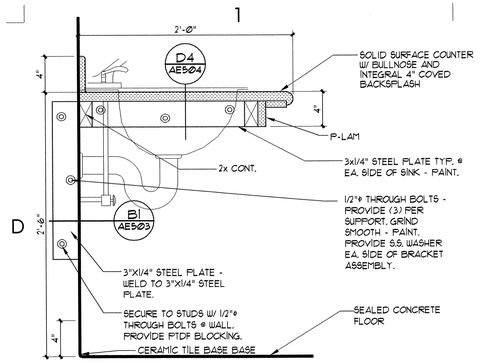
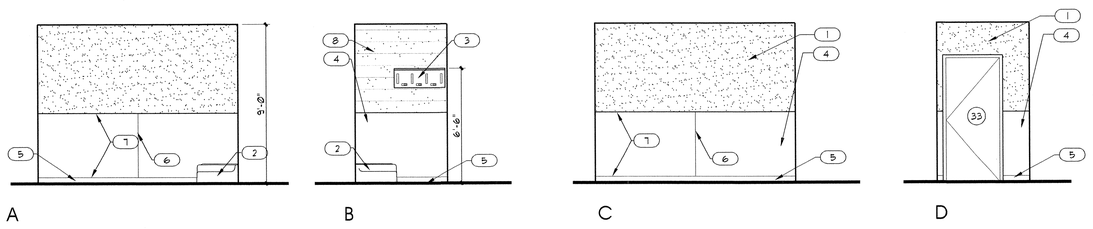


COUNTER SECTION (D1 / AE403)

SCALE: 1 1/2" = 1'-0"



INTERIOR ELEVATIONS AT JANITORS CLOSET #119 (D4 / AE403)

SCALE: 1/4" = 1'-0"

GENERAL NOTES AND LEGEND:

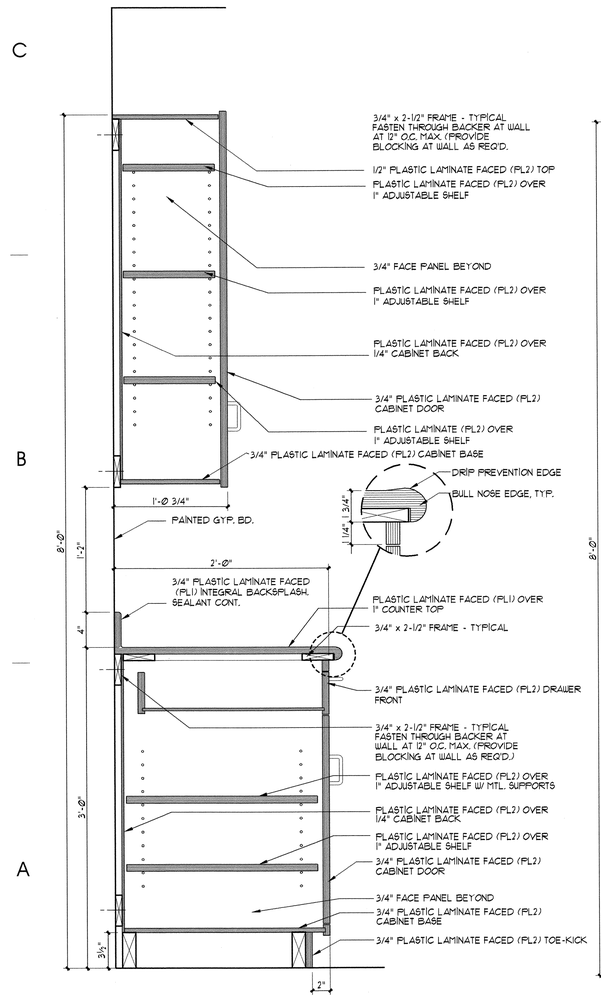
*FOR SHEET AE403 ONLY.

- SEE SHEET G1002 FOR GENERAL NOTES.
- SEE COVER SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.

KEYED NOTE LEGEND:

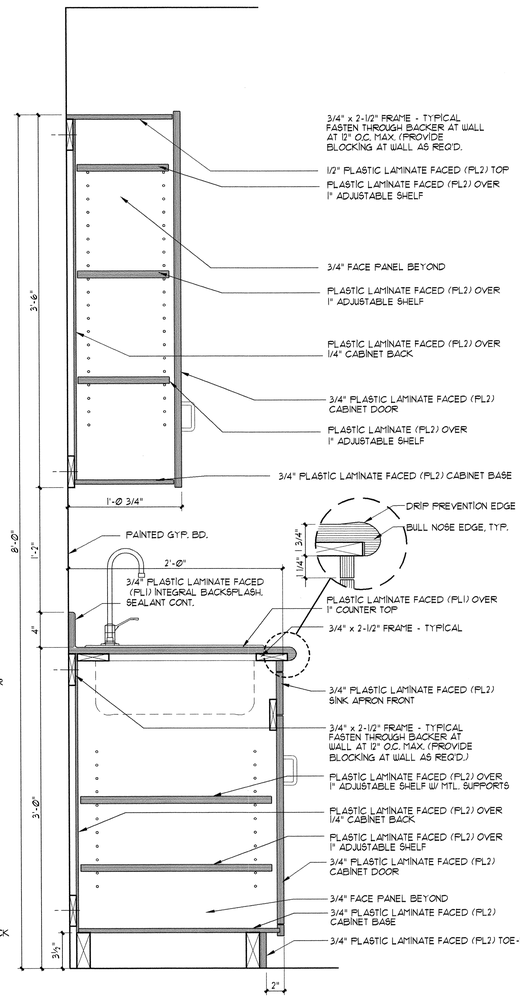
*FOR SHEET AE403 ONLY.

1. EPOXY PAINT OVER 5/8" TYP. "X" WR GYP. BD. FULL HEIGHT IN WET AREAS. SEE FINISH SHEET AE601.
2. SINK - SEE PLUMBING DRAWINGS
3. UTILITY SHELF WITH MOP AND BROOM HOLDERS - B-239-34 OR EQUAL.
4. FRP WAINSCOTTING - COLOR WHITE.
5. 4" RUBBER BASE COLOR SELECTED BY ARCHITECT
6. PLACE "T" MOLDING AT CENTER OF ROOM
7. METAL EDGE TRIM - TYP.
8. EPOXY PAINT OVER CMU. SEE FINISH SHEET AE601.



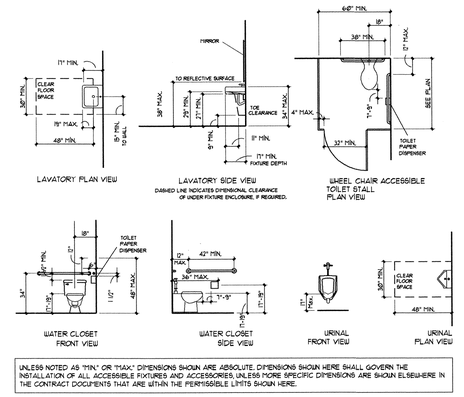
MILLWORK SECTION (A1 / AE403)

SCALE: 1 1/2" = 1'-0"



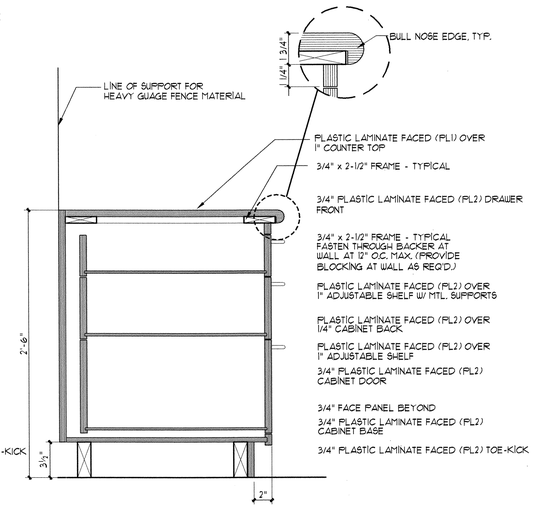
MILLWORK SECTION (A3 / AE403)

SCALE: 1 1/2" = 1'-0"



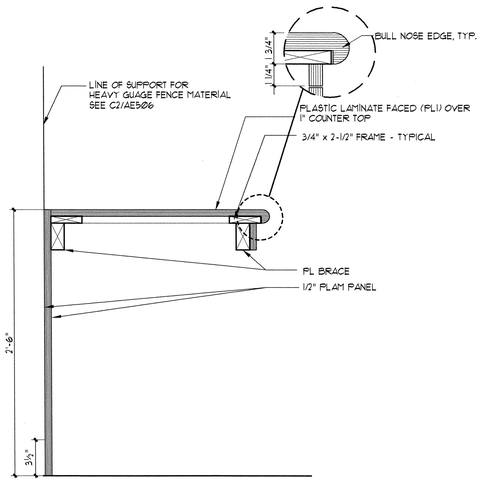
TYPICAL ACCESSIBLE MOUNTING HEIGHTS (B4 / AE403)

SCALE:NO SCALE



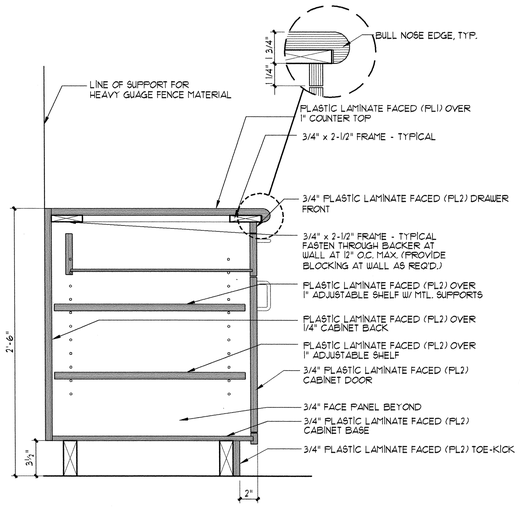
MILLWORK SECTION (A4 / AE403)

SCALE: 1 1/2" = 1'-0"



MILLWORK SECTION (B5 / AE403)

SCALE: 1 1/2" = 1'-0"



MILLWORK SECTION (A5 / AE403)

SCALE: 1 1/2" = 1'-0"

ARCHITECT AJC PROJECT #0470

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PROJECT DESCRIPTION

UTAH NATIONAL GUARD 144th COMPANY READINESS CENTER

CAMP WG WILLIAMS
RIVERTON, UTAH

SHEET NAME:

INTERIOR ELEVATIONS
MILLWORK SECTIONS

REVISIONS

MARK	DATE	DESCRIPTION

ISSUE DATA

ISSUE DATE:	DEC. 2005
ISSUE TYPE:	CONDOCS
DRAWN BY:	BJA
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